# 2221 WABASH - SINGLE FAMILY RENOVATIONS

2221 WABASH ST.

# **ARCHITECT**

4545 ARCHITECTURE | DESIGN, PLLC TIMOTHY FLINTOFF 4545 Commonwealth St Detroit Mi 48208

## PROJECT DATA

TITLE SHEET AND SHEET INDEX **BUILDING CODE AUTHORITY:** 

DEMOLITION FIRST FLOOR AND FOUNDATION PLANS

ROOF FRAMING PLANS AND DETAILS

EXTERIOR ELEVATIONS

EXTERIOR RENDERING BUILDING SECTIONS

**BUILDING SECTIONS** 

#### APPLICABLE CODES:

ZEIGER PROPERTIES, LLC

BUILDING CODE ALSO KNOWN AS THE "MICHIGAN BUILDING CODE" 2015 MICHIGAN BUILDING CODE (MBC) AS AMENDED

ALSO KNOWN AS THE "MICHIGAN MECHANICAL CODE" 2015 MICHIGAN MECHANICAL CODE AS AMENDED

City of Detroit

OWNER:

DIANE ZEIGER

2512 SAN ELIJO AVE

CARDIFF, CA 92007

ALSO KNOWN AS THE "MICHIGAN PLUMBING CODE" 2015 MICHIGAN PLUMBING CODE AS AMENDED

ALSO KNOWN AS THE "MICHIGAN ELECTRICAL CODE" 2017 NATIONAL ELECTRIC CODE (NEC) AS AMENDED & MICHIGAN AMENDMENTS PART 8.

#### **ENERGY CODE**

2015 UNIFORM ENERGY CODE

## BARRIER FREE REQUIREMENTS

AMERICANS WITH DISABILITIES ACT (ADA)

# ICC / ANSI 117.1 - 2010, EXCEPT SECTION 611 & 707

## TS1.2

SHEET INDEX

GENERAL NOTES

SPECIAL INSPECTIONS

ARCHITECTURAL SITE PLAN

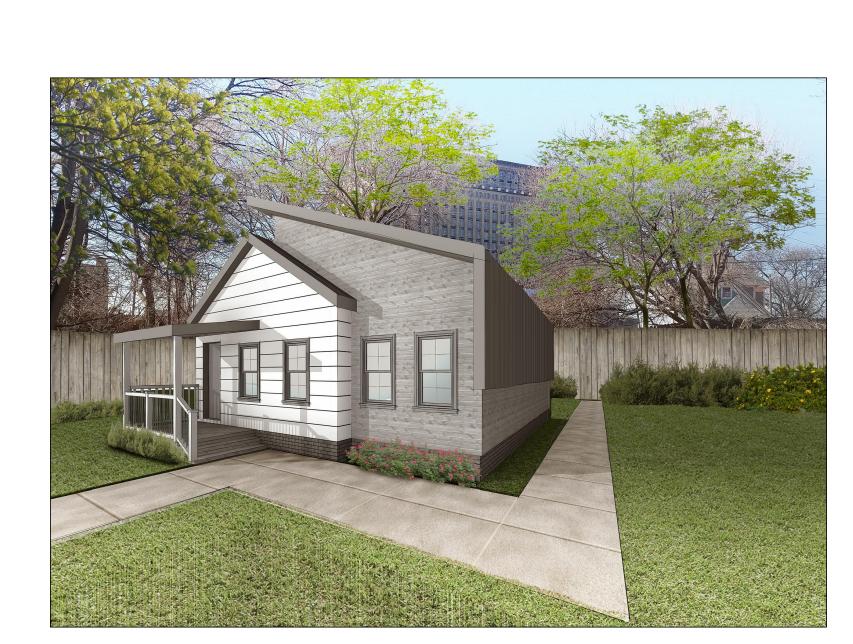
FIRST FLOOR AND FOUNDATIONS PLANS

AIR BARRIER DETAILS

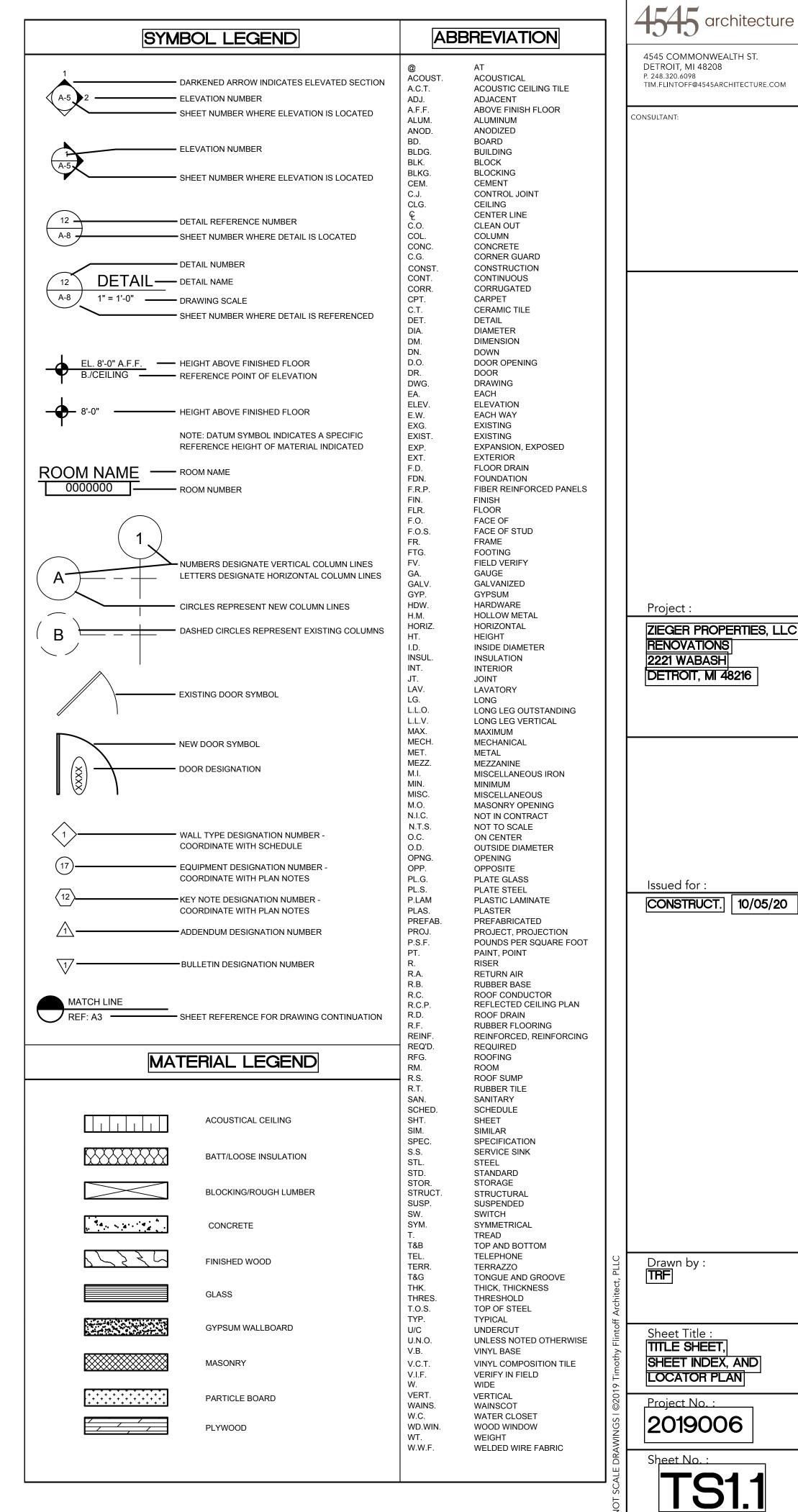
— PROJECT LOCATION 2221 Wabash St. Detroit MI







