

KING SOLOMON BAPTIST CHURCH

ROOF REPLACEMENT - 50% CD SET
6125 FOURTEENTH STREET DETROIT, MI



4219 WOODWARD AVE
SUITE 301
DETROIT, MI 48201
v 313.462.2550

QUINNEVANS.COM

DRAWING INDEX

NUMBER	SHEET NAME	SUBMISSIONS		
		50% CD	100%CD	BID SET
G001	COVER SHEET	•		
G002	LEGENDS, SYMBOLS, ABBREVIATIONS	•		
G003	COMPOSITE ROOF ZONE KEY PLAN	•		
S-001	GENERAL STRUCTURAL NOTES	•		
S-100	FRAMING PLANS	•		
S-300	SECTION AND DETAILS	•		
AD110	DEMOLITION ROOF PLANS	•		
AD201	DEMOLITION ELEVATIONS	•		
AD202	DEMOLITION ELEVATIONS	•		
A110	ROOF PLANS	•		
A201	ELEVATIONS	•		
A202	ELEVATIONS & SECTIONS	•		
A301	DETAILS	•		
Grand total: 13				

NOT FOR CONSTRUCTION

KING SOLOMON BAPTIST CHURCH
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6125 FOURTEENTH STREET DETROIT, MI

PROJECT TEAM

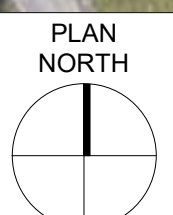
QUINN EVANS

ARCHITECT
4219 WOODWARD AVE
SUITE 301
DETROIT, MI 48201
v 313.3462.2550

RESURGET ENGINEERING

STRUCTURAL
28 W ADAMS AVE SUITE 1710
SUITE 1710
DETROIT, MI 48226
V 313.315.3290

VICINITY MAPS



No.	Date	Description

PROJECT MANAGER: A. CECIL
DESIGNED BY: S. RUTLAND

QEA No. 42134130

50% CD SET
1/24/2022

COVER SHEET

G001

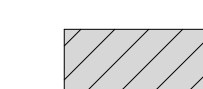

KING SOLOMON BAPTIST CHURCH

ROOF REPLACEMENT - 50% CD SET

6125 FOURTEENTH STREET DETROIT, MI

SITE INFORMATION	
ADDRESS	6125 14TH ST DETROIT, MI
LEGAL DESCRIPTION	W 14TH 34 THRU 38 PETER HUGHES 2ND SUB L26 PYS PLATS, W C R 10/57 39 AMENDED PLAT OF PETER HUGHES 2ND SUB L26 P88PLATS, W C R 10/56 216.12 IRREG
PARCEL ID #	10005106
SCOPE OF WORK	EXISTING BUILDING ROOF REPAIRS
PROPERTY CLASS	201 - COMMERCIAL
PROPERTY USE	22650 - RELIGIOUS STRUCTURE/USE
ZONING	B4
# OF BUILDINGS/ STRUCTURES	4
TOTAL AREA (SF)	35909
TOTAL ACREAGE	.526 AC
DEPTH X FRONTAGE (FT)	106 X 216
<i>*SOURCE CITY OF DETROIT PARCEL VIEWER</i>	

ROOF ZONE LEGEND

-  NIC - Not In Contract
-  Area of Work

No.	Date	Description

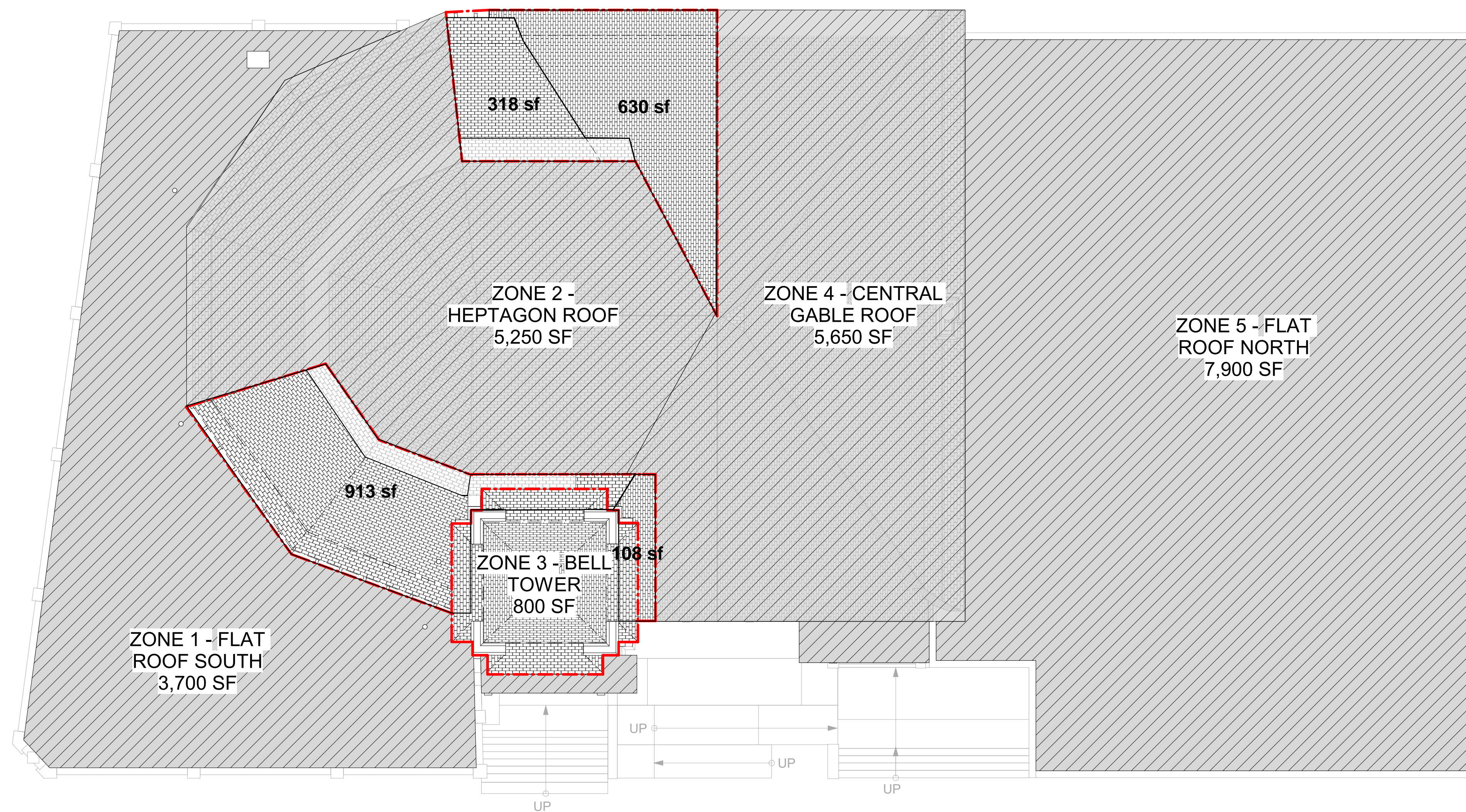
PROJECT MANAGER: A. CECIL
DRAWN BY: S. RUTLAND

QEA No.42134130

50% CD SET
1/24/2022

COMPOSITE ROOF ZONE KEY PLAN

G003



A5
G003
3/32" = 1'-0" REFERRED FROM:

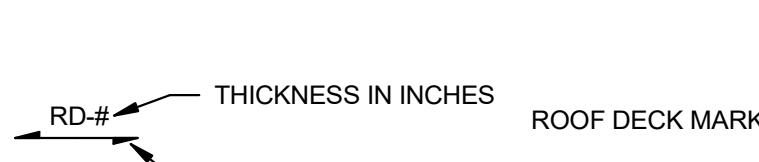
STATEMENT OF SPECIAL INSPECTIONS - WOOD CONSTRUCTION

TASK	INSPECTION FREQUENCY		REFERENCED STANDARD	MBC REFERENCE	RESPONSIBLE AGENT
	CONTINUOUS	PERIODIC			
1. PRE-FABRICATED WOOD					
A. INSPECTION OF FABRICATION PROCESS OF PRE-FABRICATED WOOD STRUCTURAL ELEMENTS.	-	X	MANUFACTURER'S CONTROL PROCEDURES	1704.2.5, 1705.5	SI

SPECIAL INSPECTION NOTES

- PERFORM SPECIAL INSPECTIONS IN ACCORDANCE WITH THE 2015 MICHIGAN (INTERNATIONAL) BUILDING CODE CHAPTER 17 AND AS MODIFIED IN THE MATERIAL SPECIFIC STATEMENTS OF SPECIAL INSPECTION.
- DESIGNATION OF RESPONSIBLE AGENT AND THEIR QUALIFICATIONS
 - SI SPECIAL INSPECTOR QUALIFIED WITH DEMONSTRATED COMPETENCE DOCUMENTED BY CERTIFICATIONS FROM RECOGNIZED AGENCIES SUCH AS AWS, ACI, MASONRY INSTITUTE OF MICHIGAN (MIM), ETC., AS SUBMITTED AND APPROVED BY THE BUILDING OFFICIAL. SPECIAL INSPECTOR MAY BE A FIRM WITH MULTIPLE SPECIALISTS AND A PROJECT MANAGER PROVIDING REPORTS.
 - TA TESTING AGENCY QUALIFIED TO TEST AND INSPECT MATERIALS AND ASSEMBLIES. TESTING AGENCY SHALL BE UNDER THE SUPERVISION OF THE SPECIAL INSPECTOR.
 - GE GEOTECHNICAL ENGINEER WHO PROVIDED THE ORIGINAL PROJECT GEOTECHNICAL SOILS INVESTIGATION REPORT.
 - SE SPECIALTY ENGINEER RESPONSIBLE FOR DESIGNING ASSEMBLIES SUCH AS PRECAST CONCRETE, STEEL JOISTS, COLD FORMED FRAMING ASSEMBLIES, ETC. SPECIALTY ENGINEER SHALL PROVIDE OBSERVATION OF FABRICATED AND INSTALLED ITEMS OF THEIR DESIGN IN ADDITION TO THE SPECIAL INSPECTION.
- TA, GE AND SE SHALL SUBMIT RECORDS OF THE INSPECTION RESULTS TO THE SI. THE SI SHALL COMPARE AND SUBMIT INSPECTION RECORDS TO THE ARCHITECT/ENGINEER AND BUILDING OFFICIAL. RECORDS SHALL INCLUDE STATEMENTS OF TESTS, WHETHER INSTALLED/FABRICATED ITEM COMPLIES WITH CONTRACT DOCUMENTS, REMEDIAL WORK PERFORMED, RETESTS.
- SI SHALL PROVIDE A DAILY REPORT OF ANY DISCREPANCIES FROM THE CONTRACT DOCUMENTS FOUND ON THE SAME DAY OF THE INSPECTION TO THE ENGINEER OF RECORD. FORMAL REPORTS OF COMPLIANCE CAN FOLLOW BY A MAXIMUM OF 2 WEEKS. SI SHALL PROVIDE AND SIGN FINAL REPORT WITH A SUMMARY OF ALL TESTS PERFORMED AND RESULTS OF THE ENGINEER OF RECORD AND BUILDING OFFICIAL, IN ACCORDANCE WITH SECTION 1704.2.4.
- SI, TA & GE SHALL BE PAID BY THE OWNER IN COMPLIANCE WITH THE MICHIGAN (INTERNATIONAL) BUILDING CODE.
- WHERE FABRICATION OF STRUCTURAL LOAD-BEARING OR LATERAL LOAD-RESISTING MEMBERS OR ASSEMBLIES IS BEING CONDUCTED ON THE PREMISES OF A FABRICATOR'S SHOP, SPECIAL INSPECTIONS OF THE FABRICATED ITEMS SHALL BE PERFORMED DURING FABRICATION. SPECIAL INSPECTIONS DURING FABRICATION ARE NOT REQUIRED WHERE THE FABRICATOR MAINTAINS APPROVED DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES THAT PROVIDE A BASIS FOR CONTROL OF THE WORKMANSHIP AND THE FABRICATOR'S ABILITY TO CONFORM TO APPROVED CONSTRUCTION DOCUMENTS AND THE GOVERNING BUILDING CODE. APPROVAL SHALL BE BASED UPON REVIEW OF FABRICATION AND QUALITY CONTROL PROCEDURES AND PERIODIC INSPECTION OF FABRICATION PRACTICES BY THE BUILDING OFFICIAL. SPECIAL INSPECTIONS ARE NOT REQUIRED WHERE THE FABRICATOR IS REGISTERED AND APPROVED IN ACCORDANCE WITH SECTION 1704.2.5.1.
- REFER TO MATERIAL SPECIFIC STATEMENTS OF SPECIAL INSPECTION AND GENERAL STRUCTURAL NOTES FOR ADDITIONAL QUALITY CONTROL, TESTING AND INSPECTIONS.

DECK AND SLAB SYMBOLS



SPECIAL CHARACTERS

- Ø Diameter, Small
- ∅ Diameter, Large
- Degree
- Bullet Point, Rd. Small
- Bullet Point, Rd. Large
- Bullet Point, Sq. Small
- Bullet Point, Sq. Large
- ± Plus or minus

ABBREVIATIONS

ADD	ADDITIONAL
APPROX	APPROXIMATE
ARCH	ARCHITECTURAL
BF	BRACED FRAME
BOF	BOTTOM OF FOOTING
BOS	BOTTOM OF STEEL
BP	BEARING PLATE
BRG	BEARING
CANT	CANTILEVERED
CFM	COLD FORMED METAL FRAMING
CP	CAST IN PLACE
CJ	CONTROL OR CONSTRUCTION JOINT
CMU	CONCRETE MASONRY UNIT
CCL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONT	CONTINUOUS OR CONTINUATION
CONTR	CONTRACTOR
COORD	COORDINATE
DIA	DIAMETER
DIAG	DIAGONAL
DM	DIMENSIONS
DWG	DRAWING
EF	EACH FACE
EL	ELEVATION
EQUAL	EQUAL
EW	EACH WAY
EX	EXISTING
EXP	EXPANSION
EXT	EXTERIOR
FIN	FINISH FLOOR
FOUND	FOUNDATION
FTG	FOOTING
GALV	GALVANIZED
GB	GRADE BEAM
HORIZ	HORIZONTAL
INT	INTERIOR
JOINT	JOINT
LLBB	LONG LEGS BACK TO BACK
LSH	LONG SIDE HORIZONTAL
LSV	LONG SIDE VERTICAL
MAX	MAXIMUM
MECH	MECHANICAL
MIN	MINIMUM
MP	MASONRY PIER
NIC	NOT IN CONTRACT
OC	ON CENTER
OD	OVERFLOW ROOF DRAIN
OH	OPPOSITE HAND
OPNG	OPENING
PC	PRECAST
PERIM	PERIMETER
PL	PLATE
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
RD	ROOF DECK
REF	REFERENCE
REINF	REINFORCING
REQD	REQUIRED
RTU	ROOF TOP UNIT
SIM	SIMILAR
SLBB	SHORT LEGS BACK TO BACK
SOG	SLAB ON GRADE
SP	SPACING
TOC	TOP OF CONCRETE
TOP	TOP OF FOOTING
TOSLAB	TOP OF SLAB
TOS	TOP OF STEEL
TYP	TYPICAL
UN	UNLESS OTHERWISE NOTED
VERT	VERTICAL
VIF	VERIFY IN FIELD
W/O	WITHOUT
WWF	WELDED WIRE FABRIC

WOOD

- Framing Lumber: Spruce Pine Fir No. 2 or better or as noted otherwise.
- Laminated Veneer Lumber (LVL): All LVL members shall have the following minimum properties: Fb = 2600 psi, Fv = 285 psi, E = 1900 ksi.
- TimberStrand (LSL): All LSL members shall have the following minimum properties: Fb = 1700 psi, Fc = 635 psi, E = 1900 ksi.
- Parallel (PSL): All PSL posts shall have the following minimum properties: Fb = 2400 psi, Fc parallel to grain = 2500 psi, E = 1800 ksi.
- Wolmanized Parallel (WPSL): All exterior exposed posts shall be WPSL posts shall have the following minimum properties: Fb = 1728 psi, Fc parallel to grain = 1450 psi, E = 1566 ksi.
- Wood Structural Panel Sheathing: All panels shall be and rated by the American Plywood Association (APA).
- Nails: Standard Common with the following minimum penetrations into support member:
 - 6d (diameter 0.113") with 1.25" penetration
 - 8d (diameter 0.131") with 1.50" penetration
 - 10d (diameter 0.148") with 1.62" penetration
 - 16d (diameter 0.162") with 1.75" penetration
- Bolts for connections: ASTM A307 with ASTM A563 heavy hex nuts and hardened washers, Grade A, unless noted otherwise.
- Special Treatments (American Wood Preservers Institute Standards): All wood in contact with concrete, masonry or soil. Pressure treated with Wolman CCA preservative or equal.
- Minimum Nailing Requirements (See drawings for areas with greater requirements):
 - A. Roof: Nail all sheathing panels with 8d common nails at 6" o.c. at all supported edges and at 12" o.c. at all intermediate supports. Use two plyclips between each support for spans of 48" o.c. and one plywood clip between each support for lesser spans at all unsupported panel edges.
 - B. Floor: Nail all sheathing panels with 8d common nails at 6" o.c. at all supported edges and 8d at 12" o.c. at all intermediate supports.
 - C. Walls: Nail all sheathing panels with 8d common nails at 6" o.c. at all edges and at 12" o.c. at all intermediate supports (3/8" or 7/16" panels on studs spaced at 24" o.c. requires 6" spacing at all intermediate supports).
- General Framing and Carpentry: Connect all items as per NDS "Fastening Schedule", unless noted otherwise.
- Framing Connections
 - A. All framing connections not shown or otherwise indicated on the drawings shall be connected in a manner similar to the connections shown in the drawings or with approved Simpson Strong-Tie Connectors or Equal. The following notations refer to Simpson Strong-Tie Connectors:
 - Joist and Rafters: "J" or "R" hangers as required.
 - Beams: "EG" Hangers and "HGLB" Beam Seats.
 - Hinge connectors: "HCCT"
 - Columns: "CC" Column and "CB" Column Bases.
 - Hold Down Anchors: "HDL" and "HTT."
- Blocking, Bridging, and Bracing: Provide solid shaped blocking at least 2" (nominal) thick and full depth of joist at ends and at each support of joist. Provide approved bridging at 8'-0" o.c. maximum between joist end supports. Solid blocking between joists shall be nailed to the wood plate at the top of the wall with one Simpson "AS3" framing anchor per each piece of blocking. Fill all holes in the framing anchors with 8-d short nails.
- Laminated built-up beams of 2x member 12 in. or less in depth shall be spiked together with not less than 16-d spikes at twelve-inch (12 in.) centers, staggered. Unless so spiked, or if the depth of beam is more than twelve inches (12 in.), the laminations shall be connected together with 1/2" diameter bolts at 24 in. o.c. staggered. Bolts shall be placed 1/4" of the depth of the member from the top and bottom of the member.
- Pre-fabricated Steel Plate Wood Trusses:
 - A. Design Loading: The truss manufacturer is responsible for design and fabrication of the trusses. They shall be designed to support the concentrated and other distributed loads as shown on the framing plans in addition to the following uniform loads:
 - Roof Trusses:
 - Dead Load (Top Chord) = 10 psf
 - Dead Load (Bottom Chord) = 15 psf
 - Live Load (Top Chord) = 20 psf
 - Wind loading per load maps
 - Total load deflection limit = span/240
 - Live load deflection limit = span/360
 - B. Correlate the design with all mechanical equipment, fire sprinkling systems and hanging walls supported by the trusses. Provide extra trusses where required.
 - C. Submittals: Complete calculations and shop drawings indicating all member forces, stresses, lumber grades, dimensions, steel truss plate sizes and locations shall be submitted and reviewed by the engineer before fabrication. Each connector shall be dimensioned on the shop drawings as to its exact location at the joint. Shop drawings and calculations shall bear the seal of a professional engineer licensed in the State of the Project. After truss installation, the fabricator shall certify in writing that the trusses have been installed according to its specifications.
 - D. Steel Connector Plates: Use only galvanized steel connector plates that comply with the Truss Plate Institute publication, TP1-1995. All steel connector plates must be approved by the International Conference of Building Officials Evaluation Services. Submit a copy of the ICBO Report for the connector plate used. Values established by this committee must be indicated on the shop drawings.
 - The minimum size for any connector shall be 15 square inches.
 - All steel gusset plates shall be located on the joint as the stresses require and shall provide a minimum bite of 2.5" length on all tension members.
 - Plates shall be pressed or rolled into member to obtain full penetration without crushing the outer surfaces of wood.
 - Steel plates at compression web members shall be designed to resist 100% of the compression force without considering wood to wood bearing.
 - All steel plate dimensions shall be increased by 10% above that required by analysis. Stress increases for steel connector plate values for duration of load are not allowed.
 - E. Wood Members: All wood members of the truss shall be constructed of kiln dried lumber. The trusses shall be handled and stored in a manner to prevent moisture from being absorbed by the wood. Grade stamps shall be visible on framing members. Splices in chords shall occur at 1/4 of the panel span from a joint.
 - F. The trusses shall be designed by the truss supplier according to the following criteria:
 - Bending moments in the top and bottom chords shall be based on the following moment coefficients:
 - 1/8 for one and two continuous span conditions.
 - 1/10 for three or more continuous span conditions.
 - Web members shall be designed using an effective length factor: K = 1.0
 - G. Lateral Bracing: Lateral bracing and bridging may be required by the design of the pre-fabricated wood roof truss to reduce the buckling length of individual truss members and provide stability during erection. This bracing or bridging may be in the form of 2 x 4 horizontal bracing or bridging with 2 x 4 cross-bracing spaced at 24'-0" o.c. maximum and at each end of the bracing or bridging. The 2 x 4 cross bracing shall be connected to the truss top chord and the horizontal bracing and truss bridging is to be supplied and installed at the location specified on the pre-fabricated wood roof truss design drawings by the General Contractor.
 - H. Other requirements for truss stability and erection shall comply with the Truss Plate Institute publications entitled "Commentary and Recommendations for Bracing Wood Trusses" and "Commentary and Recommendations for Handling and Erecting Wood Trusses". The contractor shall have copies of these publications on site and shall be familiar with their contents.
 - I. Prior to the fabrication of the pre-fabricated wood trusses, the contractor shall submit, in writing, proof of compliance of in-plant inspection by an ICBO approved independent inspection agency. The in-plant inspections shall comply with section 1704.2 of the International Building Code.
 - J. The truss manufacturer's identification stamp shall be clearly visible.

POST-INSTALLED ANCHORS IN CONCRETE

- Expansion anchors
 - A. Expansion Anchors shall be per CODE requirements.
 - B. Expansion Anchors shall be: Kwik-Bolt TZ (ESR-1917) by Hill, Power-Such SD2 (ESR-2502) by Power Fasteners, Strong Bolt (ESR-1771) by Simpson, TruBolt® (ESR-2427) by ITW Red Head or approved equal.
 - C. For interior condition use carbon steel anchors and for exterior condition use stainless steel anchors.
 - D. Tension test 50% of all expansion anchors to test load provided by manufacturer.
- Adhesive anchors
 - A. Comply with CODE requirements.
 - B. Adhesive anchors shall be HIT-HY 200 (ESR-3187) by Hill, HIT-RE 500 SD (ESR-2732) by Hill, Set-XP (ESR-2508) by Simpson, or approved equal.
 - C. For interior condition use carbon steel anchors and for exterior condition use stainless steel anchors.
 - D. Tension test 50% of all expansion anchors to test load provided by manufacturer.

SPECIAL INSPECTIONS

- Special inspections shall be provided by the Owner's Testing Lab in accordance with the code and the project specifications. The special inspector shall observe the work for conformance with the construction documents. The special inspector shall send reports to the inspector of record, architect, engineer, contractor and Owner. All discrepancies shall be brought to the attention of the contractor for correction. When work is done to the satisfaction of the inspector, then the special inspector shall submit a final signed report stating that, to the best of their knowledge, the work was completed in conformance with the plans, specifications, and the applicable workmanship provisions of the CODE.
- Refer to Special Inspection tables and notes for specific requirements.

EXISTING CONSTRUCTION

- Before submitting a proposal for work, and/or preparing shop drawings for this work each Bidder, Contractor and Sub-Contractor shall visit the site and become fully acquainted with the existing conditions, temporary construction required, type of equipment required to perform the work.
- Verify the existing, location and elevation of existing utilities, sewers, drains, etc. in demolition areas and adjacent to new work before proceeding, do not proceed with work until discrepancies have been resolved.
- Provide fire safety precautions during field cutting and welding operations, meeting the Owner's requirements.
- Provide temporary protection of existing equipment during work, satisfying the Owner's requirements.
- Provide temporary protection to prevent damage from the weather and vandalism.
- Coordinate work with the Owner's personnel to avoid any interference in their operations.
- Refer to "SHORING AND BRACING" notes for additional requirements.

SHORING AND BRACING

- Contractor shall provide temporary shoring and bracing of existing construction, new construction and underground utilities as follows:
 - A. Where shown or noted on the Drawings.
 - B. Where existing construction is to be altered or disturbed until permanent support is in place.
 - C. Where existing construction is not undergoing alteration and is to remain undisturbed but is disturbed as a result of the work of this contract.
 - D. As required for safe erection, installation of new construction, equipment, etc.
- When needed for Contractor's "means and methods" of construction, and other safety related issues:
 - A. Shoring and bracing shown on the Drawings is conceptual. Contractor shall be responsible for verifying existing conditions, shoring and bracing calculations, methods of installation, transfer of loads through to final load support, and work sequence phasing with new construction.
 - B. Shoring and bracing shall be performed by a Contractor with minimum 5 years demonstrated experience in similar size and scope of shoring and bracing projects.
 - C. Shoring and bracing shall be designed by a Professional Engineer registered in the State of the Project with minimum 5 years demonstrated experience in similar size and scope of shoring and bracing projects. Design loads and methods shall conform to applicable codes. Soil and material strengths shall be verified by tests, unless conservative estimates that do not affect deflections and deformations are approved by the Architect/Structural Engineer.
 - D. Contractor shall submit drawings and calculations sealed and signed by the Contractor's Professional Engineer showing complete design including temporary conditions, final conditions and sequence of work.
 - E. Before starting work, Contractor shall perform condition survey of the existing building structure, exterior facade and interior finishes, including photographic documentation and submit survey to the Owner for record.
 - F. During the shoring and bracing operations, Contractor shall:
 - A. Keep the existing and new construction in a safe condition.
 - B. Monitor existing and new construction to detect any signs of distress or deformation.
 - C. Take immediate steps to prevent distress, deformation or damage.
- Contractor shall continuously monitor the shoring and bracing system. Contractor shall review and ascertain that all field connections are completed according to the Contractor's design and issue approval for inspection of the work by the Testing Agency.
- After completion of shoring and bracing and completion of work requiring shoring and bracing, Contractor shall repair any damage to the existing and new construction, without any cost to the Owner, and to the satisfaction of the Owner and Architect/Structural Engineer.

GENERAL NOTES

- Governing Design Code: 2015 Michigan Building Code with local jurisdiction amendments (hereafter referred to as "CODE").
- All construction shall be in accordance with the following:
 - A. CODE
 - B. Drawings and Specifications
- The structural drawing notes are intended to work together and be complementary with the project specifications. Consult the specifications for additional requirements in each section. Information provided on structural drawings shall take precedence over typical details and structural notes.
- Typical details and general notes shall apply, UNO.
- The structural drawings shall be used in conjunction with the architectural drawings. See architectural drawings for information not shown, including but not limited to the following:
 - A. Setting out dimensions and angles of all grid lines
 - B. Setting out dimensions of concrete walls and wall openings that are not shown on the structural drawings.
 - C. Dimensions not shown on the structural drawings
 - D. Waterproofing system and details
- Contractor is responsible for the coordinating all equipment pad sizes and locations with the actual layout provided in the shop drawings.
- Drawing scales noted on structural drawings are for reference only. Do NOT scale drawings. The contractor shall verify dimensions provided with the architect prior to proceeding with work.

STRUCTURAL OBSERVATIONS

- Resurget Engineering shall provide Structural Observation of the structural systems for general conformance to the drawings and specifications at significant stages of construction and at completion of the primary structural system as defined in Code.
- Structural Observations does not include or waive any of the responsibilities of the Special Inspector as required per the Section "Special Inspections".
- At the conclusion of work included in permit, the structural observer will submit to the building official a written statement that the structural observations have been completed and that to the best of their knowledge the work is in conformance with the construction documents.
- Structural Observation on this project shall be conducted on the following structural elements:
 - A. Stick Built Wood Construction

SHOP DRAWINGS:

- Verify all existing dimension before submitting shop drawings for review.
- Review all shop drawings for accuracy and compliance with shop drawing before submitting for review. Review of shop drawings does not relieve the Contractor of any responsibility or errors and omissions.
- Use of 2D Drawing or 3D REVIT model does not relieve the Contractor of any responsibility specified in the contract documents.
- Allow a minimum of 10 working days for review by Structural Engineer of each set of submitted contract drawing. Submit shop drawings in reasonable quantities with at least 10 working days before submittals. Review time stated is for Structural Engineer only, add additional time to schedule as required for review by other disciplines.
- Contractor shall coordinate work between multiple trades before submitting shop drawings. Dimensions and elevations specific to equipment installation shall be provided and coordinated prior to submittal for review. Failure to provide these dimensions shall result in return of shop drawings without review.
- Structural Engineer is not responsible for coordination of work marked as "by others" on shop drawings.

DESIGN CRITERIA

Design is in accordance with CODE	CODE REFERENCE
Risk Category	III IBC Table 1604.5 ASCE Table 1.5-1

FLOOR LIVE LOADS

ROOF	20 PSF	CODE REFERENCE
		ASCE Table 4-1

SNOW LOADS

Ground Snow Load	Pg = 25 PSF	CODE REFERENCE
Flat Roof Snow Load	Pf = 22 PSF (minimum)	ASCE Section 7.3
Exposure Factor	Ce = 1.0	ASCE Table 7-2
Importance Factor	I = 1.1	ASCE Table 1.5-2
Thermal Factor	Ct = 1.0	ASCE Table 7-3

WIND LOADS

Ultimate Design Wind...	V(ULTIMATE) = 120 MPH	CODE REFERENCE
Nominal Design Wind Speed	V(SERVICE) = 89 MPH	IBC Section 1609.3.1
Exposure Category	B	ASCE Section 26.7.3
Internal Pressure...	± 0.18 (Enclosed)	ASCE Section 26.11-1

COMPONENTS AND CLADDING ROOF

Support Beams (A > 100 SF)	Zone 1			CODE REFERENCE
	Zone 2	Zone 3	Zone 4	
Roof Sheathing (A = 50 SF)	-25 PSF	-29 PSF	-41 PSF	ASCE Table 30.7-2
Deck Fasteners (A < 10 SF)	-27 PSF	-45 PSF	-68 PSF	ASCE Table 30.7-2

COMPONENTS AND CLADDING WALLS

A = 100 SF	Zone 4		CODE REFERENCE
	21/-23 PSF	20/-26 PSF	
A = 50 SF	22/-24 PSF	22/-28 PSF	ASCE Table 30.7-2
A = 10 SF	25/-27 PSF	25/-33 PSF	ASCE Table 30.7-2

Building design displacements

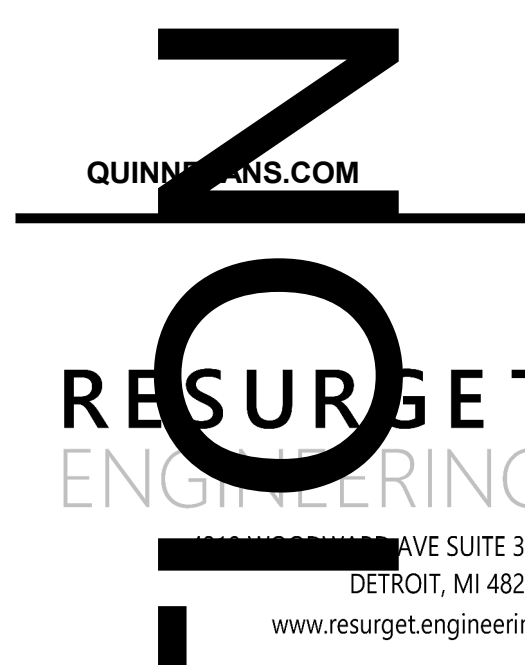
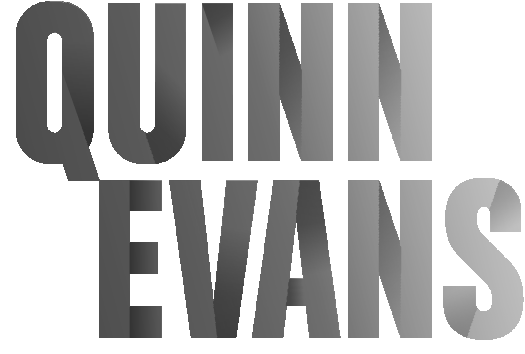
Wind drift at 10 year Design Wind Speed = h/400

SEISMIC LOADS

Seismic Importance Factor	Ie = 1.25	CODE REFERENCE
Short Period Spectral Response Acceleration	SS = 0.1022 g	ASCE Section 11.4.1
1.0 sec. Period Spectral Response Acceleration	S1 = 0.0457 g	ASCE Section 11.4.1
Site Class	D	ASCE Section 11.4.2
Design Short Spectral Response Acceleration	SDS = 0.11 g	ASCE Section 11.4.4
Design Short Period Spectral Response Acceleration	SD1 = 0.073 g	ASCE Section 11.4.4
Seismic Design Category	B	ASCE Section 11.6
Seismic Force Resisting System	Ordinary Plain Masonry Shear Walls	ASCE Table 12.2-1
Seismic Response Coefficient	CS = 0.098	ASCE Section 12.8.1.1
Response Modification Factor	R = 1.5	ASCE Table 12.2-1
Analysis Procedure	Equivalent Lateral Force	ASCE Section 12.8
Building design displacements	Seismic Inelastic Story Drift (Delta m) = 2.0%	