EXTERIOR RENDERING



- Contractor shall visit the site and become familiar with the existing conditions. Contractor shall verify conditions covering or affecting the structural work; obtain and verify all dimensions and elevations to ensure the proper strength, fit and location of the structural work; report to the Architect/Structural Engineer any and all conditions/discrepancies which may interfere with or otherwise affect or prevent the proper execution and completion of the new work in compliance with the Construction Documents. All discrepancies shall be fully resolved prior to commencing work.
- 3. Existing construction not undergoing alteration is to remain undisturbed. Where such construction is disturbed as a result of the operations of this contract, Contractor shall repair or replace as required and to the satisfaction of the Architect/Structural Engineer and Owner's Representative.
- 4. Contractor shall verify the existence, location and elevation of existing utilities. sewers, drains, etc. in demolition areas before proceeding with the work. All discrepancies shall be documented and reported to the Architect/Structural Engineer and Owner's Representative for resolution.
- 5. Should uncharted piping or other utilities be encountered during excavation, Contractor shall consult the Architect/Structural Engineer and Owner's Representative for resolution.
- 6. Contractor shall provide fire watch during field cutting and welding operations,
- meeting the Owner's requirements. 7. Contractor shall provide temporary protection of existing equipment during
- execution of work, satisfying the Owner's requirements. 8. Contractor shall provide temporary protection to prevent damage from the
- weather and vandalism. Contractor shall coordinate work with the Owner's personnel to avoid any
- interference in their operations.
- 10. Refer to SHORING AND BRACING notes for additional requirements.

- Structural sawn lumber, glued laminated timber and connections have been designed in accordance with the "National Design Specifications for Wood Construction.
- Plywood has been designed in accordance with "Plywood Design Specification". Structural sawn lumber shall be Spruce-Pine-Fir No.1/No.2 or better with base design values: Fb = 875 psi Fc (perpendicular) = 425 psi
  - Ft = 450 psi Fc (parallel) 1,150 psi Fv = 135 psi E = 1,400 ksi
- 4. With the moisture content of the wood in service will not exceed 19%.
- Structural sawn lumber used specifically for top and sill plates shall be Spruce-Pine-Fir Select Structural Grade, unless noted otherwise, with base design values:
  - Fb = 1250 psi Fc (perpendicular) = 425 psi Ft = 700 psi Fc (parallel) 1,400 psi
- Fv = 135 psi E = 1,500 ksiWith the moisture content of the wood in service will not exceed 19%.
- Structural glued laminated timbers shall be produced in accordance with ANSI/AITC A190.1 and ASTM D3737, stress class 24F-1.8E with base design values:
  - Fbx = 2,400 psi Fc (perpendicular) = 650 psi
- Fvx = 240 psi E = 1,800 ksiWith the moisture content of the wood in service will not exceed 16%.
- 6. Structural composite lumber shall conform to ASTM D5456 with the following allowable design
- stresses: Microllam LVL Parallam PSL Timberstrand LSL E = 2,000 ksi E = 2,000 ksiE = 1,500 ksi Fb = 2,500 psiFb = 2,900 psiFb = 2,250 psi Fc (parallel) = 2,310 psi Fc (parallel) = 2,900 psi Fc (parallel) = 1,950 psi Fc (perpendicular) = 750 psi Fc (perpendicular) = 650 psi Fc (perpendicular) = 650 psi Fv = 285 psi Fv = 290 psi
- 7. Plywood shall conform to "Voluntary Product Standard PS 1, Structural Plywood". Oriented Strand Board (OSB) shall conform to "Voluntary Product Standard PS 2, Performance
- Standard for Wood-Based Structural-Use Panels". Prefabricated wood I-joist shall conform to ASTM D5055.
- 10. Roof sheathing shall be 15/32", APA Rated Sheathing, 32/16 min., Exposure 1. Install with the long dimension or strength axis of the panel across supports and with panel continuous over two or more spans. Provide 6d common nails at 6" on center along supported panel edges and 12" on center at intermediate supports.
- 11. Floor sheathing shall be 23/32", APA Rated Sheathing, 48/24 minimum, Exposure 1. Install with the long dimension or strength axis of the panel across supports and with panel continuous over two or more spans. Panel edges shall be tongue-and-groove or supported on 2-inch lumber blocking. Provide 8d common nails at 6" on center along supported panel edges and 12" on center at
- 12. Floor sheathing shall be glued and nailed to supporting members. Adhesive shall meet APA
- Specification AFG.01, applied in accordance with the Manufacturer's recommendations. 13. Shear wall sheathing shall be 15/32", APA Rated Sheathing, 32/16 minimum, exterior. All panel edges are backed with 2-inch nominal framing. Install panels vertically or horizontally. Provide 8d
- common nails at 6" on center along panel edges and 12" on center on intermediate supports. 14. Provide one line of bridging for each eight feet of span for roof joists and floor joists. The bridging shall consist of 1 inch by 3 inch lumber, double nailed at each end, of equivalent metal bracing of
- equal rigidity or full depth solid blocking. 15. Joists shall be supported laterally at the ends and at each support by solid blocking not less than 2
- inches in thickness and the full depth of the joist. 16. Holes bored in joists shall not be within 2 inches of the top or bottom of the joist and the diameter of any such hole shall not exceed one-third the depth of the joist.
- 17. Joist framing from opposite side of a beam, girder or bearing wall shall be lapped at least 3 inches. 18. Joist framing into the side of a wood girder shall be supported by framing anchors or joist hangers.
- 19. Provide minimum three studs at each corner of an exterior wall.
- 20. Holes bored in load bearing and shear wall studs shall not exceed one-fourth the depth of the stud, within the middle third of the stud. No holes are allowed in bearing posts and shear wall ends.
- 21. Bearing and exterior wall study shall be capped with double top plates installed to provide inches and shall be nailed with not less than eight 16d face nails on each side the joint.
- 22. Bolts and lag screws shall conform to ASTM A307. 23. Nails, spikes and staples shall conform to ASTM F1667.
- 24. The fastening schedule for wood framing elements shall comply with the Governing Building Code, Table 2304.10.1.
- 25. Truss and wood I-joist Manufacturer shall visit the site to observe and approve truss and I-joist placement, bearing, bracing, and connections prior to enclosing the wood framing.
- 26. Lumber shall be so handled and covered as to prevent marring and moisture absorption from snow
- or rain until the building is enclosed.
- 27. Erection of structural timber framing shall be in accordance with AITC.