SUPERIMPOSED DEAD LOAD

Typical Roof	5 PSF (MEP)
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1000 W. WASHINGTON AVENUE SUITE 306
 DETROIT, MI 48201
 www.resurgetengineering.com

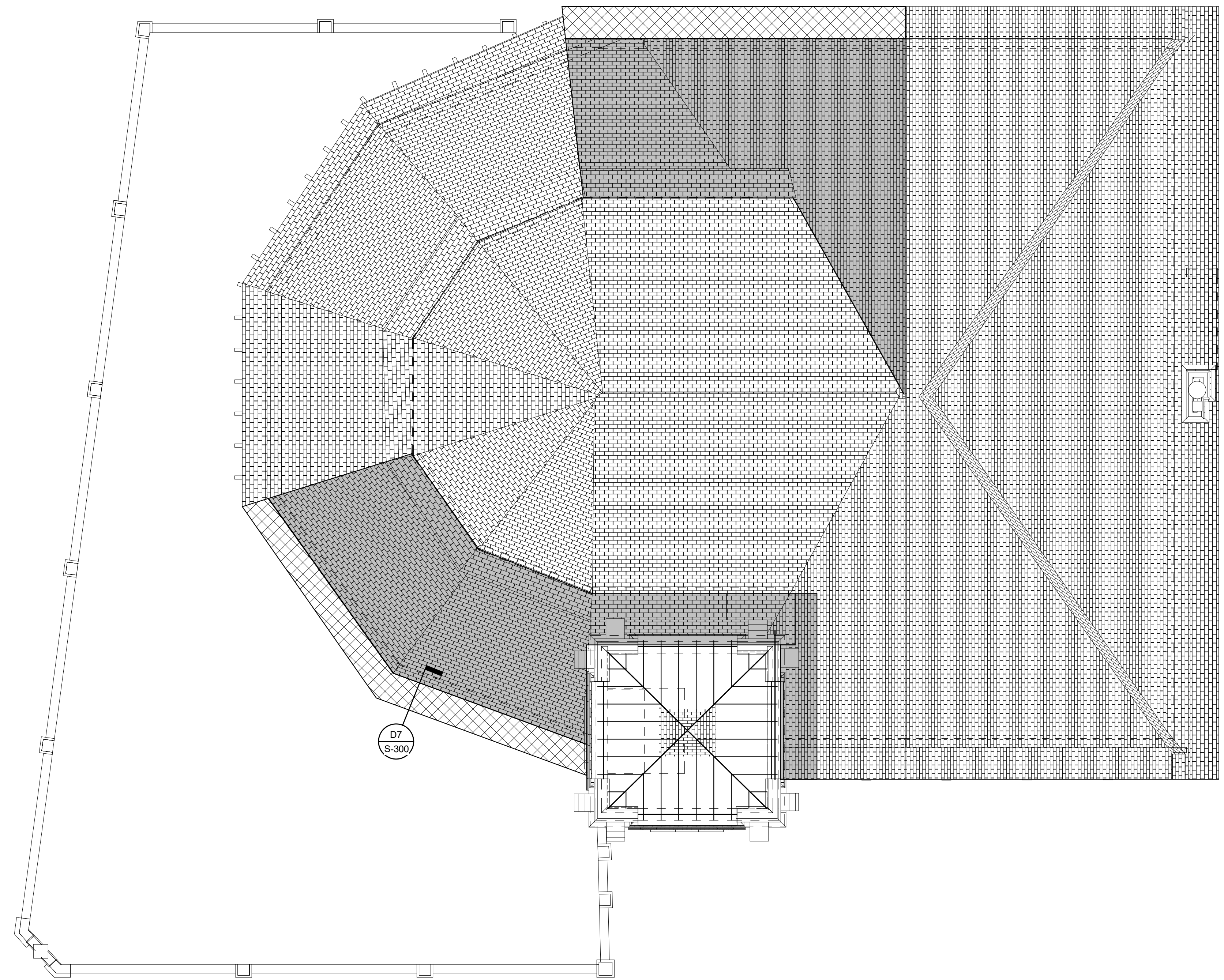
NOT FOR CONSTRUCTION
 KING SOMON BAPTIST CHURCH
 ROOF REPLACEMENT
 6125 FOURTEENTH STREET
 No. Date Description
 PROJECT MANAGER: MS
 50% CD SET
 1/24/2022
 GENERAL STRUCTURAL NOTES
S-001

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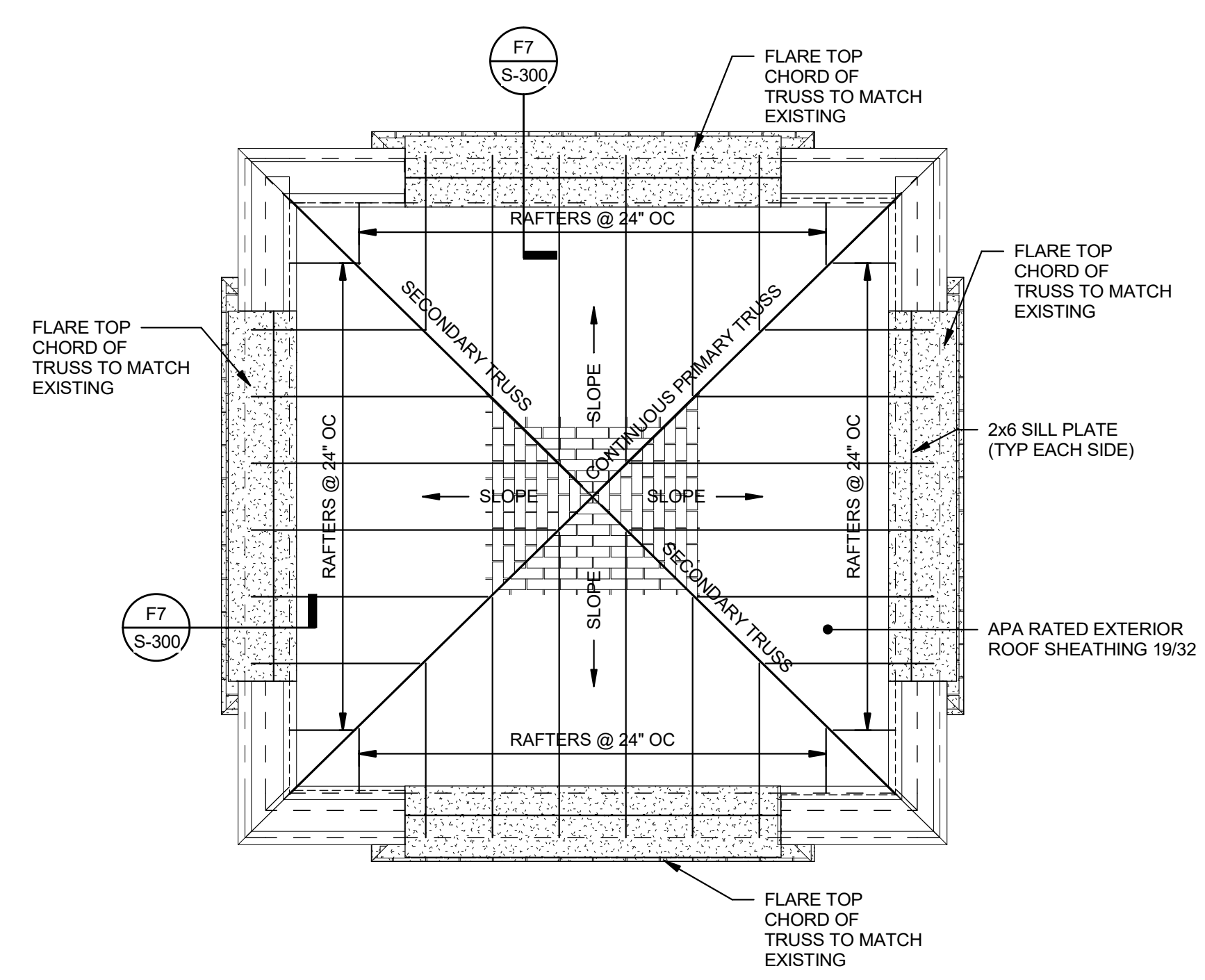
STRUCTURAL ITEMS REQUIRED FOR ROOFING INTEGRITY AND SAFETY			
HATCH AREA	DESCRIPTION	UNITS	COMMENT
	EXISTING ROOF FLARED OVERHANG WITH DECORATIVE WOOD BRACKETS CANTILEVERING OUT FROM BEARING WALL. REMOVE ROOF SHEATHING AND BRACKETS. MEASURE AND PRESERVE BRACKET GEOMETRY AND DETAIL FOR FUTURE REPLICATION.	300SF	OVERHANG NOT REQUIRED FOR ROOF ENCLOSURE.
	SLOPING LOWER ROOF AREA IN VERY POOR CONDITION. ASSUME REPLACEMENT OF 100% OF SHEATHING AND 60% REPLACEMENT OR REINFORCING OF SUPPORT RAFTERS.	SHEATHING: 1600SF RAFTERS: 300LF	EXTREME CARE NEEDED DURING CONSTRUCTION. REMOVAL OF ROOFING NEEDS TO BE CARRIED OUT FROM SAFE PLATFORM OR LIFT.

NOTES:

1. REPLACE EXISTING TONGUE AND GROOVE ROOF SHEATHING WITH 3/4" EXTERIOR RATED PLYWOOD SPANNING TO 2X6@16" OC. 2X6 SPANNING TO RAFTERS AT APPROXIMATELY 4'-0" OC.
2. REPLACE BADLY DAMAGED RAFTERS SPANNING UP TO 16 FT WITH (3)2X12.
3. REINFORCED MODERATELY DAMAGED RAFTERS WITH (2)2X12.



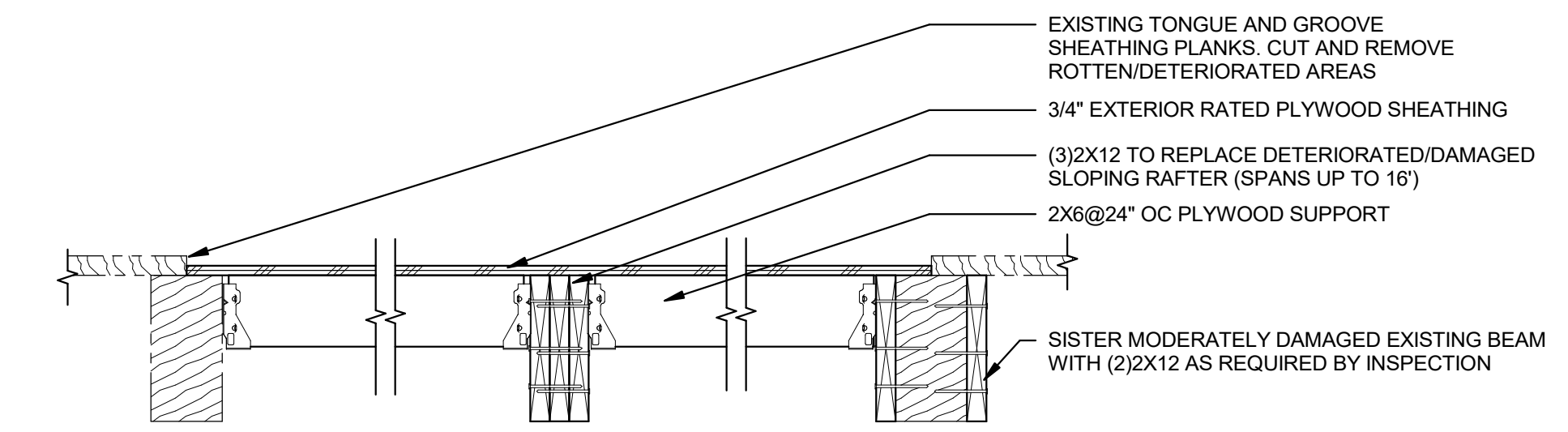
HEPTAGONAL AND CENTRAL GABLE ROOF FRAMING PLAN
SCALE: 1/8" = 1'-0"
NORTH



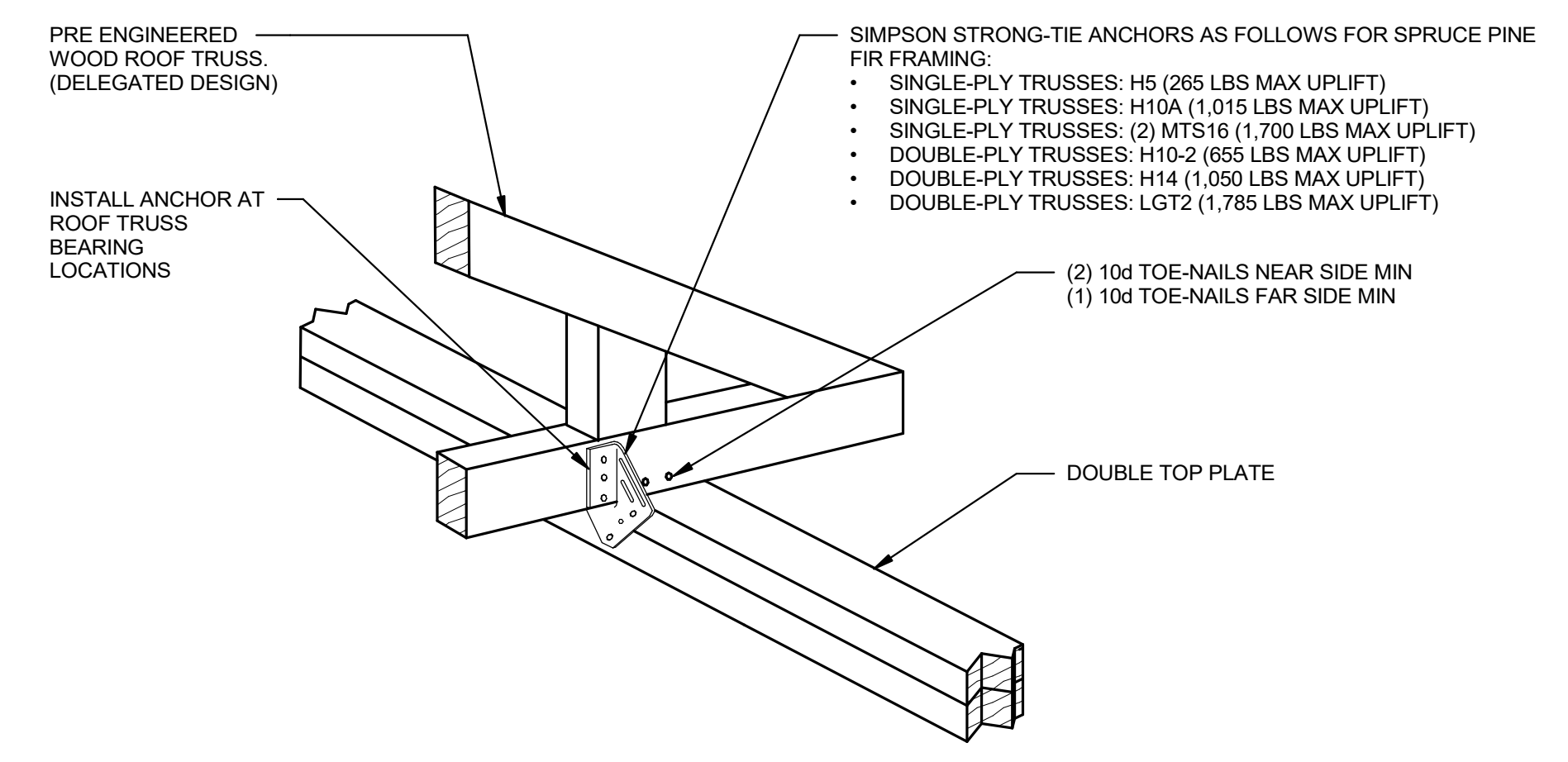
BELL TOWER ROOF FRAMING PLAN
SCALE: 1/4" = 1'-0"
NORTH

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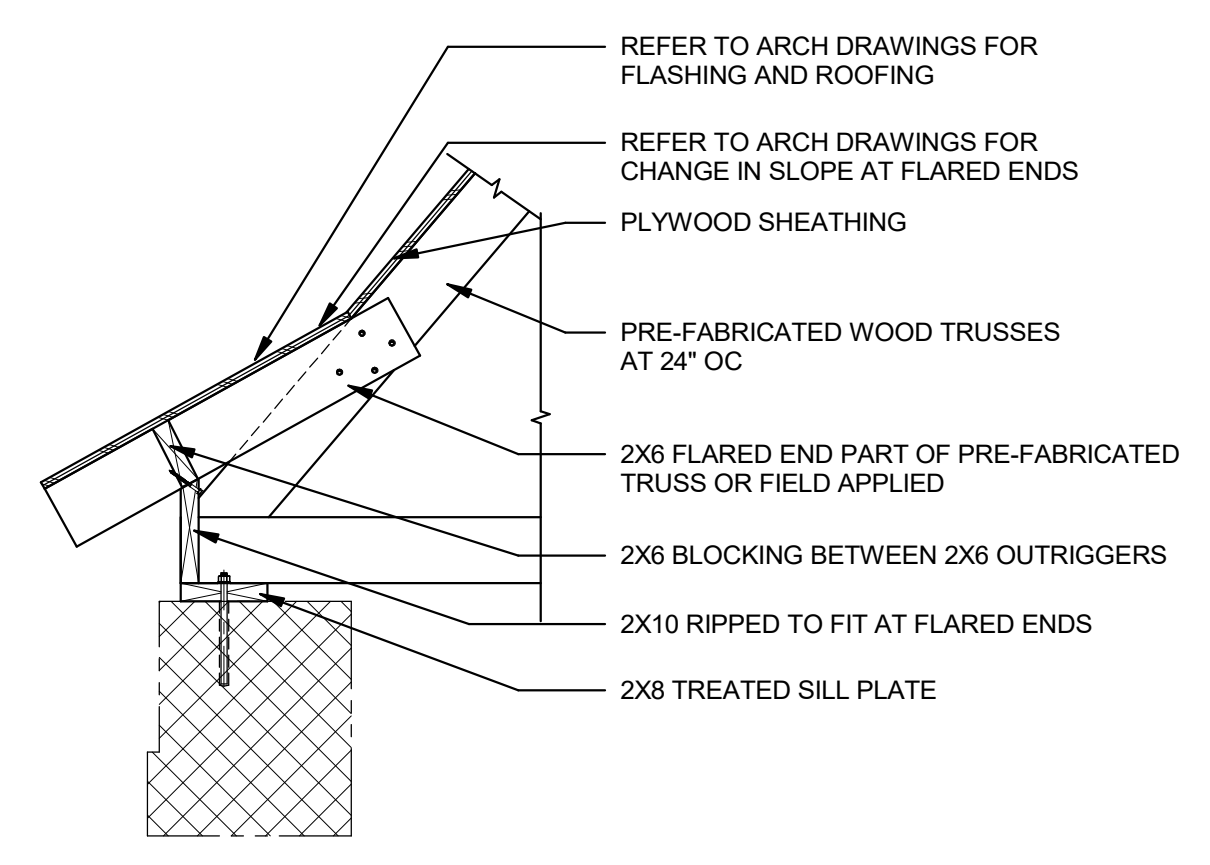


L10 WOOD REPAIR DETAIL
SCALE: 1" = 1'-0"

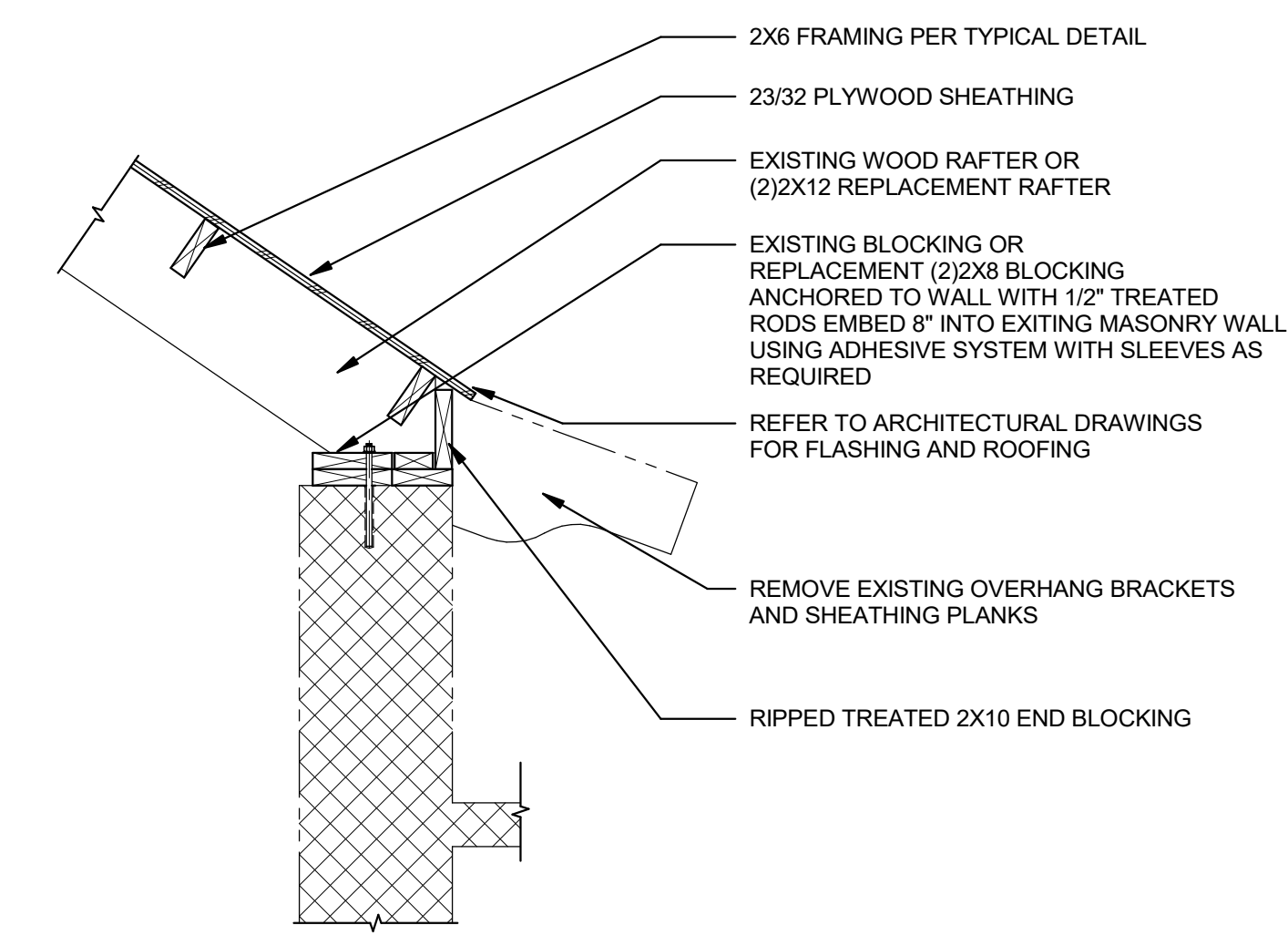


- NOTES:
1. MAXIMUM WALL STUD SPACING 16" OC
 2. SELECT TRUSS ANCHOR BASED ON APPROVED TRUSS SHOP DRAWINGS WITH TRUSS REACTIONS.
 3. TRUSS MANUFACTURER OR CONTRACTOR SHALL REQUEST FOR ADDITIONAL HOLDOWN ANCHORS AT TRUSS GIRDERS, MULTI-PLY TRUSSES, JACK TRUSSES AND HIP/VALLEY TRUSSES.
 4. MULTIPLE ANCHORS MAY BE USED (PER SIMPSON STRONG-TIE RECOMMENDED DETAILS) TO ANCHOR HIGHER UPLIFT VALUES.
 5. DETAIL APPLIES TO INTERIOR AND EXTERIOR BEARING WALLS.

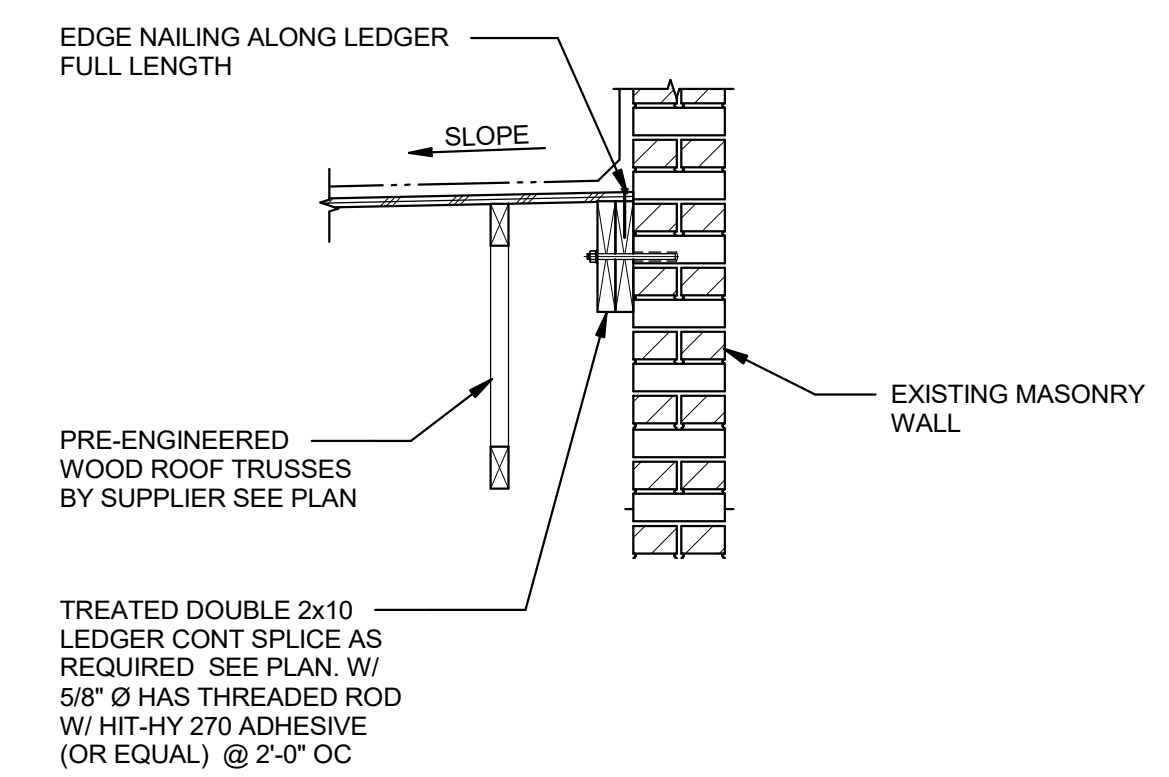
F10 TYPICAL ROOF TRUSS CONNECTION
SCALE: 3/4" = 1'-0"



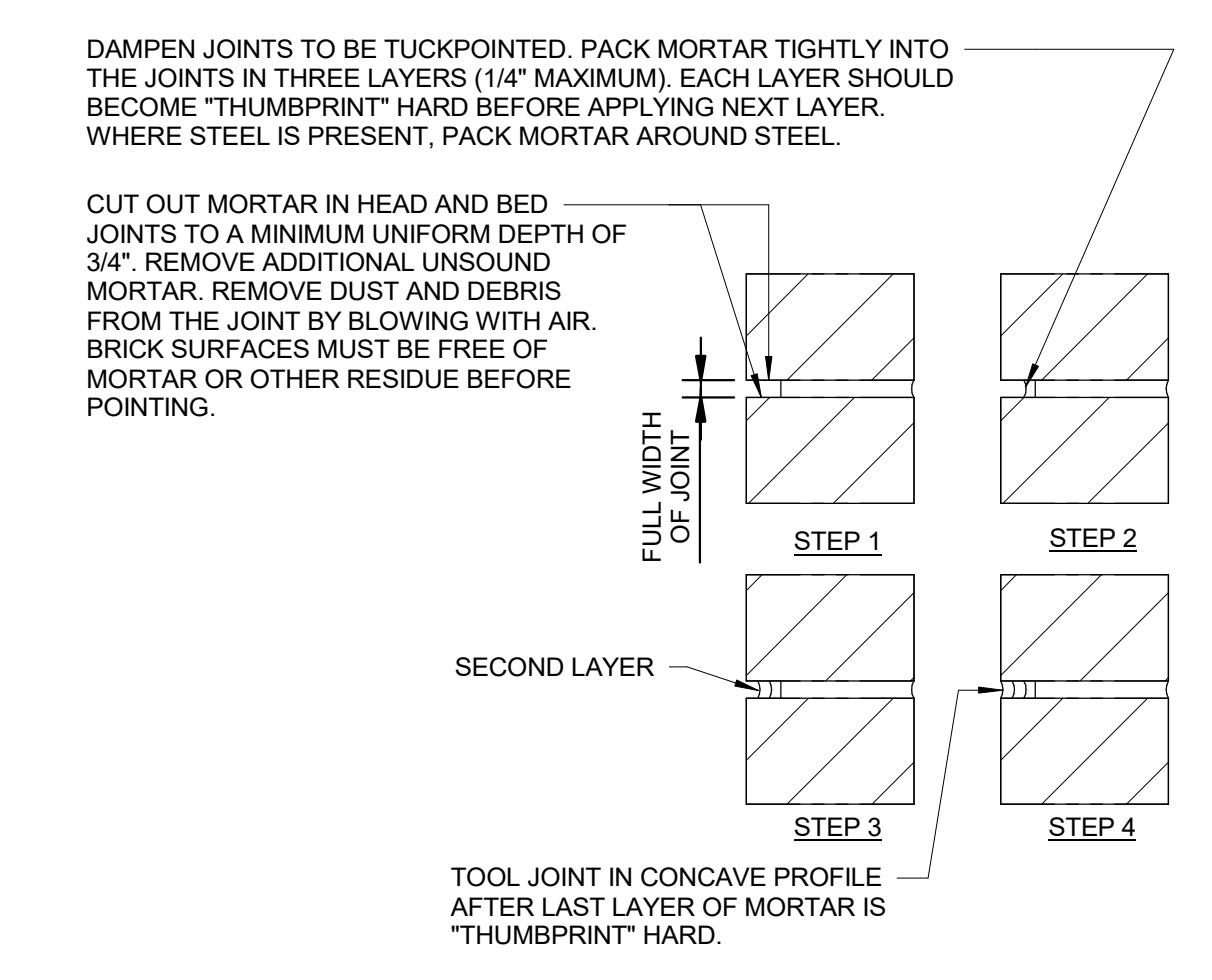
F7 FLARED TRUSS SUPPORT
SCALE: 3/4" = 1'-0"



D7 EAVE OVERHANG DETAIL
SCALE: 3/4" = 1'-0"



D10 ROOF TRUSSES PARALLEL TO MASONRY WALL
SCALE: 3/4" = 1'-0"



A10 MASONRY TUCK POINTING DETAIL
SCALE: 3/4" = 1'-0"