# Concrete structural framing has been designed by the Ultimate Strength Method per ACI 318

- "Building Code Requirements for Structural Concrete". Concrete work shall conform to the requirements of ACI 301, "Specifications for Structural Concrete of Buildings", and ACI 318 "Building Code Requirements for Structural Concrete" except as modified
- by Structural requirements noted on the Drawings. All concrete work shall conform to ACI 201.2R, "Guide to Durable Concrete". Parking structures shall also conform to ACI 362.1R, "Guide to Durable Concrete for Parking Structures"
- Cement shall conform to ASTM C150 "Specification for Portland Cement" type I or III.
- Concrete aggregates shall conform to ASTM C33 "Specification for Concrete Aggregates".
- Reinforcing shall conform to ASTM A615 grade 60. Reinforcement shall be fabricated and erected according to the ACI standards: "Details and Detailing of Concrete Reinforcement", ACI 315 - and "Manual of Engineering and Placing Drawings for
- Reinforced Concrete Structures", ACI 315R. Welded wire fabric shall be furnished in flat sheets (rolls not permitted) and shall conform to ASTM
- A-185 and have a minimum side and end lap of 8 inches. Welding of reinforcing steel is prohibited unless specifically detailed. Welding where detailed shall
- conform to AWS D1.4 specification. 10. Concrete shall have a minimum 28-day compressive strength as follows: Foundations: 3.000 psi
- 3,000 psi Slab-on-grade 11. Exterior concrete, and interior concrete subjected to freeze/thaw cycles, salt, etc., including walls,
- shall be air-entrained 6% +/- 1%. 12. Concrete shall be normal weight, unless indicated otherwise.
- 13. Contractor shall submit the concrete mix design(s) for review by the Structural Engineer. Proportion mix designs and provide proof of mix design strength as defined in ACI 301. The submittal shall include cement type and source, cement cube strength, aggregate gradations, water tests, admixture catalog information and cylinder strength test results from 30 tests, on specimens with identical mix
- design, for each concrete mix, or other proof of strength per ACI 301. 14. The approved materials and mix design shall be fully documented and reviewed by the Testing Agency for full compliance. Responsibility for obtaining the required design strength is the
- Contractor's responsibility. 15. Use of calcium chloride, chloride ions, or other salts in concrete is not permitted.
- 16. Minimum lap splice shall be Class B per ACI 318. Location of lap splices shall be as indicated on Construction Documents and/or as shown on the approved reinforcing steel shop drawings.
- 17. Reinforcing steel shall not be cut, bent or straightened in the field unless approved by the Structural Engineer or as indicated on the Drawings.
- 18. Reinforcing steel shall be placed with the following concrete cover unless noted otherwise: A. Concrete cast against earth (not formed): 3"
- B. Formed concrete exposed to earth or weather: a. #5 bars or smaller:
- b. #6 thru #18 bars: C. Formed concrete not exposed to earth or weather:
  - a. Slabs, joists, and walls, #11 bars or smaller: 3/4" b. Slabs, joists, and walls, #14 bars or larger:
- c. Beams, columns, pedestals, and tensions ties: 1 1/2"
- Clearance between parallel bars in a horizontal layer shall not be less than the bar diameter, 1", or 4/3 d agg, whichever is greater. Clearance between parallel bars in two or more horizontal layers, shall not be less
- Clearance between longitudinal bars in columns, pedestals, struts, and boundary elements in
- walls shall not be less than 1.5 times the bar diameter, 1 1/2", or 4/3 d agg, whichever is
- Maximum deviation from these requirements shall be: + 3/8" for sections with dimensions of 8" or less
- + 1/2" for sections with dimensions over 8"
- 19. Tie embeds securely in place prior to placing concrete.
- 20. Do not place pipes or ducts exceeding one quarter the slab or wall thickness within the slab or wall unless specifically shown and detailed on the Structural drawings. Pipes or duct shall be located
- within middle third of slab or wall thickness. 21. Anchor rods and steel embeds (furnished by Structural Steel Contractor) shall be set by template to within a 1/8" tolerance in any direction with minimum embedment and exact projection indicated on
- the drawings, prior to placing concrete. 22. No aluminum conduit or products containing aluminum or any other material injurious to the concrete shall be embedded in the concrete.
- 23. Dowels into foundation shall match size and spacing of vertical reinforcement at all columns, piers and walls, unless otherwise noted.
- 24. Contractor shall coordinate all masonry dowel sizes and spacing to be cast into concrete with
- masonry reinforcing shop drawings. 25. Provide two #5 bars (one each face) around unframed openings in slabs and walls. Place bars
- parallel to sides of openings and extend them 24 inches beyond corners, unless otherwise noted. 26. Locate sleeves, openings, embeds, etc. as indicated on the drawings. The Concrete Contractor shall check with other trades to make sure the sleeves, openings and embeds that are to be provided and set by them are in place prior to placing of concrete in the area involved.
- Contractor shall obtain approval prior to placing openings or sleeves not shown on the Drawings, through any structural member
- 28. Contractor shall review Architectural, Mechanical and Electrical drawings for bases, openings, sleeves, anchors, inserts, conduits, recesses and other devices in concrete work before placing
- 29. Horizontal construction joints are permitted only where indicated. The location of vertical construction joints shall be submitted to the Structural Engineer for review and approval. Construction joints shall be thoroughly mechanically roughened, cleaned and bonding agent applied before placement of
- 30. For control joints in slabs, space joints at maximum 15'-0" on center unless otherwise noted on the Drawings. For control joints in walls, space joints at maximum 10'-0" on center unless otherwise
- 31. Construction joints shall be furnished with a full length keyway centered on members. Where the size of key is not shown on the Drawings, the key shall be 25% of the cross-section dimension of the
- member and minimum 1-1/2 inches into the first pour of concrete Provide waterstops in construction joints in cast-in-place concrete elements that have one side exposed to the weather or soil and the other side occurring adjacent to enclosed space. Refer to
- Drawings and Specifications for other waterproofing and damp proofing details. 33. Provide 3/4" by 3/4" chamfer strips at all exposed corners of concrete members, unless noted otherwise.
- overlapping at corners and at intersections. End joints in double top plates shall be offset at least 48 34. Provide dovetail slots in concrete members where masonry abuts and where required for veneer The Concrete Contractor shall be responsible for all pour sequences and construction procedures for

all concrete work to account for temperature differentials and shrinkage occurring during the

- construction phase until the building is permanently in a mechanically controlled environment. 36. Coordinate vapor retarder requirements with floor finish requirements. 37. Concrete shall be placed to the constant top of slab elevations, while maintaining the minimum
- concrete thickness noted on the Drawings. Construction joints for supported slabs shall be as noted on the Drawing or as approved by the
- 39. The use of chlorides such as deicing salts is prohibited for melting ice prior to placement of concrete.
- 40. Sizes of concrete placements shall not exceed the following, unless otherwise indicated on the plans: 40 feet maximum length Slabs on grade: Place in alternating strips (approximate width 30 feet & maximum length 200 feet)
- 41. For floor finish tolerances for interior slabs, refer to Specifications. 42. Curing of concrete surfaces shall conform to ACI 308.1 "Standard Specification for Concrete Curing" and ACI 308R "Guide to Curing Concrete".
- joint sealant unless noted otherwise. All joint edges, including top and bottom surfaces and vertical and horizontal surfaces shall be formed or tooled as required. Joint sealant shall be applied only to the top, vertical, and horizontal surfaces unless noted otherwise on the Drawings. 44. Joints to be prepared and filled with joint sealant shall include, but are not limited to, construction