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KING SALOMON BAPTIST CHURCH

ROOF REPLACEMENT

6125 FOURTEENTH STREET

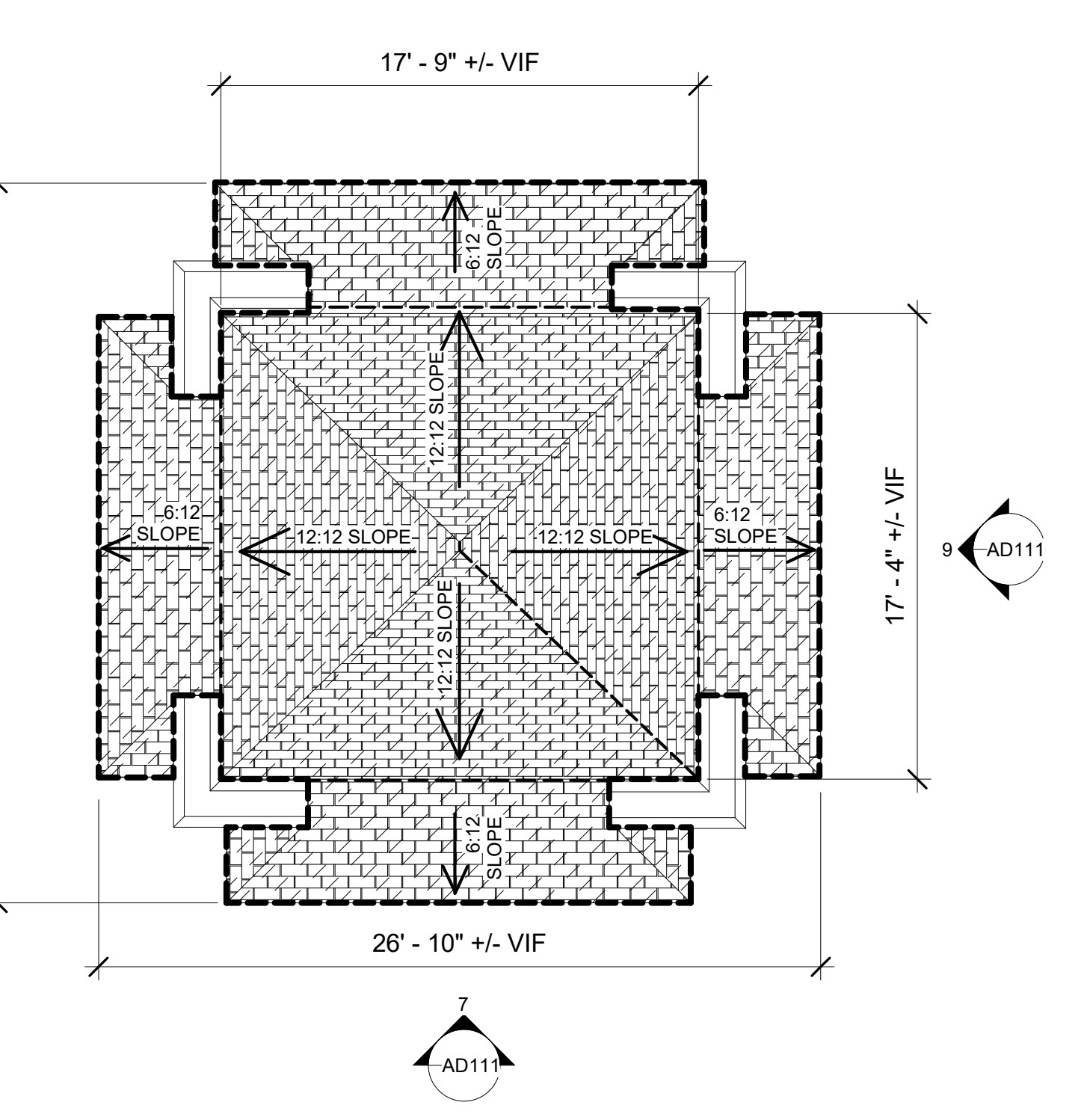
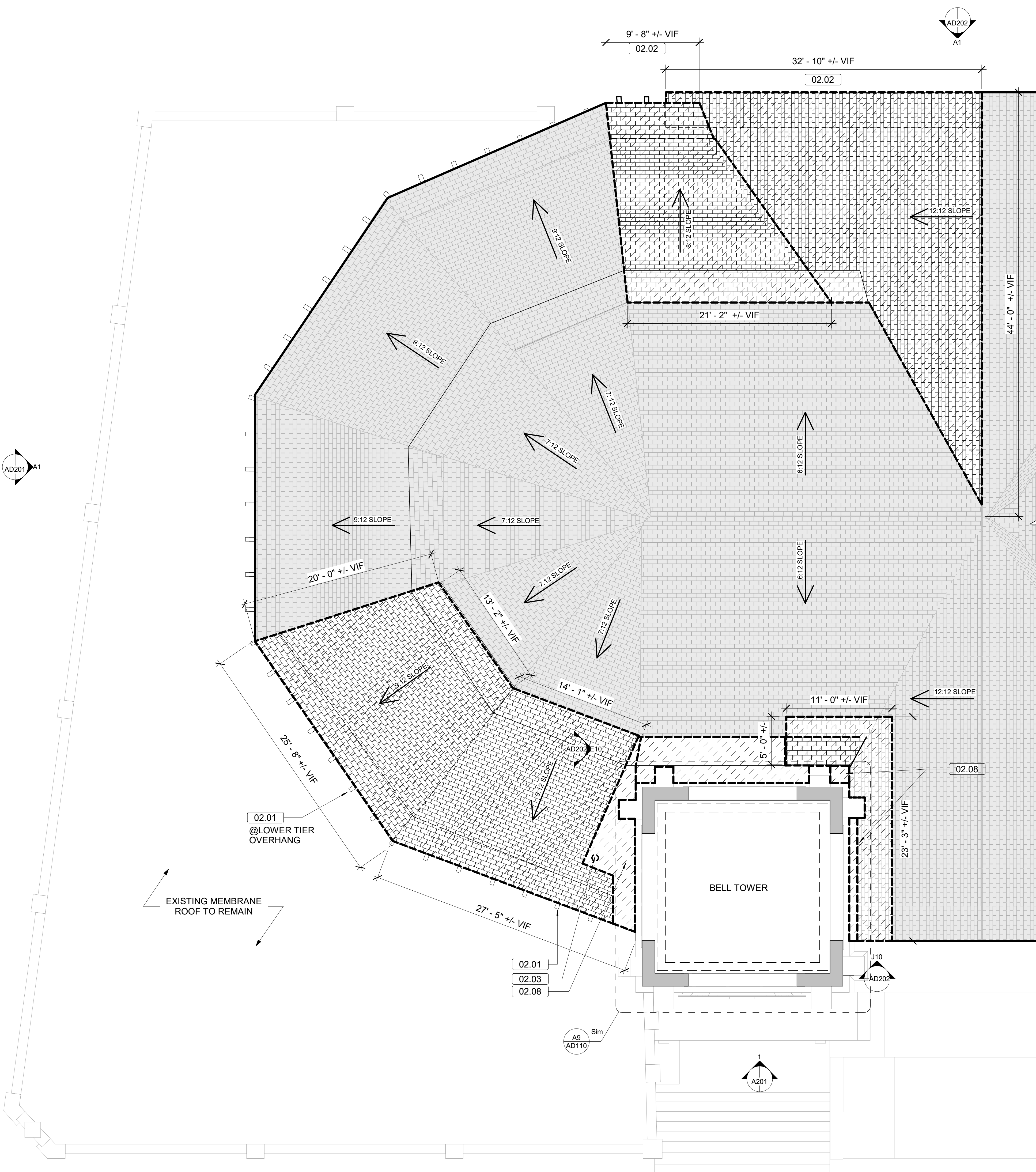
No.	Date	Description
PROJECT MANAGER	BY	
MS	SM	

50% CD SET
1/24/2022

SECTIONS AND DETAILS

S-300

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 PLOTTING DATE & TIME: 1/24/2022 3:17:20 PM



GENERAL DEMO NOTES

D 1	PROTECT FROM WEATHER ANY OPENINGS TO THE INTERIOR IN THE WORK AREA, EITHER EXISTING OR CREATED DURING DEMOLITION
D 2	THE CONTRACTOR SHALL PROVIDE SHORING, BRACING, OR OTHER TEMPORARY SUPPORT TO MAINTAIN THE STRUCTURAL INTEGRITY OF CONSTRUCTION TO REMAIN SUPPORTED BY WALLS, COLUMNS, BEAMS OR OTHER ITEMS TO BE REMOVED.
D 3	CONTRACTOR TO PERFORM SURVEY AND ANALYSIS OF EXISTING BUILDING PRIOR TO COMMENCING WITH DEMOLITION OPERATIONS. DO NOT REMOVE CONSTRUCTION IF THE STRUCTURAL INTEGRITY OF THE BUILDING MAY BE COMPROMISED UNTIL APPROPRIATE TEMPORARY SUPPORTS ARE IN PLACE. DESIGN OF SHORING IS THE RESPONSIBILITY OF THE CONTRACTOR.
D 4	CONTRACTOR SHALL FIELD VERIFY ALL EXISTING CONDITIONS AND DIMENSION PRIOR TO BEGINNING WORK AND NOTIFY THE ARCHITECT OF ANY DISCREPANCIES PRIOR TO BEGINNING WORK.
D 5	ALL DIMENSIONS ARE BASED ON LIMITED FIELD VERIFICATION. CONTRACTOR TO VERIFY EXISTING CONDITIONS AND SCOPE OF DEMOLITION WORK WITH REQUIREMENTS FOR NEW CONSTRUCTION
D 6	PROTECT EXISTING CONSTRUCTION TO REMAIN.
D 7	PRESENCE OF HAZARDOUS MATERIALS IS UNKNOWN. CONTRACTOR IS RESPONSIBLE FOR PROPER REMOVAL AND DISPOSAL OF ALL REMOVED MATERIALS
D 8	SALVAGE 12' MIN. OR ENTIRE PIECE OF EACH DISTINCT PROFILED TRIM PIECE BEING REMOVED - FOR OWNER RECORD

KEYNOTES

KEY VALUE	TEXT
02.01	REMOVE DECORATIVE WOOD RAFTER TAILS FROM OVERHANGS OF ROOF AREA IDENTIFIED FOR REMOVAL - REFER TO STRUCTURAL FOR FRAMING SCOPE
02.02	REMOVE DECORATIVE WOOD BRACKETS UNDER OVERHANG OF ROOF AREA IDENTIFIED FOR REMOVAL - REFER TO STRUCTURAL FOR FRAMING SCOPE
02.03	REMOVE VENT STACK AND FLASHING
02.08	REMOVE MEMBRANE OVER-ROOFING AND FLASHING (AROUND BASE OF BELL TOWER, ALL THREE SIDES). REMOVE MEMBRANE BURIED LAYERS OF ROOFING, DECKING, AND FLASHINGS COMPLETE - REFER TO STRUCTURAL RE: FRAMING

DEMOLITION LEGEND

	EXISTING CONSTRUCTION TO REMAIN
	ITEM/ CONSTRUCTION TO BE REMOVED, SALVAGED OR REINSTALLED - AS NOTED
	EXISTING ASPHALT SHINGLE ROOF TO REMAIN
	EXTENT OF ROOF REMOVAL. REMOVE ASPHALT SHINGLES, UNDERLAYMENT, SHEATHING OVERHANG, DECKING, EDGING/TRIM AND FLASHINGS COMPLETE - REFER TO STRUCTURAL RE: FRAMING



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KING SOLOMON BAPTIST CHURCH

ROOF REPLACEMENT - 50% CD SET

6125 FOURTEENTH STREET DETROIT, MI

No.	Date	Description

PROJECT MANAGER: A. CECIL
 DATE: 1/24/2022
 DRAWN BY: S. RUTLAND

QEA No.42134130

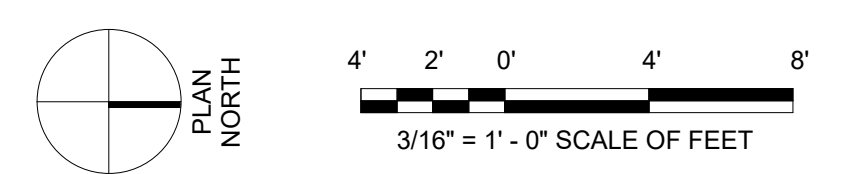
50% CD SET
 1/24/2022

DEMOLITION ROOF PLANS

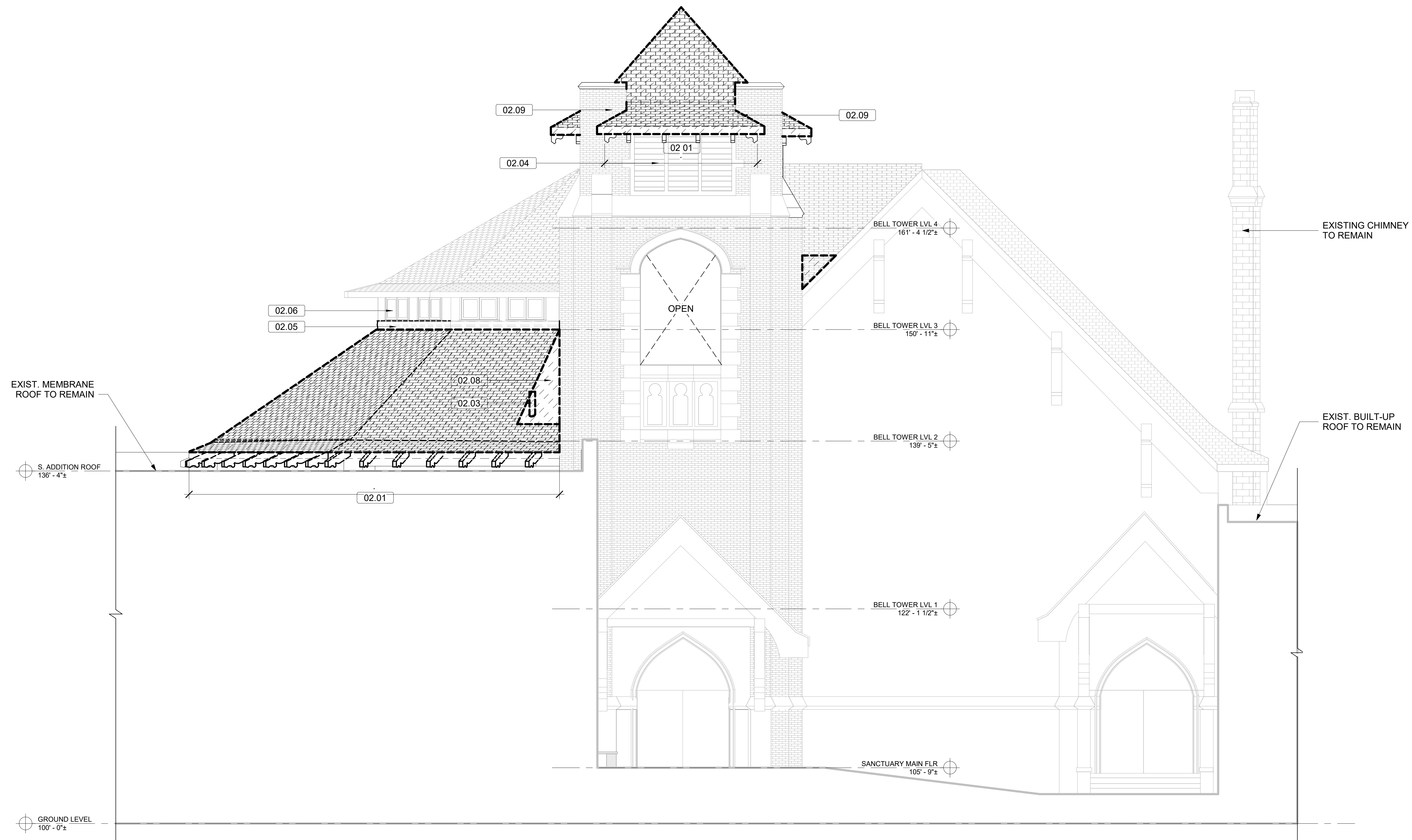
AD110

A1 HEPTAGON AND CENTRAL GABLE ROOF DEMOLITION PLAN
 3/16" = 1'-0" REFERRED FROM: AD110

A9 BELL TOWER ROOF DEMOLITION PLAN
 3/16" = 1'-0" REFERRED FROM: AD110

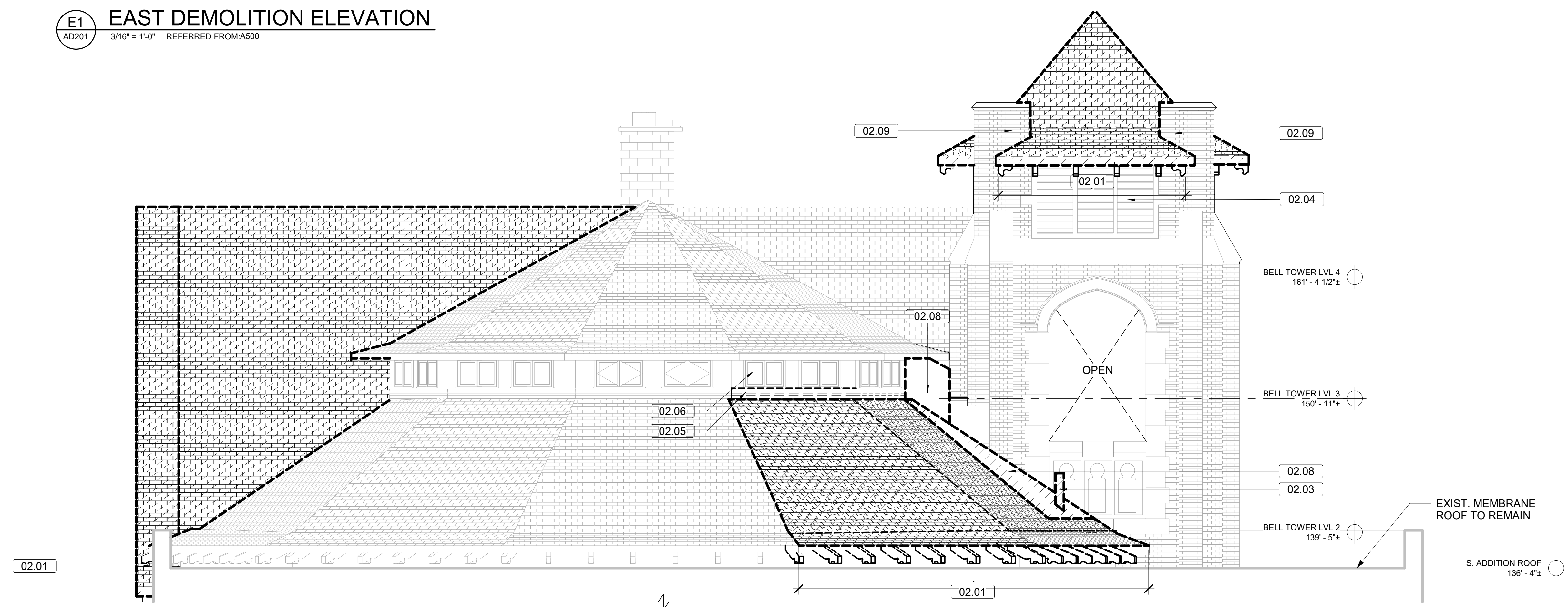


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E1 EAST DEMOLITION ELEVATION

AD201 3/16" = 1'-0" REFERRED FROM:A500



A1 SOUTH DEMOLITION ELEVATION

AD201 3/16" = 1'-0" REFERRED FROM:A500

GENERAL DEMO NOTES

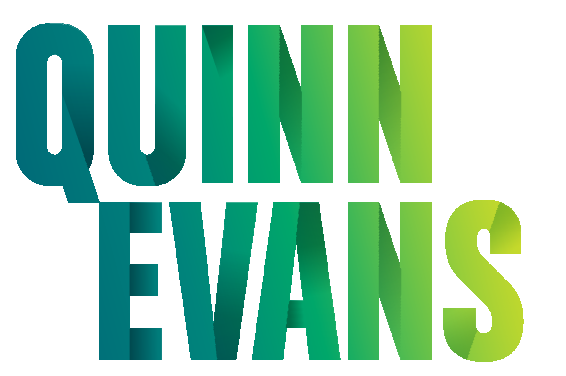
D 1	PROTECT FROM WEATHER ANY OPENINGS TO THE INTERIOR IN THE WORK AREA, EITHER EXISTING OR CREATED DURING DEMOLITION
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D 7	PRESENCE OF HAZARDOUS MATERIALS IS UNKNOWN. CONTRACTOR IS RESPONSIBLE FOR PROPER REMOVAL AND DISPOSAL OF ALL REMOVED MATERIALS
D 8	SALVAGE 12' MIN. OR ENTIRE PIECE OF EACH DISTINCT PROFILED TRIM PIECE BEING REMOVED - FOR OWNER RECORD

KEYNOTES

KEY VALUE	TEXT
02.01	REMOVE DECORATIVE WOOD RAFTER TAILS FROM OVERHANGS OF ROOF AREA IDENTIFIED FOR REMOVAL - REFER TO STRUCTURAL FOR FRAMING SCOPE
02.03	REMOVE VENT STACK AND FLASHING
02.04	EXISTING WOOD LOUVERS TO REMAIN
02.05	REMOVE METAL HEAD COUNTERFLASHING
02.06	COVER AND PROTECT GLASS IN EXISTING WINDOWS TO REMAIN IN WORK AREA
02.08	REMOVE MEMBRANE OVER-ROOFING AND FLASHING (AROUND BASE OF BELL TOWER, ALL THREE SIDES). REMOVE MEMBRANE BURIED LAYERS OF ROOFING, DECKING, AND FLASHINGS COMPLETE - REFER TO STRUCTURAL RE: FRAMING
02.09	REMOVE COPPER FLASHING AND ANY ASSOCIATED MASTIC OR SEALANTS. PREP AND REPOINT ABANDONED REGLET JOINTS IN MASONRY WITH MORTAR TO MATCH EXISTING

DEMOLITION LEGEND

	EXISTING CONSTRUCTION TO REMAIN
	ITEM/ CONSTRUCTION TO BE REMOVED, SALVAGED OR REINSTALLED - AS NOTED
	EXISTING ASPHALT SHINGLE ROOF TO REMAIN
	EXTENT OF ROOF REMOVAL- REMOVE ASPHALT SHINGLES, UNDERLAYMENT, SHEATHING OVERHANG, DECKING, EDGING TRIM, AND FLASHINGS COMPLETE - REFER TO STRUCTURAL RE: FRAMING



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KING SOLOMON BAPTIST CHURCH

ROOF REPLACEMENT - 50% CD SET

6125 FOURTEENTH STREET DETROIT, MI

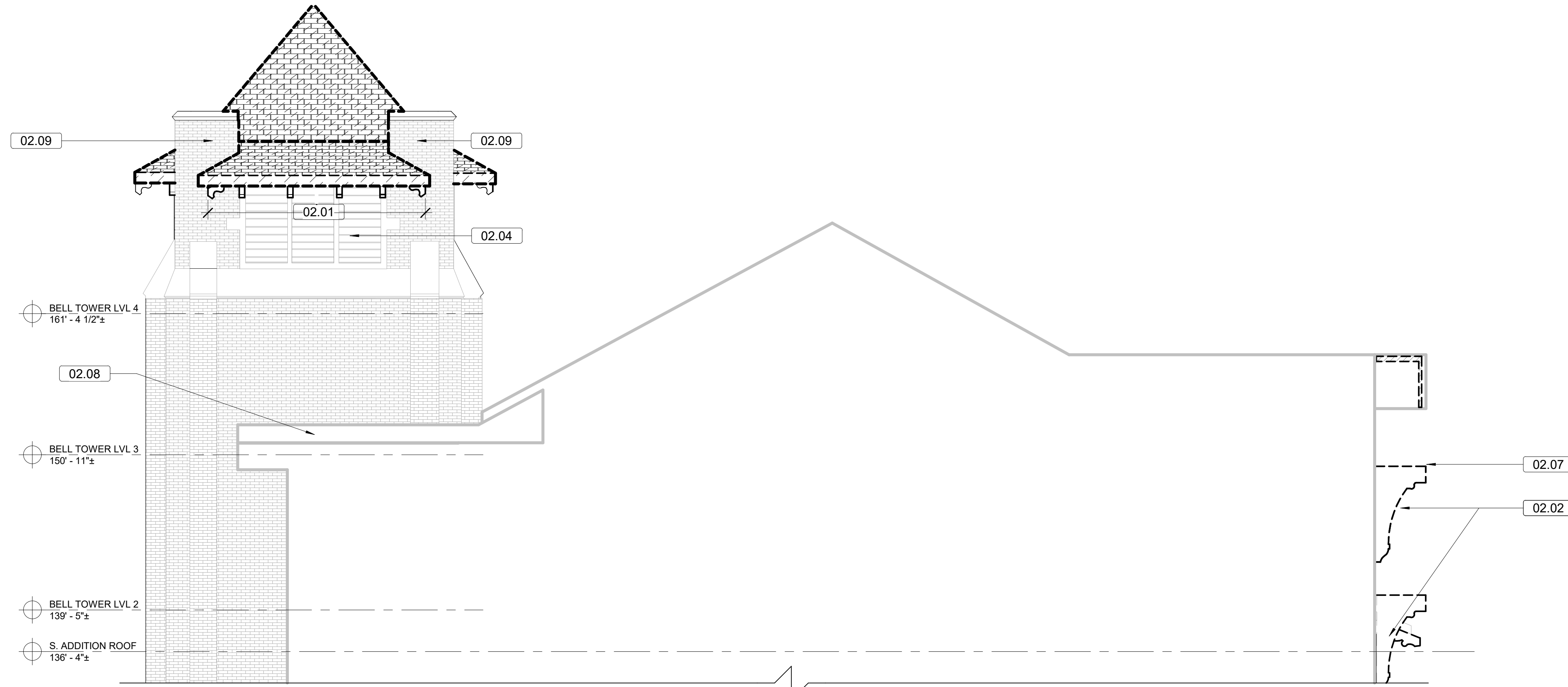
No.	Date	Description
PROJECT MANAGER:	DATE:	BY:
A. CECIL		S. RUTLAND

QEA No.42134130
 50% CD SET
 1/24/2022

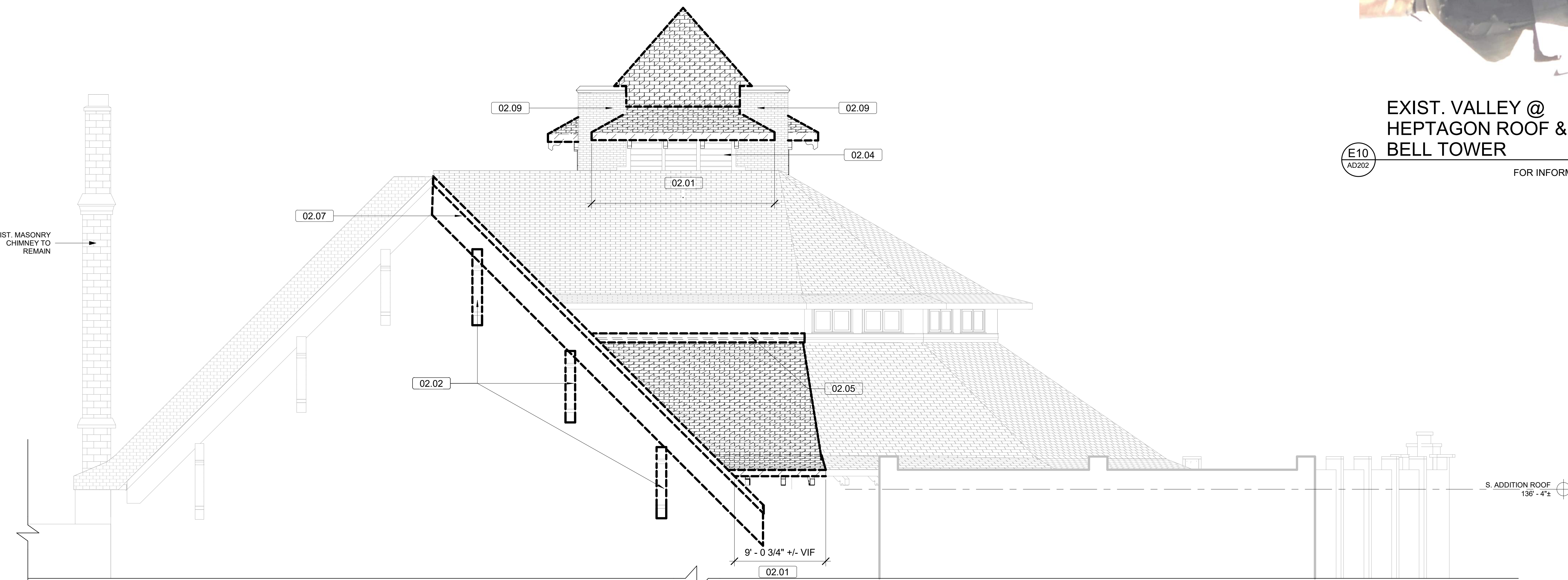
DEMOLITION ELEVATIONS

AD201

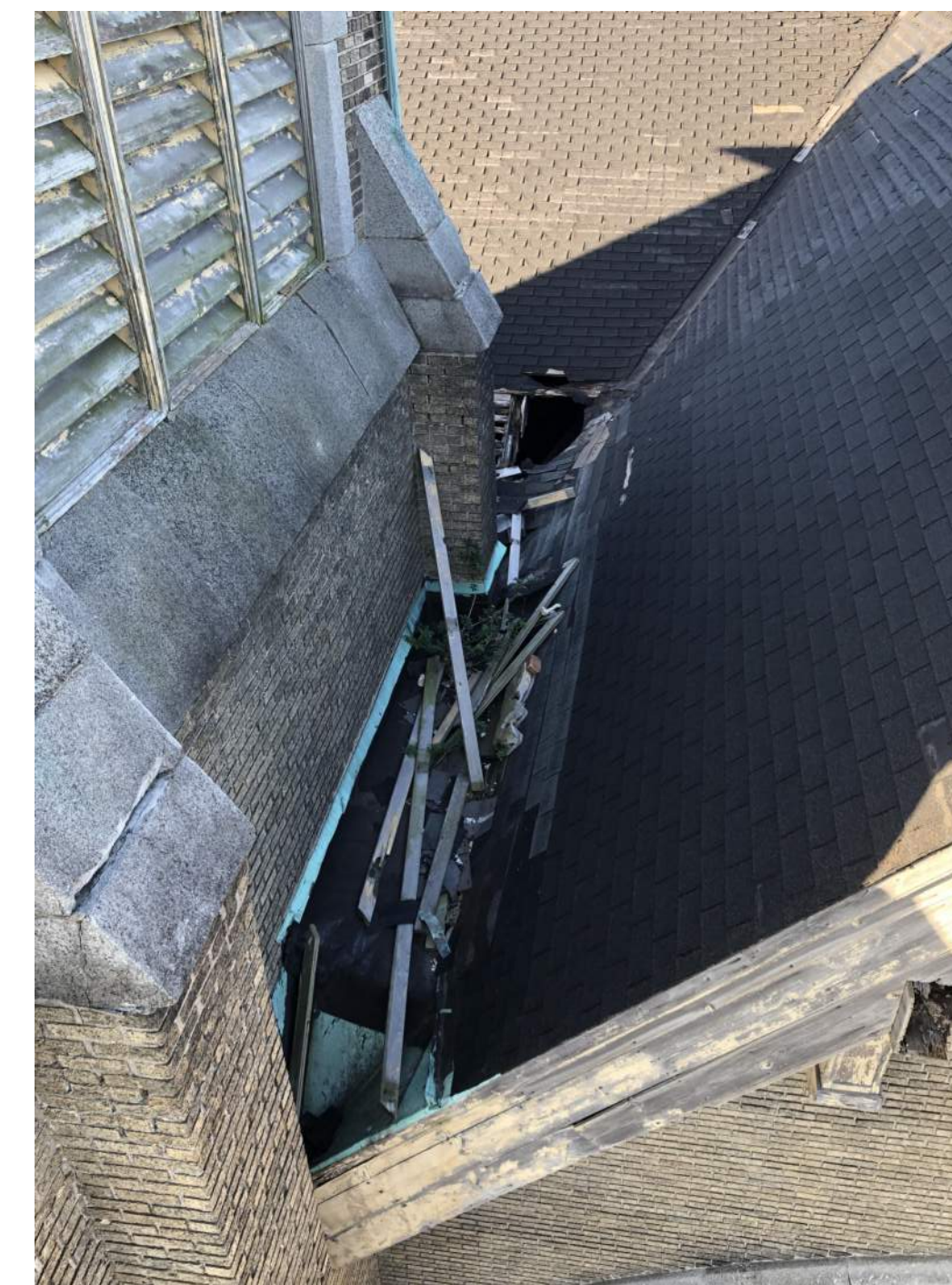
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F1
AD202
3/16" = 1'-0" REFERRED FROM: A500
NORTH DEMOLITION ELEVATION



A1
AD202
3/16" = 1'-0" REFERRED FROM: A500
WEST DEMOLITION ELEVATION



J10
AD202
EXIST. VALLEY @ GABLE ROOF & BELL TOWER

FOR INFORMATION ONLY



E10
AD202
EXIST. VALLEY @ HEPTAGON ROOF & BELL TOWER

FOR INFORMATION ONLY

GENERAL DEMO NOTES

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- D 8 SALVAGE 12" MIN. OR ENTIRE PIECE OF EACH DISTINCT PROFILED TRIM PIECE BEING REMOVED - FOR OWNER RECORD

KEYNOTES

KEY VALUE	TEXT
02.01	REMOVE DECORATIVE WOOD RAFTER TAILS FROM OVERHANGS OF ROOF AREA IDENTIFIED FOR REMOVAL - REFER TO STRUCTURAL FOR FRAMING SCOPE
02.02	REMOVE DECORATIVE WOOD BRACKETS UNDER OVERHANG OF ROOF AREA IDENTIFIED FOR REMOVAL - REFER TO STRUCTURAL FOR FRAMING SCOPE
02.04	EXISTING WOOD LOUVERS TO REMAIN
02.05	REMOVE METAL HEAD COUNTERFLASHING
02.07	REMOVE WOOD FASCIA & TRIM ASSEMBLY ALONG EDGE OF ROOF AREA DESIGNATED FOR REMOVAL
02.08	REMOVE MEMBRANE OVER-ROOFING AND FLASHING (AROUND BASE OF BELL TOWER, ALL THREE SIDES). REMOVE MEMBRANE BURIED LAYERS OF ROOFING, DECKING, AND FLASHINGS COMPLETE - REFER TO STRUCTURAL RE: FRAMING
02.09	REMOVE COPPER FLASHING AND ANY ASSOCIATED MASTIC OR SEALANTS. PREP AND REPOINT ABANDONED REGLET JOINTS IN MASONRY WITH MORTAR TO MATCH EXISTING

DEMOLITION LEGEND

- EXISTING CONSTRUCTION TO REMAIN
- ITEM: CONSTRUCTION TO BE REMOVED, SALVAGED OR REINSTALLED - AS NOTED
- EXISTING ASPHALT SHINGLE ROOF TO REMAIN
- EXTENT OF ROOF REMOVAL: REMOVE ASPHALT SHINGLES, UNDERLAYMENT, SHEATHING OVERHANG, DECKING, EDGING/TRIM, AND FLASHINGS COMPLETE - REFER TO STRUCTURAL RE: FRAMING



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ROOF REPLACEMENT - 50% CD SET