43. Joints between the structural (and architectural) members shall be properly prepared and filled with

- joints, control joints, isolation joints, and all interface joints between similar and dissimilar members. Specific locations may be indicated on the Drawings, or may be required by approved shop drawings, or may occur due to the construction sequence selected by the Contractor. 45. Prior to placing concrete adjacent to existing concrete, mechanically roughen, then thoroughly clean
- and de-grease existing concrete surfaces. Apply epoxy bonding agent prior to placing fresh concrete. Bonding agent shall be "Sika Armatec 110 EpoCem" by Sika Corporation, or approved equal. Follow all Manufacturer's instructions for surface preparation, mixing and application.
- 46. Prior to placing concrete topping, mechanically roughen, then thoroughly clean and de-grease existing concrete surfaces. Soak existing concrete surfaces for minimum 12 hours. Place a concrete-slurry of cement and water within 1 hour of topping placement. 47. Non-shrink grout shall conform to ASTM C1107. Grout shall be premixed, non-shrink, non-catalyzed
- flowable, and 5,000 psi fluid consistency. 48. Reinforcing steel, anchor rods and embed placement shall be inspected, prior to placement of concrete, in accordance with ACI 318 and code required Special Inspection by qualified Inspector prior. These inspections are not included in the basic services of the Structural Engineer of Record.

natural aggregate grout with a minimum 7-day compressive strength of 7,000 psi plastic, 6,000 psi

- Concrete masonry has been designed in accordance with ACI 530, "Building Code Requirements for Masonry Structures" and shall be constructed in accordance with ACI 530.1. "Specifications for Masonry Structures"
- 2. Concrete Masonry to have a minimum 28-day compressive strength f m=1,500 psi unless noted otherwise.
- 3. Concrete Masonry units (CMU) shall conform to the following standards:
- LoadBearing Units: ASTM C90 b. Normal Weight Units: greater than 125 pcf
- 4. Load-bearing CMU shall be at minimum medium weight units, unless noted otherwise. Mortar for all masonry shall conform to ASTM C270 with minimum compressive strength of 1,800 psi. Mortar below grade shall be type M. Elsewhere mortar may be either type M or S unless specifically indicated otherwise. Use either Portland cement/lime or masonry cement for mortar.
- 6. Grout shall conform to ASTM C476 with minimum 28-day compressive strength of 3,000
- Steel bar reinforcement shall conform to ASTM A615, grade 60.
- Horizontal joint reinforcement shall be "Ladder" type with W1.7 for low walls without cavity wall with veneer diameter longitudinal bars. Spacing of horizontal joint reinforcing shall be 16" on center, maximum.
- 9. Minimum vertical CMU wall reinforcing shall be continuous #5 bars at 48" on center, unless noted otherwise. Dowels to concrete foundation to match size and spacing of reinforcing unless noted otherwise. Reinforce CMU core at corners, each side of control joints and
- each side of wall openings with additional (2) #5 continuous vertical reinforcing bars. 10. Vertical cells containing reinforcing and grout shall form a continuous cavity, free of mortar
- 11. Horizontal bond beams shall be placed at all floor levels, all stair landing levels, roof level, and top of parapets. Bond beams shall be reinforced with (2) #5 minimum continuous horizontal reinforcing bars positioned at the top of the fully grouted bond beam, unless noted otherwise. Coordinate bond beam elevations with Architectural Drawings and approved masonry reinforcing shop drawings.
- 12. Horizontal bond beam and vertical reinforcing shall be continuous unless noted otherwise. Lap splice reinforcing per the schedule below or use mechanical splices adequate for 125% of specified yield strength of the bar. Lap vertical reinforcement with minimum dowels of same size and spacing that have been previously installed in the foundations. Dowel embedment in concrete shall conform to the requirements of the CAST-IN-PLACE CONCRETE notes.
  - BAR SIZE LAP SPLICE LENGTH 30" 48" Provide mechanical splice
- 13. Reinforcing bars shall be held in position by wire ties or other approved means to insure
- design location and lap. Place bars and lap prior to grouting. 14. Grouting of masonry walls shall conform to recommended procedure for "low lift grouting" or "high lift grouting" as outlined in the NCMA TEK 3-2A – "Grouting Concrete Masonry Walls" and ACI 530.1/ASCE 6 "Specification for Masonry Structures". Grout lifts shall not
- exceed 5 feet without mechanically consolidated (vibrated) grout pours Lifts of grout shall be keyed 4 inches into the previous course of masonry below.
- Masonry below grade shall be grouted solid. 17. Sampling and Testing of mortar and grout shall be in accordance ASTM C780 and ASTM
- C1019, respectively. One test of each is required for each 5,000 square foot of wall. 18. Construction and testing of masonry prisms shall be in accordance with the procedure
- outlined in the ASTM C1314. 19. Special Inspection of masonry construction is required. Refer to project specifications and ACI 530 for quality assurance requirements. Special Inspection shall include at minimum:
- Mortar and grout testing. Reinforcement placement and lap verification.
- Verification of clear grout space prior to grouting. Verification of proper grouting procedures (grout lift and consolidation).
- 20. Contractor shall brace masonry walls to resist wind loads until floors and roofs are in place, and the masonry has reached 75% of the required strength f m. Bracing shall be provided in accordance with OSHA - Construction Safety Standards for Masonry Wall Bracing and
- NCMA TEK 3-4B "Bracing Concrete Masonry Walls During Construction". 21. Contractor shall shore masonry walls above masonry bond beam lintels until the masonry is placed full height and has reached the required strength.

## **FOOTINGS AND FOUNDATION**

otherwise on the Drawings

- 1. Contractor shall verify all conditions, including underground utilities and field measurements at job site and report any discrepancies to Owner's
- Representative 2. Provide necessary sheeting, shoring, bracing, etc. as required during
- excavations to protect sides of excavations. 3. Comply fully with requirements of OSHA and other regulatory agencies for
- safety provisions. 4. Top of spread footing elevations noted on plan are minimum elevations. In all cases, footings are to bear on undisturbed natural soils or engineered fill having a minimum net allowable bearing capacity of 2,000 psf.
- 5. Bottom of spread footing elevations to extend a minimum of 42" below grade at locations subject to freeze-thaw action. 6. Sides of foundations shall be formed. All concrete surfaces shall be maintained smooth and vertical. Slope sides of excavations as approved by the Geotechnical Engineer, and clean up sloughing before and during concrete
- Engineer shall make recommendations for specific preparation and procedure