6125 FOURTEENTH STREET DETROIT, MI

No. Date Description

PROJECT MANAGER: A. CECIL
DESIGNED BY: S. RUTLAND

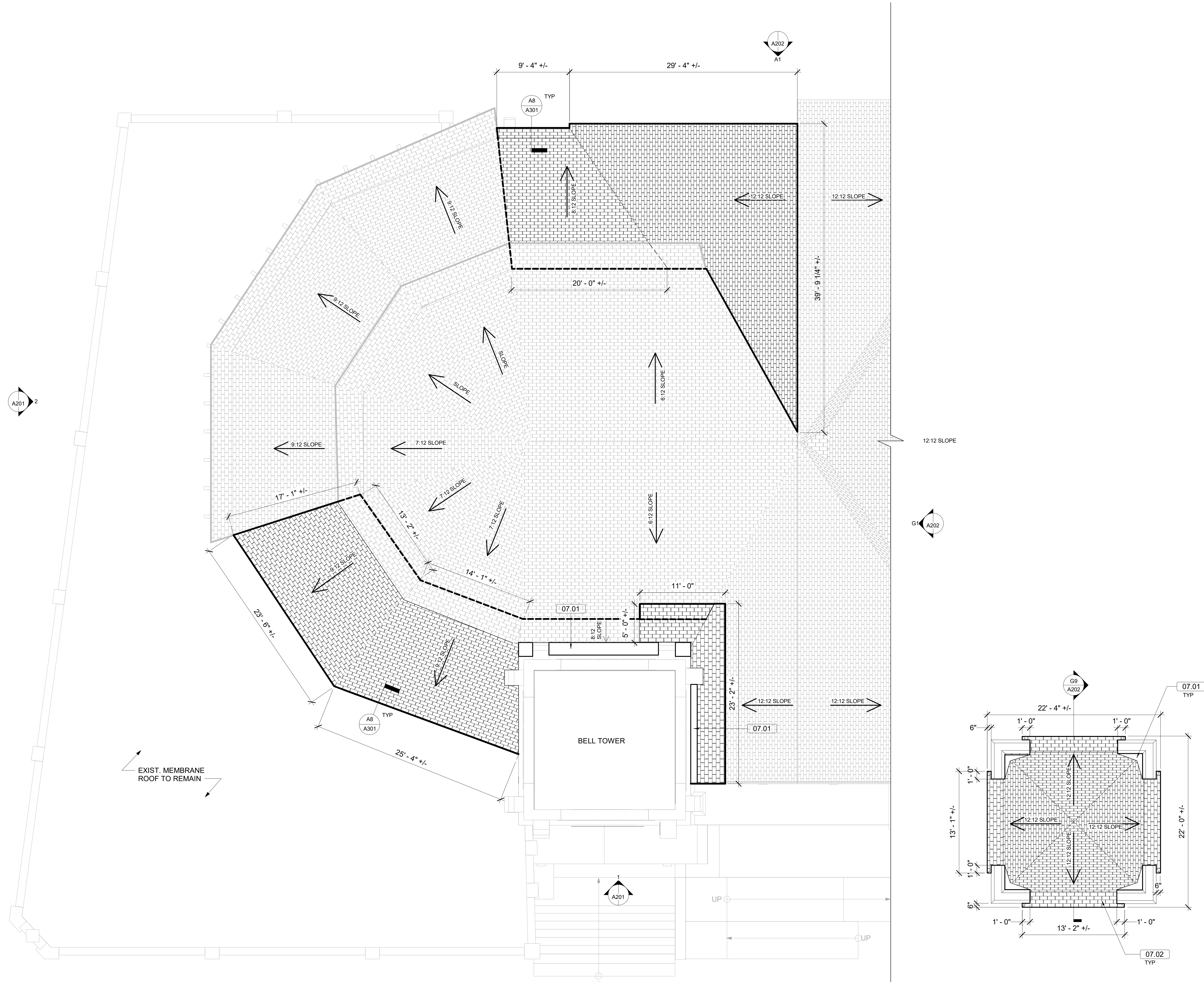
QEA No.42134130

50% CD SET
1/24/2022

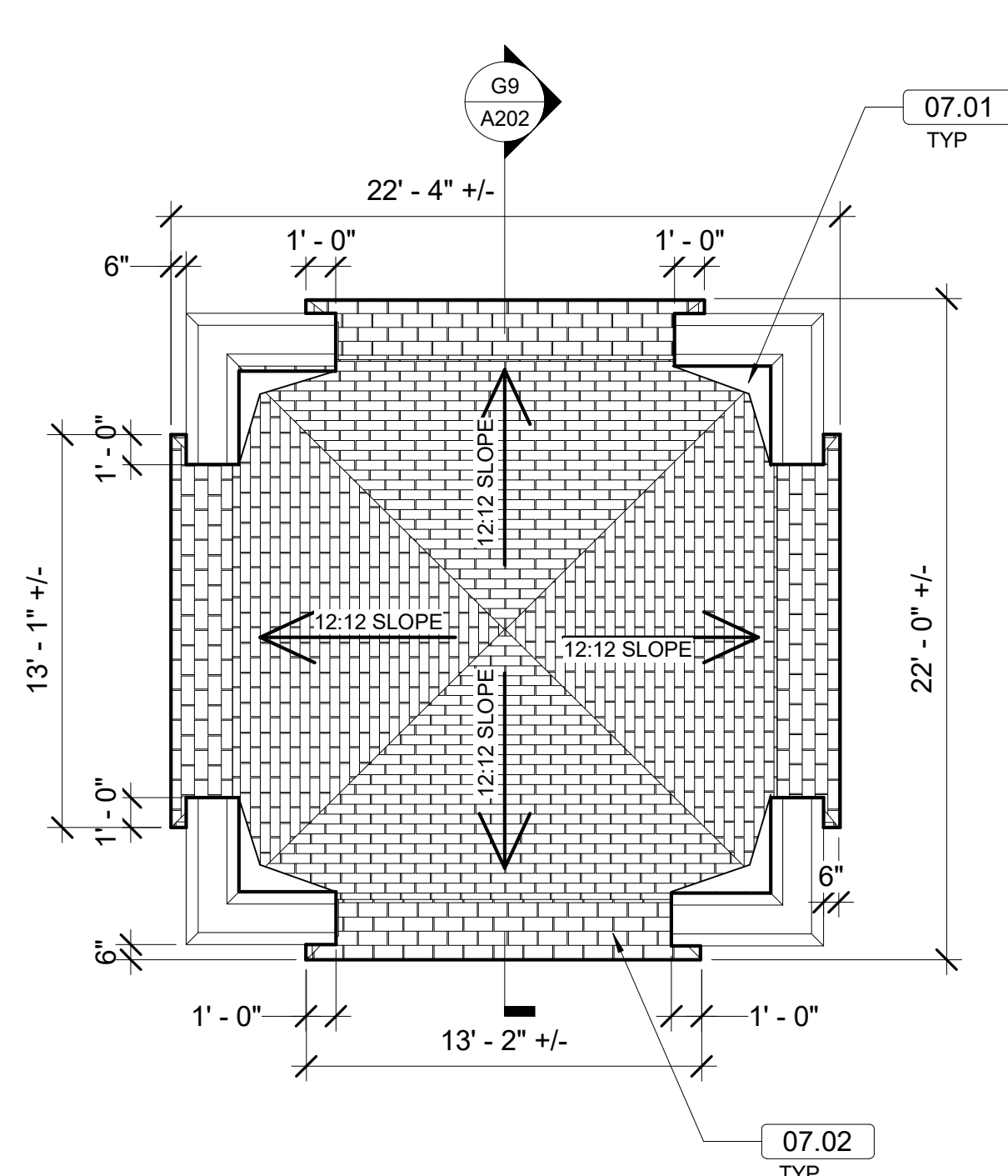
DEMOLITION ELEVATIONS

AD202

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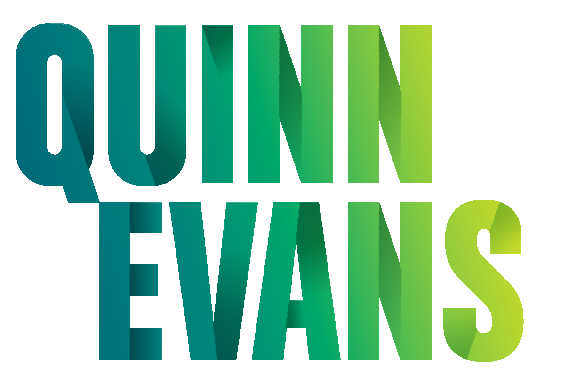


A1 HEPTAGON AND CENTRAL GABLE ROOF PLAN
 3/16" = 1'-0" REFERRED FROM:



A10 BELL TOWER ROOF PLAN
 3/16" = 1'-0"

GENERAL ROOF NOTES	
R 1	SLOPE OF NEW ROOFING TO MATCH EXISTING IN AREA OF REPLACEMENT, U.O.N.
R 2	NEW FLASHING TO BE PREFINISHED ALUM., U.O.N.
R 3	INTERWEAVE AND LAP NEW SHINGLES WITH EXISTING TO REMAIN SHINGLES AT INTERSECTION OF AREA OF WORK WITH AREAS NOT IN CONTRACT. RESECURE EXISTING SHINGLES IMPACTED BY EFFORTS.



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KEYNOTES	
KEY VALUE	TEXT
07.01	CONTINUOUS PREFINISHED ALUMINUM VALLEY AND CRICKET FLASHING OVER NEW PLYWOOD SHEATHING AND SUPPORT FRAMING, AS NEEDED
07.02	SLOPE OF SKIRT TO MATCH EXISTING, EA. SIDE

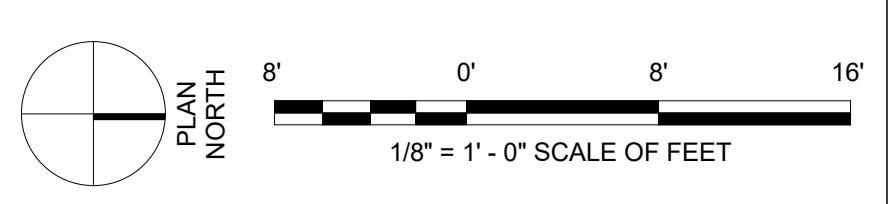
KING SOLOMON BAPTIST CHURCH

ROOF REPLACEMENT - 50% CD SET

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ROOF LEGEND

- EXISTING ASPHALT SHINGLE ROOF TO REMAIN
- EXTENT OF NEW ROOF - PROVIDE ARCHITECTURAL GRADE ASPHALT SHINGLES OVER WATERPROOFING UNDERLAYMENT, OVER NEW SHEATHING - REFER TO STRUCTURAL



NOT FOR CONSTRUCTION

No.	Date	Description

PROJECT MANAGER: A. CECIL
 DESIGNER: S. RUTLAND

QEA No. 42134130

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ROOF PLANS

A110

GENERAL ROOF NOTES

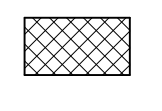
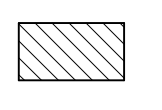
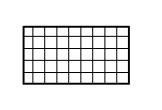



R 1	SLOPE OF NEW ROOFING TO MATCH EXISTING IN AREA OF REPLACEMENT, U.O.N.
R 2	NEW FLASHING TO BE PREFINISHED ALUM., U.O.N.
R 3	INTERWEAVE AND LAP NEW SHINGLES WITH EXISTING TO REMAIN SHINGLES AT INTERSECTION OF AREA OF WORK WITH AREAS NOT IN CONTRACT. RESECURE EXISTING SHINGLES IMPACTED BY EFFORTS.

KEYNOTES

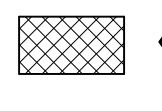

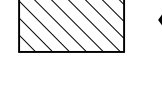
KEY VALUE	TEXT
07.01	CONTINUOUS PREFINISHED ALUMINUM VALLEY AND CRICKET FLASHING OVER NEW PLYWOOD SHEATHING AND SUPPORT FRAMING, AS NEEDED
07.03	PROVIDE CONTINUOUS PREFINISHED ALUMINUM COUNTERFLASHING - PROFILE TO MATCH EXIST.

MASONRY REPAIR LEGEND

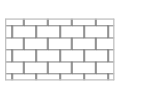
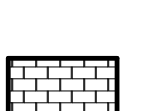
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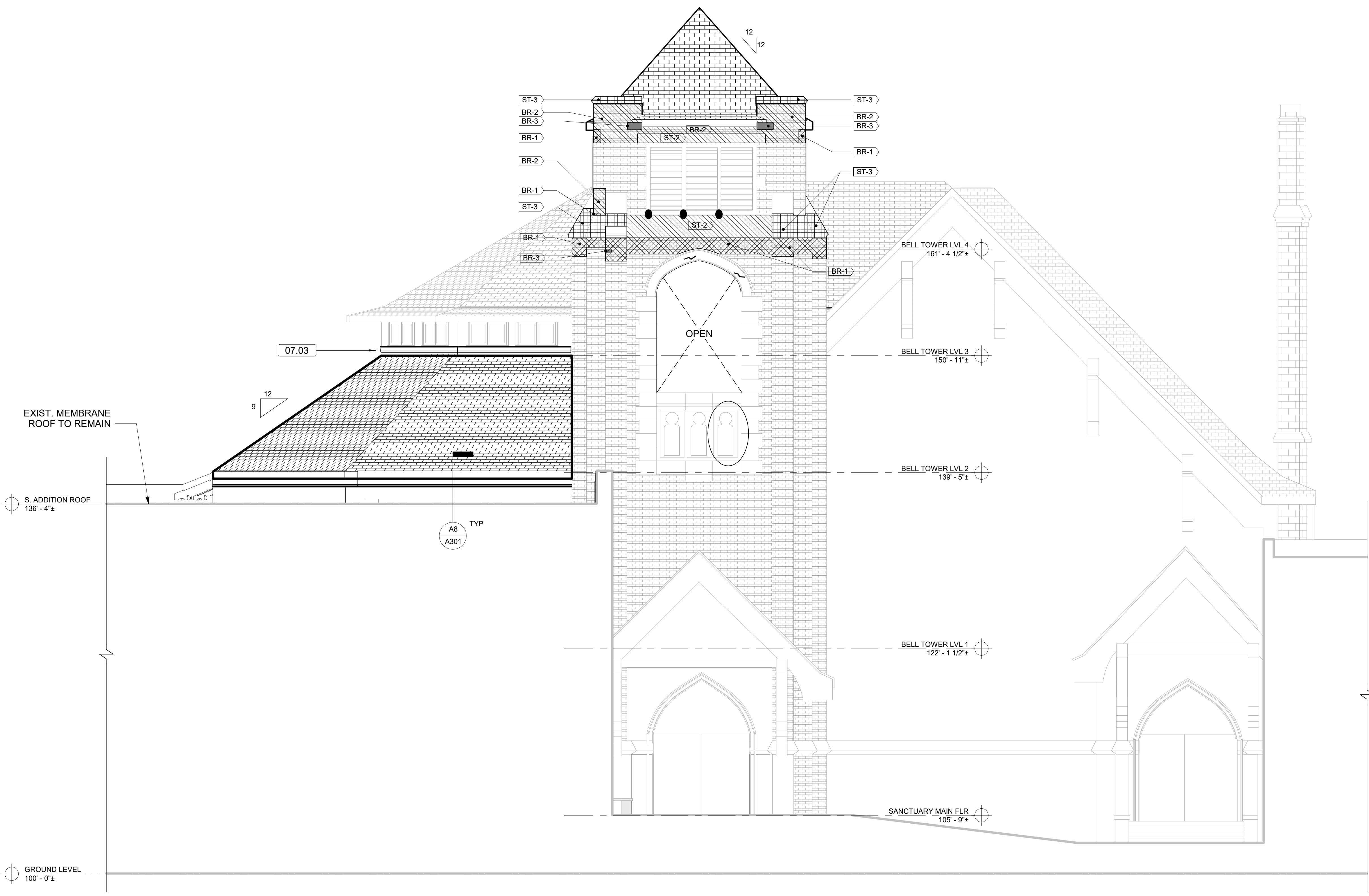
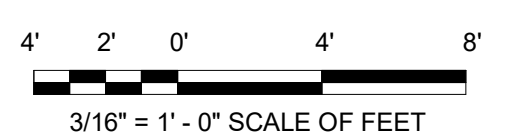
-  **ST-1** RESET: REMOVE & SALVAGE SOUND BUT DAMAGED UNITS. RESET IN MORTAR WITH NEW GALV. ANCHORS OR DOWELS
-  **ST-2** REPOINT: REMOVE LOOSE MORTAR & PREP JOINTS. REPOINT OPEN JOINTS WITH MORTAR TO MATCH EXISTING
-  **ST-3** REPLACE: REMOVE DETERIORATED UNITS. REPLACE WITH CAST STONE TO MATCH! RECREATE ORIGINAL IN PROFILE, COLOR, & TEXTURE
-  **ST-4** CRACK REPAIR: ROUTE CRACK & PREP JOINT, EPOXY CRACK REPAIR JOINT IN-SITU
-  **ST-5** SPALL: REMOVE REMAINING LOOSE MATERIAL FROM SURFACE. PREP SURFACE & PATCH REPAIR SPALL WITH FILL MATERIAL FLUSH WITH FACE OF EXISTING STONE TO REMAIN
-  **ST-6** SPALL: REMOVE DETACHED OR LOOSE STONE MATERIAL FROM SURFACE ONLY