placement. If existing soil conditions warrant earth forming, Geotechnical

- 7. Where footing steps are necessary, they shall be no steeper than one vertical to two horizontal unless noted otherwise. Footings shall be centered under columns and walls unless specifically detailed
- 9. No footings or slabs shall be placed on or against sub-grade containing free water, frost or ice. Should water or frost, however slight, enter a footing excavation after sub-grade approval, the sub-grade shall be reinspected by the Geotechnical Engineer/Testing Laboratory after removal of water or frost.
- 10. The Contractor shall provide all necessary measures to prevent any frost or ice from penetrating any footing or slab sub-grade before and after placing of concrete until the full building enclosure is completed and heated.
- 11. Excavated material shall be legally disposed of off the Owner's property or stored at the site or used for backfilling operations as required in accordance with the Geotechnical Engineer's recommendations and Project Specification
- requirements. 12. Contractor shall furnish all required de-watering equipment to maintain a dry
- excavation until backfill is complete. 13. Where new footings are adjacent or abut existing foundations, carefully hand excavate and determine bottom of existing foundation. If different than anticipated, adjust new foundations to match existing. In no case shall the new footing be lower than the existing without protection against undermining such
- as underpinning or shoring. 14. Foundation bearing soils shall be inspected by a qualified Geotechnical Engineer. The testing shall include, but not be limited to, identification of soils at and below the foundation bearing level, and the allowable bearing capacity of
- 15. A Geotechnical Engineer registered in the State of the Project shall inspect the condition and assure the adequacy of all subgrades, fills, backfills before placement of foundations, footings, slabs and walls. They shall submit reports to the Architect/Engineer describing their investigations, including any non-conforming work.

- The structural notes are intended to augment the drawings and specifications. Should conflicts exist between the Drawings, Specifications and the Structural notes, the strictest provision shall govern.
- 2. The Structural drawings form an integral part of Contract Documents, which include Architectural, Structural, Mechanical, Electrical, Civil/Site drawings and
- Specifications. Coordinate the Structural drawings with the requirements shown in the other components of the Contract Documents. Typical details and other sections/details apply to conditions that are similar to the conditions described in the sections/details, even if they are not specifically
- referenced on the plans. The Contractor shall be responsible for means, methods, sequences and procedures of construction.
- The structure is designed to be selfsupporting and stable after it is fully completed per requirements of Contract Documents. Contractor shall determine erection procedures and sequence, and ensure the safety of the building and its component parts during erection. This includes the addition of temporary bracing, guys or tiedowns if necessary. Contractor shall retain ownership of
- such material after completion of the project. Construction shall comply fully with the applicable provisions of OSHA and the local Governing Codes, current edition, and all requirements specified in the codes shall be adhered to as if they were called for or shown on the drawings. This shall not be construed to mean that requirements set forth on the drawing may be modified because they are more stringent than the code requirements or because they are not specifically required by code.
- Governing Building Code Michigan (International) Building Code 2015. Standards listed in structural note sections refer to the version and effective date identified in the REFERENCED STANDARDS Chapter in the Governing Building Code.
- Work constructed per these drawings shall be inspected by an Independent Testing Agency retained to ensure compliance with the requirements shown on the Drawings. Special Inspections required by the Governing Building Code, local building department and the Contract Documents shall be performed by a qualified Special Inspector. Project site visits by the Engineer do not constitute or replace inspection.

#### SHOP DRAWINGS Submit shop drawings for review as indicated in material section of general

- Structural notes 2. Use of Engineering Drawings as erection drawings by the Contractor is strictly
- Allow in the schedule detailing, fabrication and erection a minimum of 10 working days for review of each shop drawing submittal by the Structural Engineer. Submit shop drawings in reasonable quantities at reasonable intervals (not more than 70 drawings per submittal per week). The 10 working days stated herein, will be in addition to the review time required by other project team members. Submit a shop drawing submittal schedule prior to the
- first submittal. 4. Review of shop drawings and other submittals by the Structural Engineer does not relieve the Contractor of the responsibility to check the shop drawings prior to submittal. Errors and omissions associated with the preparation of shop drawings not conforming to the Construction Documents are the responsibility of the shop drawing preparer.
- 5. Shop drawings are an aid for field placement and are superseded by the Contract Documents. Contractor shall ensure that construction is in accordance with the latest Contract Documents. Shop drawing review is only for general compliance with the Contract Documents. Review of the shop drawings by the Structural Engineer does not guarantee that the shop drawings are correct nor
- infer that the shop drawings supersede the Contract Documents. 6. Contractor shall provide a set of approved shop drawings bearing the review stamp of the Structural Engineer, to the local building department and to the
- project site. 7. Notes on submitted shop drawings for work "by others" cannot be responsibly approved by Structural Engineer. Contractor shall coordinate responsibility for materials, connections, etc. prior to shop drawing submittal to the Structural
- 8. Contractor shall verify all relevant dimensions and elevations for equipment installations against purchased Manufacturer's certified equipment drawings. Contractor shall coordinate dimensions that depend upon specific equipment, such as elevator openings, mechanical equipment supports, etc., prior to submittal. Such dimensions shall be provided on the shop drawings prior to submittal to the Structural Engineer. Contractor's failure to provide such dimensions on submitted shop drawings will result in shop drawing return

#### SHORING AND BRACING Contractor shall provide temporary shoring and bracing of existing construction, new construction, and underground utilities as follows:

- Where shown or noted on the Drawings. Where existing construction is to be altered or disturbed until permanent
- support is in place. Where existing construction is not undergoing alteration, and is to remain undisturbed but is disturbed as a result of the work of this contract.
- As required for safe erection, installation of new construction, equipment, When needed for Contractor's "means and methods" of construction and

other safety related issues.

Shoring and bracing shown on the Drawings is conceptual. Contractor shall be responsible for verifying existing conditions, shoring and bracing calculations, methods of installation, transfer of loads through to final load support, and work sequence phasing with new construction.

Shoring and bracing shall be performed by a Contractor with minimum 5 years

verified by tests, unless conservative estimates that do not affect deflections

- demonstrated experience in similar size and scope of shoring and bracing Shoring and bracing shall be designed by a Professional Engineer registered in the State of the Project with minimum 5 years demonstrated experience in similar size and scope of shoring and bracing projects. Design loads and methods shall conform to applicable codes. Soil and material strengths shall be
- and deformations are approved by the Architect/Structural Engineer. Contractor shall submit drawings and calculations sealed and signed by the Contractor's Professional Engineer showing complete design including temporary conditions, final conditions and sequence of work.
- Before starting work, Contractor shall perform condition survey of the existing building structure, exterior façade and interior finishes, including photographic documentation and submit survey to the Owner for record.

7. During the shoring and bracing operations, Contractor shall:

Owner and Architect/Structural Engineer.

- a. Keep the existing and new construction in a safe condition. Monitor existing and new construction to detect any signs of distress or deformation. Take immediate steps to prevent distress, deformation or damage.
- according to the Contractor's design and issue approval for inspection of the work by the Testing Agency. 9. After completion of shoring and bracing and completion of work requiring shoring and bracing, Contractor shall repair any damage to the existing and new construction, without any cost to the Owner, and to the satisfaction of the

Contractor shall continuously monitor the shoring and bracing system.

Contractor shall review and ascertain that all field connections are completed

4545 architecture

4545 COMMONWEALTH ST. DETROIT, MI 48208 P. 248.320.6098

TIM.FLINTOFF@4545ARCHITECTURE.COM

CONSULTANT:

Project:

ZIEGER PROPERTIES, LLC RENOVATIONS 2221 WABASH DETROIT, MI 48216

Issued for: CONSTRUCT. 10/05/20

GENERAL NOTES

Project No.

		INSPECTION FREQUENCY		REFERENCED		RESPONSIBLE	
	TASK	CONTINUOUS	PERIODIC	STANDARD	IBC REFERENCE	AGENT	
SITE	PREPARATION:						
Α.	VERIFY SITE PREPARED IN ACCORDANCE WITH APPROVED GEOTECHNICAL REPORT.	-	X	GEOTECHNICAL REPORT	1705.6, 1803	SI/GE	
EXC	AVATION:						
A.	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	-	X	GEOTECHNICAL REPORT	1705.6	SI/GE	
FILL	PLACEMENT:				1705.6, 1803.5		
Α.	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	-	X			SI/GE/TA	
B.	VERIFY USE OF PROPER MATERIALS, DENSITIES, AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	Х	-	GEOTECHNICAL REPORT			
C.	PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT THE SITE HAS BEEN PREPARED PROPERLY.	-	X				
SHA	LLOW FOUNDATIONS:						
Α.	IDENTIFICATION OF SOILS AT AND BELOW FOUNDATION BEARING LEVEL.	-	X	GEOTECHNICAL REPORT	1705.6	SI/GE	
B. GEC	VERIFY ALLOWABLE BEARING CAPACITY OF FOUNDATION BEARING SOILS. PIER FOUNDATIONS:	-	X				
A.	VERIFY SHAFT DIAMETER AND CONDITION OF SHAFT.	X				SI/GE	
В.	VERIFY BEARING SOILS.	X			1705.7		
C.	DETERMINE CAPACITIES OF TEST GEOPIERS (MODULUS AND UPLIFT) AND CONDUCT ADDITIONAL LOAD TESTS AS REQUIRED.	X	<u>-</u>	GEOTECHNICAL REPORT			
D.	OBSERVE GEOPIER INSTALLATION OPERATION AND MAINTAIN A COMPLETE AND ACCURATE RECORD OF EACH GEOPIER INCLUDING OBSERVING SUBSURFACE CONDITIONS AND SOILS AND BOTTOM STABILIZATION TESTS.	х	-	SECTEORING ALIVER ON			
Ξ.	COORDINATE ALL ACTIVITIES WITH INSTALLER'S FULL TIME QUALITY CONTROL REPRESENTATIVE.	Х	-				
PILE	FOUNDATIONS:				1705.7		
A.	VERIFY PILE MATERIALS, SIZES AND LENGTHS COMPLY WITH THE REQUIREMENTS.	X	-				
В.	DETERMINE CAPACITIES OF TEST PILES AND CONDUCT ADDITIONAL LOAD TESTS, AS REQUIRED.	х	-				
C.	OBSERVE DRIVING OPERATIONS AND MAINTAIN COMPLETE AND ACCURATE RECORDS FOR EACH PILE.	Х	-				
D.	VERIFY PLACEMENT LOCATIONS AND PLUMBNESS, CONFIRM TYPE AND SIZE OF HAMMER, RECORD NUMBER OF BLOWS PER FOOT OF PENETRATION, DETERMINE REQUIRED PENETRATIONS TO ACHIEVE DESIGN CAPACITY, RECORD TIP AND BUTT ELEVATIONS AND DOCUMENT ANY PILE DAMAGE.	х	-	GEOTECHNICAL REPORT		SI/GE	
E.	FOR STEEL PILES, PERFORM ADDITIONAL INSPECTION IN ACCORDANCE WITH REQUIREMENTS OF STEEL CONSTRUCTION.	-	-				
F.	FOR CONCRETE PILES AND CONCRETE FILLED PILES, PERFORM ADDITIONAL INSPECTIONS IN ACCORDANCE WITH REQUIREMENTS OF CONCRETE CONSTRUCTION.	-	-				
G.	FOR SPECIALTY PILES, PERFORM ADDITIONAL INSPECTIONS AS DETERMINED BY THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE.	-	-				
PIEF	V/CAISSON/AUGER CAST PILE FOUNDATIONS:						
Α.	DETERMINE CAPACITIES OF TEST PIER/CAISSON/AUGER CAST PILE AND CONDUCT ADDITIONAL LOAD TESTS AS REQUIRED.	х	-				
В.	OBSERVE PIER/CAISSON/AUGER CAST PILE INSTALLATION OPERATION AND MAINTAIN A COMPLETE AND ACCURATE RECORD OF EACH ELEMENT INCLUDING OBSERVING SUBSURFACE CONDITIONS AND SOILS AND BOTTOM STABILIZATION TESTS.	Х	-	GEOTECHNICAL REPORT	Г 1705.8	SI/GE	
C.	COORDINATE ALL ACTIVITIES WITH INSTALLER'S FULL TIME QUALITY CONTROL REPRESENTATIVE.	Х	-	SECTEORINGAL NEFORT		SiiGL	
D.	VERIFY PLACEMENT LOCATIONS AND PLUMBNESS, CONFIRM PIER/CAISSON/AUGER CAST PILE DIAMETERS, BELL DIAMETERS (IF APPLICABLE), LENGTHS, EMBEDMENT INTO BEDROCK (IF APPLICABLE), AND ADEQUATE END-BEARING STRATA CAPACITY. RECORD CONCRETE OR GROUT VOLUMES.	х	-				

# SPECIAL INSPECTION NOTES

1. SPECIAL INSPECTIONS SHALL BE PERFORMED IN ACCORDANCE WITH THE 2012 MICHIGAN (INTERNATIONAL) BUILDING CODE CHAPTER 17 AND AS MODIFIED HEREIN.

## 2. <u>DESIGNATIONS</u>

- SI: SPECIAL INSPECTOR QUALIFIED WITH DEMONSTRATED COMPETENCE DOCUMENTED BY CERTIFICATIONS FROM RECOGNIZED AGENCIES SUCH AS AWS, ACI, MASONRY INSTITUTE OF MICHIGAN (MIM), ETC., AS SUBMITTED AND APPROVED BY THE BUILDING OFFICIAL. SPECIAL INSPECTOR MAY BE A FIRM WITH MULTIPLE SPECIALISTS AND A PROJECT MANAGE PROVIDING REPORTS.
- TA: TESTING AGENCY QUALIFIED TO TEST AND INSPECT MATERIALS AND ASSEMBLIES. TESTING AGENCY SHALL BE UNDER THE SUPERVISION OF THE SPECIAL INSPECTOR.
- GE: GEOTECHNICAL ENGINEER WHO PROVIDED THE ORIGINAL PROJECT GEOTECHNICAL SOILS INVESTIGATION REPORT.
- SE: SPECIALTY ENGINEER RESPONSIBLE FOR DESIGNING ASSEMBLIES SUCH AS PRECAST CONCRETE, STEEL JOISTS, COLD FORMED FRAMING ASSEMBLIES, ETC. SPECIALTY ENGINEER SHALL PROVIDE OBSERVATION OF FABRICATED AND INSTALLED ITEMS OF THEIR DESIGN, IN ADDITION TO THE SPECIAL INSPECTION.
- TA, GE AND SE SHALL SUBMIT RECORDS OF THE INSPECTION RESULTS TO THE SI. THE SI SHALL COMPILE AND SUBMIT INSPECTION RECORDS TO THE ARCHITECT/ENGINEER AND BUILDING OFFICIAL.
  RECORDS SHALL INCLUDE STATEMENTS OF TESTS, WHETHER INSTALLED/FABRICATED ITEM COMPLIES WITH CONTRACT DOCUMENTS, REMEDIAL WORK PERFORMED, RETESTS,
- SI SHALL PROVIDE A DAILY REPORT OF ANY DISCREPANCIES FROM THE CONTRACT DOCUMENTS FOUND ON THE SAME DAY OF THE INSPECTION TO THE ENGINEER OF RECORD. FORMAL REPORTS OF COMPLIANCE CAN FOLLOW BY A MAXIMUM OF 2 WEEKS. SI SHALL PROVIDE AND SIGN A FINAL REPORT WITH A SUMMARY OF ALL TESTS PERFORMED AND RESULTS TO THE ENGINEER OF RECORD AND BUILDING OFFICIAL.
- 5. SI, TA & GE SHALL BE PAID BY THE OWNER IN COMPLIANCE WITH THE MICHIGAN (INTERNATIONAL) BUILDING CODE.