BRICK

-  **BR-1** RECONSTRUCT: DISMANTLE, REPLACE OR DETERIORATED MASONRY. SALVAGE SOUND UNITS FOR REINSTALLATION & REPLACE BROKEN UNITS. RESET SALVAGED AND NEW UNITS IN MORTAR (WITH NEW TIES) TO REBUILD
-  **BR-2** REPOINT: REMOVE DETACHED OR LOOSE MORTAR & PREP JOINTS. REPOINT OPEN JOINTS WITH MORTAR MATCH EXISTING
-  **BR-3** REPLACE: REMOVE BROKEN OR MISSING UNITS & ASSOC. MORTAR. REPLACE WITH BRICKS TO MATCH EXISTING (SIZE, COLOR, TEXTURE) IN NEW MORTAR BED & JOINTS TO MATCH EXISTING

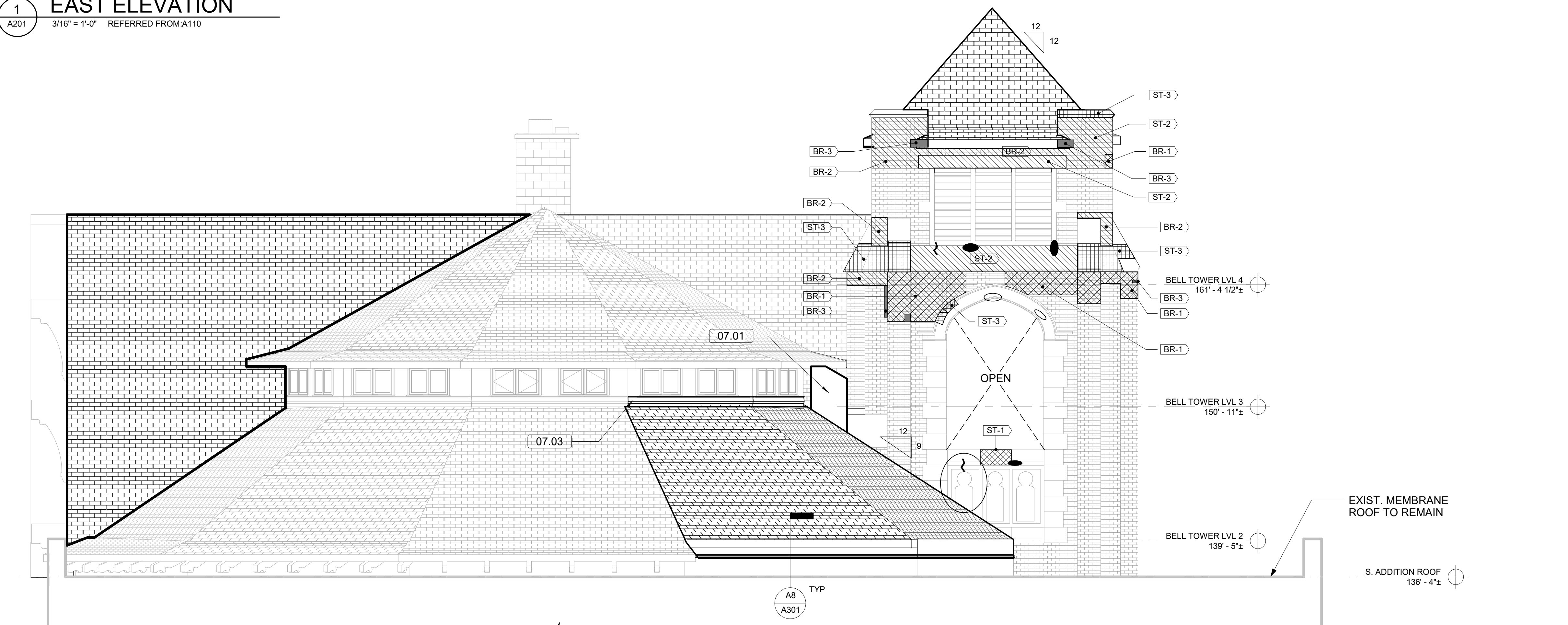
ROOF LEGEND

-  EXISTING ASPHALT SHINGLE ROOF TO REMAIN
-  EXTENT OF NEW ROOF: PROVIDE ARCHITECTURAL GRADE ASPHALT SHINGLES OVER WATERPROOFING UNDERLAYMENT, OVER NEW SHEATHING - REFER TO STRUCTURAL

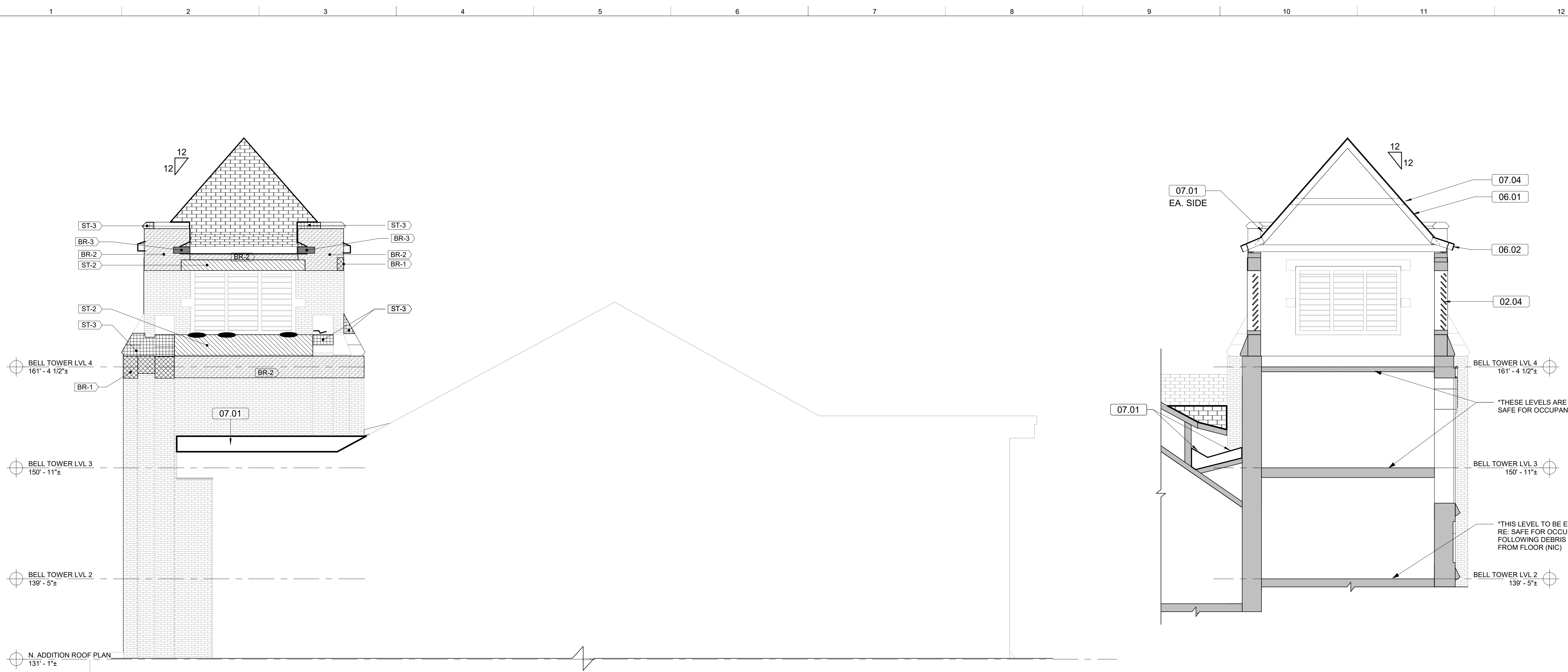


1 EAST ELEVATION
A201 3/16\"/>

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2 SOUTH ELEVATION
A201 3/16\"/>



G1 NORTH ELEVATION
 A202 3/16" = 1'-0" REFERRED FROM A110

G9 BELL TOWER BUILDING SECTION
 A202 3/16" = 1'-0" REFERRED FROM A110

GENERAL ROOF NOTES

R 1	SLOPE OF NEW ROOFING TO MATCH EXISTING IN AREA OF REPLACEMENT, U.O.N.
R 2	NEW FLASHING TO BE PREFINISHED ALUM., U.O.N.
R 3	INTERWEAVE AND LAP NEW SHINGLES WITH EXISTING TO REMAIN SHINGLES AT INTERSECTION OF AREA OF WORK WITH AREAS NOT IN CONTRACT. RESECURE EXISTING SHINGLES IMPACTED BY EFFORTS.

KEYNOTES

KEY VALUE	TEXT
02.04	EXISTING WOOD LOUVERS TO REMAIN
06.01	PLYWOOD SHEATHING OVER NEW WOOD TRUSSES - REFER TO STRUCTURAL
06.02	1X PRESSURE TREATED WOOD TIM. PTD. OVER 2X SILL PLATES AND BLOCKING REFER TO STRUCTURAL
07.01	CONTINUOUS PREFINISHED ALUMINUM VALLEY AND CRICKET FLASHING OVER NEW PLYWOOD SHEATHING AND SUPPORT FRAMING, AS NEEDED
07.03	PROVIDE CONTINUOUS PREFINISHED ALUMINUM COUNTERFLASHING - PROFILE TO MATCH EXIST.
07.04	ARCHITECTURAL GRADE ASPHALT SHINGLE ROOFING OVER WATERPROOFING UNDERLAYMENT

MASONRY REPAIR LEGEND

STONE

	ST-1 RESET: REMOVE & SALVAGE SOUND BUT DISPLACED UNITS; RESET IN MORTAR WITH NEW GALV. ANCHORS OR DOWELS
	ST-2 REPOINT: REMOVE LOOSE MORTAR & PREP JOINTS; REPOINT OPEN JOINTS WITH MORTAR TO MATCH EXISTING
	ST-3 REPLACE: REMOVE DETERIORATED UNITS; REPLACE WITH CAST STONE TO MATCH/RECREATE ORIGINAL IN PROFILE, COLOR, & TEXTURE
	ST-4 CRACK REPAIR: ROUTE CRACK & PREP JOINT; EPOXY CRACK REPAIR JOINT IN-SITU
	ST-5 SPALL: REMOVE REMAINING LOOSE MATERIAL FROM SURFACE; PREP SURFACE & PATCH REPAIR SPALL WITH FILL MATERIAL FLUSH WITH FACE OF EXISTING STONE TO REMAIN
	ST-6 SPALL: REMOVE DETACHED OR LOOSE STONE MATERIAL FROM SURFACE ONLY

BRICK

	BR-1 RECONSTRUCT: DISMANTLE DISPLACE OR DETERIORATED MASONRY; SALVAGE SOUND UNITS FOR REINSTALLATION & REPLACE BROKEN UNITS; RESET SALVAGED AND NEW UNITS IN MORTAR (WITH NEW TIES) TO REBUILD
	BR-2 REPOINT: REMOVE DETACHED OR LOOSE MORTAR & PREP JOINTS; REPOINT OPEN JOINTS WITH MORTAR MATCH EXISTING
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ROOF LEGEND

	EXISTING ASPHALT SHINGLE ROOF TO REMAIN
	EXTENT OF NEW ROOF: PROVIDE ARCHITECTURAL GRADE ASPHALT SHINGLES OVER WATERPROOFING UNDERLAYMENT, OVER NEW SHEATHING - REFER TO STRUCTURAL

PROJECT INFORMATION

PROJECT MANAGER: A. CECIL
 DESIGNER: S. RUTLAND

QEA No. 42134130

50% CD SET
 1/24/2022

ELEVATIONS & SECTIONS

A202

Scale: 3/16" = 1'-0" SCALE OF FEET

QUINN EVANS

4219 WOODWARD AVE
 SUITE 301
 DETROIT, MI 48201

v 313.462.2550

QUINNEVANS.COM

NOT FOR CONSTRUCTION

KING SOLOMON BAPTIST CHURCH

ROOF REPLACEMENT - 50% CD SET

6125 FOURTEENTH STREET DETROIT, MI

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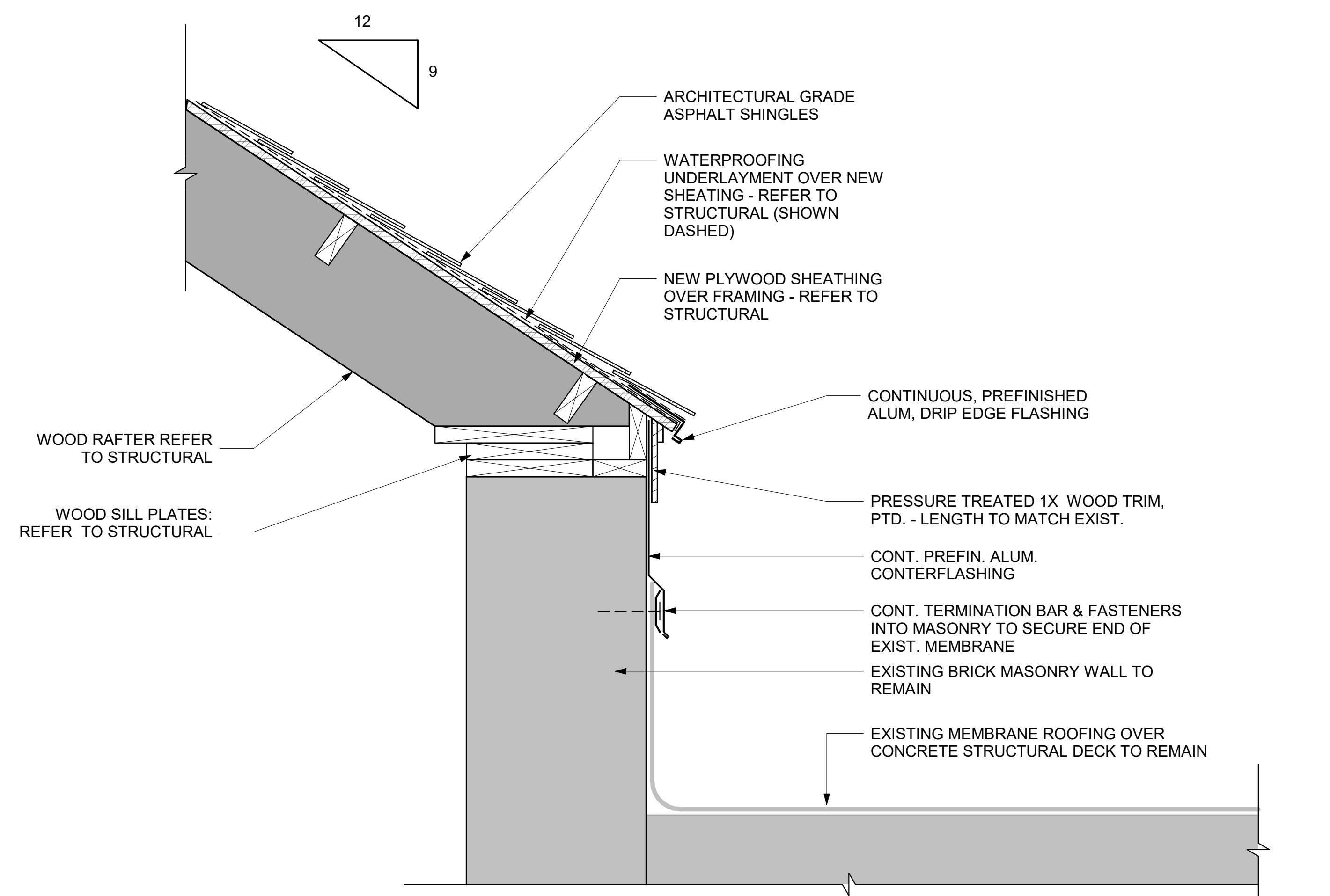
A1 WEST ELEVATION
 A202 3/16" = 1'-0" REFERRED FROM A110

KING SOLOMON BAPTIST CHURCH

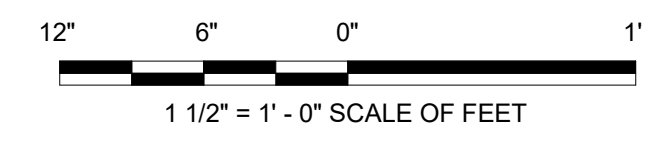
ROOF REPLACEMENT -
50% CD SET

6125 FOURTEENTH STREET DETROIT, MI

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A8 SHORTENED EAVE OVERHANG AT HEPTAGON TIER TYP.
A301 1 1/2" = 1'-0" REFERRED FROM A110



No.	Date	Description
PROJECT MANAGER:	DESIGNED BY:	
A. CECIL	S. RUTLAND	
QEA No. 42134130		
50% CD SET		
1/24/2022		

DETAILS

A301