TASK	INSPECTION FREQUENCY		REFERENCED	   IBC REFERENCE	RESPONSIBLE	
IASK	CONTINUOUS PERIODIC		STANDARD	IBC REFERENCE	AGENT	
PRE-FABRICATED WOOD:			MANUFACTURER'S	1704.5, 1705.5	SI	
A. INSPECTION OF PRE-FABRICATED WOOD STRUCTURAL ELEMENTS	-	Χ	SPECIFICATIONS	1704.3, 1703.3		
TIMBER FRAMING:						
A. INSPECTION OF FIELD GLUING OPERATIONS	X	-	7	1705.5	SI	
B. INSPECTION OF NAILING, BOLTING, ANCHORING, AND OTHER FASTENING OF COMPONENTS WITHIN THE LATERAL-FORCE-RESISTING SYSTEM INCLUDING DRAG STRUTS, BRACES, AND TIE-DOWNS.		-	MANUFACTURER'S SPECIFICATIONS			
C. INSPECTION FOR WOOD SHEAR WALLS, SHEAR PANELS, AND DIAPHRAGMS THAT ARE INCLUDED IN THE LATERAL-FORCE-RESISTING SYSTEM AND FOR WHICH THE PROVISIONS REQUIRE THE SPACING OF NAILS, SCREWS, OR FASTENERS FOR WOOD SHEATHING TO BE 6 IN. OR LESS ON CENTER.	х	-				
FLOOR AND ROOF DIAPHRAGMS:				TABLE 2306.2,	CI	
A. INSPECTION OF DIAPHRAGMS	X	-	<sup>-</sup>	1704.1, 1705.5.1	SI	

# SPECIAL INSPECTION REQUIREMENTS - MASONRY: LEVEL A

## MINIMUM TESTS

NONE

MINIMUM INSPECTION

#### VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS.

## MINIMUM TESTS

SPECIAL INSPECTION REQUIREMENTS - MASONRY: LEVEL B QUALITY ASSURANCE

VERIFICATION OF SLUMP FLOW AND VISIBILITY STABILITY INDEX (VSI) AS DELIVERED TO THE PROJECT SITE IN ACCORDANCE WITH SPECIFICATIONS ARTICLE 1.5 B.1.b.3 FOR SELF-CONSOLIDATING GROUT.

VERIFICATION OF I'M AND IAAC IN ACCORDANCE WITH SPECIFICATIONS ARTICLE 1.4 B PRIOR TO CONSTRUCTION, EXCEPT WHERE SPECIFICALLY EXEMPTED BY THIS CODE

#### MINIMUM INSPECTION

			FREQUENCY		REFERENCE FOR CRITERIA			
		INSPECTION TASK	CONTINUOUS	PERIODIC	IBC SECTION	ACI 530 ASCE 5 TMS 402	ACI 530.1 ASCE 6 TMS 602	RESPONSIBLE AGENT
1.		IFY COMPLIANCE WITH THE APPROVED SUBMITTALS.	-	X	-	-	ART. 1.5	SI
2.		PECTION OF ANCHORS INSTALLED IN HARDENED MASONRY AND GROUT ST-INSTALLED)	-	X	-	-	-	SI/TA
3.		MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN MPLIANCE:						
	A.	PROPORTIONS OF SITE-PREPARED MORTAR.	-	Х	-	-	ART. 2.1, 2.6A	
	B.	CONSTRUCTION OF MORTAR JOINTS.	-	Х	-	-	ART. 3.3B	CI
	C.	GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES.	-	Х	-	-	ART. 2.4B, 2.4H	SI
	D.	LOCATION OF REINFORCEMENT, CONNECTORS, PRESTRESSING TENDONS AND ANCHORAGES.	-	Х	-	-	ART. 3.4, 3.6A	
	E.	PRESTRESSING TECHNIQUE.	-	Х	-	-	ART. 3.6B	
	F.	PROPERTIES OF THIN-BED MORTAR FOR AACMASONRY.	X FOR FIRST 5000 SQ.FT. OF ACC MASONRY	X AFTER FIRST 5000 SQ.FT. OF ACC MASONRY	-	-	ART. 2.1C	
4.	PRIC	OR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:						
	A.	GROUT SPACE.	-	Х	-	-	ART. 3.2D, 3.2F	
	B.	GRADE, TYPE, AND SIZE OF REINFORCEMENT AND ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES.	-	Х	-	SEC. 1.16	ART. 2.4, 3.4	
	C.	PLACEMENT OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES.	-	Х	-	SEC. 1.16	ART. 3.2E, 3.4, 3.6A	SI
	D.	PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS.	-	Х	-	-	ART. 2.6B, 2.4 G.1.b	
	E.	CONSTRUCTION OF MORTAR JOINTS.		Х			ART. 3.3B	
5.	VER	IFY DURING CONSTRUCTION:		•			'	
	A.	SIZE AND LOCATION OF STRUCTURAL ELEMENTS.	-	Х	-	-	ART. 3.3F	
	B.	TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION.	-	х	-	SEC. 1.16.4.3, 1.17.1	-	SI
	C.	WELDING OF REINFORCEMENT.	Х	-	-	SEC. 2.1.8.7.2, 3.3.3.4(c), 8.3.3.4(b)	-	
	D.	PREPARATION, CONSTRUCTION AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°F).	-	х	-	-	ART. 1.8 C, 1.8 D	SI SI/TA
	E.	APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE.	Х	-	-	-	ART. 3.6 B	
	F.	PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE.	Х	-	-	-	ART. 3.5, 3.6 C	
	G.	PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS.	X FOR FIRST 5000 SQ.FT. OF ACC MASONRY	X AFTER FIRST 5000 SQ.FT. OF ACC MASONRY	-	-	ART. 3.3 B.8	
6.	OBS PRIS	ERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR SMS.	-	Х	SEC. 2105.2.2, 2105.3	-	ART. 1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3, 1.4 B.4	SI/TA

# SPECIAL INSPECTION REQUIREMENTS - MASONRY: LEVEL C QUALITY ASSURANCE

# MINIMUM TESTS

VERIFICATION OF I'M AND IAAC IN ACCORDANCE WITH SPECIFICATIONS ARTICLE 1.4 B PRIOR TO CONSTRUCTION AND FOR EVERY 5,000 SQ.FT. DURING CONSTRUCTION

VERIFICATION OF PROPORTIONS OF MATERIALS IN PREMIXED OR PREBLENDED MORTAR, PRESTRESSING GROUT, AND GROUT OTHER THAN SELF-CONSOLIDATING GROUT, AS DELIVERED TO THE PROJECT SITE.

VERIFICATION OF SLUMP FLOW AND VISIBILITY STABILITY INDEX (VSI) AS DELIVERED TO THE PROJECT SITE IN ACCORDANCE WITH SPECIFICATIONS ARTICLE 1.5 B.1.b.3 FOR SELF-CONSOLIDATING GROUT.

# MINIMUM INSPECTION

		MINIMUM INSP	ECTION	_			
INSPECTION TASK		FREQUENCY		REFERENCE FOR CRITERIA			
		CONTINUOUS	PERIODIC	IBC SECTION	ACI 530 ASCE 5 TMS 402	ACI 530.1 ASCE 6 TMS 602	RESPONSIBLE AGENT
1.	VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS.	-	Χ	-	-	ART. 1.5	SI
2.	INSPECTION OF ANCHORS INSTALLED IN HARDENED MASONRY AND GROUT (POST-INSTALLED)	-	Х	-	-	-	SI/TA
3.	VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:						
	A. PROPORTIONS OF SITE-MIXED MORTAR, GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS.	-	X	-	-	ART. 2.1, 2.6 A, 2.6 B, 2.6 C, 2.4 G.1.b	
	B. GRADE, TYPE, AND SIZE OF REINFORCEMENT AND ANCHOR RODS, AND PRESTRESSING TENDONS AND ANCHORAGES.	-	X	-	SEC. 1.16	ART. 2.4, 3.4	
	C. PLACEMENT OF MASONRY UNITS AND CONSTRUCTION OF MORTAR JOINTS.	-	Χ	-	•	ART. 3.3 B	
	D. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES.	Х	-	-	SEC. 1.16	ART. 3.2 E, 3.4, 3.6 A	
	E. GROUT SPACE PRIOR TO GROUTING.	Χ	-	-	-	ART. 3.2 D, 3.2 F	
	F. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS.	Х	-	-	-	ART. 3.5, 3.6 C	
	G. SIZE AND LOCATION OF STRUCTURAL ELEMENTS.	-	Х	-	-	ART. 3.3 F	
	H. TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION.	х	-	-	SEC. 1.16.4.3, 1.17.1	-	SI
	I. WELDING OF REINFORCEMENT.	Х	-	-	SEC. 2.1.8.7.2, 3.3.3.4(c), 8.3.3.4(b)	-	
	J. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°F).	-	X	SEC. 2104.3, 2104.4	-	ART. 1.8 C, 1.8 D	
	K. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE.	Х	-	-	-	ART. 3.6 B	
	L. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS.	Х	-	-	-	ART. 3.3 B.8	
	M. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY.	X	-	-	-	ART. 2.1 C.1	
7.	OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS.	x	-	SEC. 2105.2.2, 2105.3	-	ART. 1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3, 1 4 R 4	SI/TA

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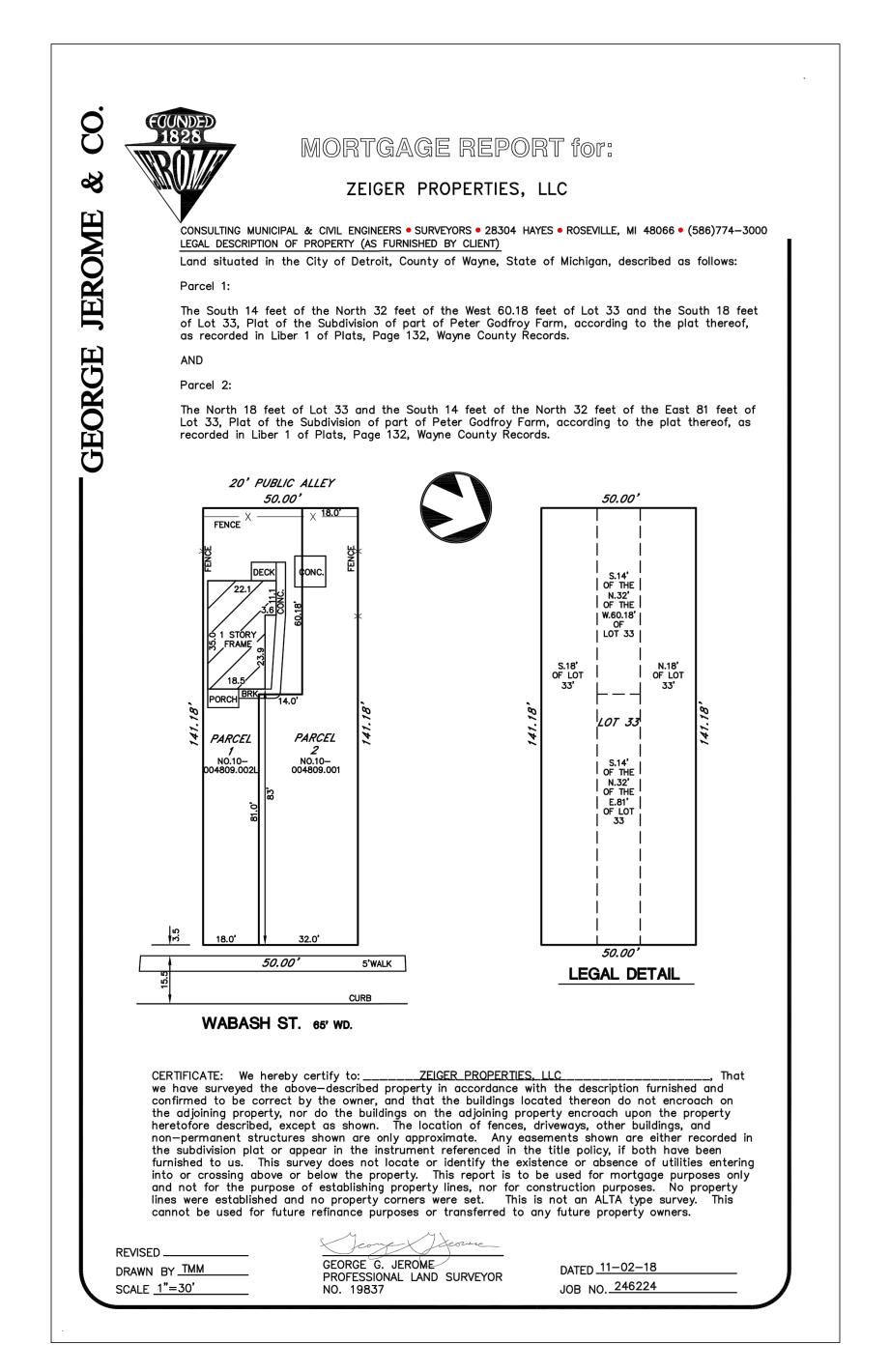
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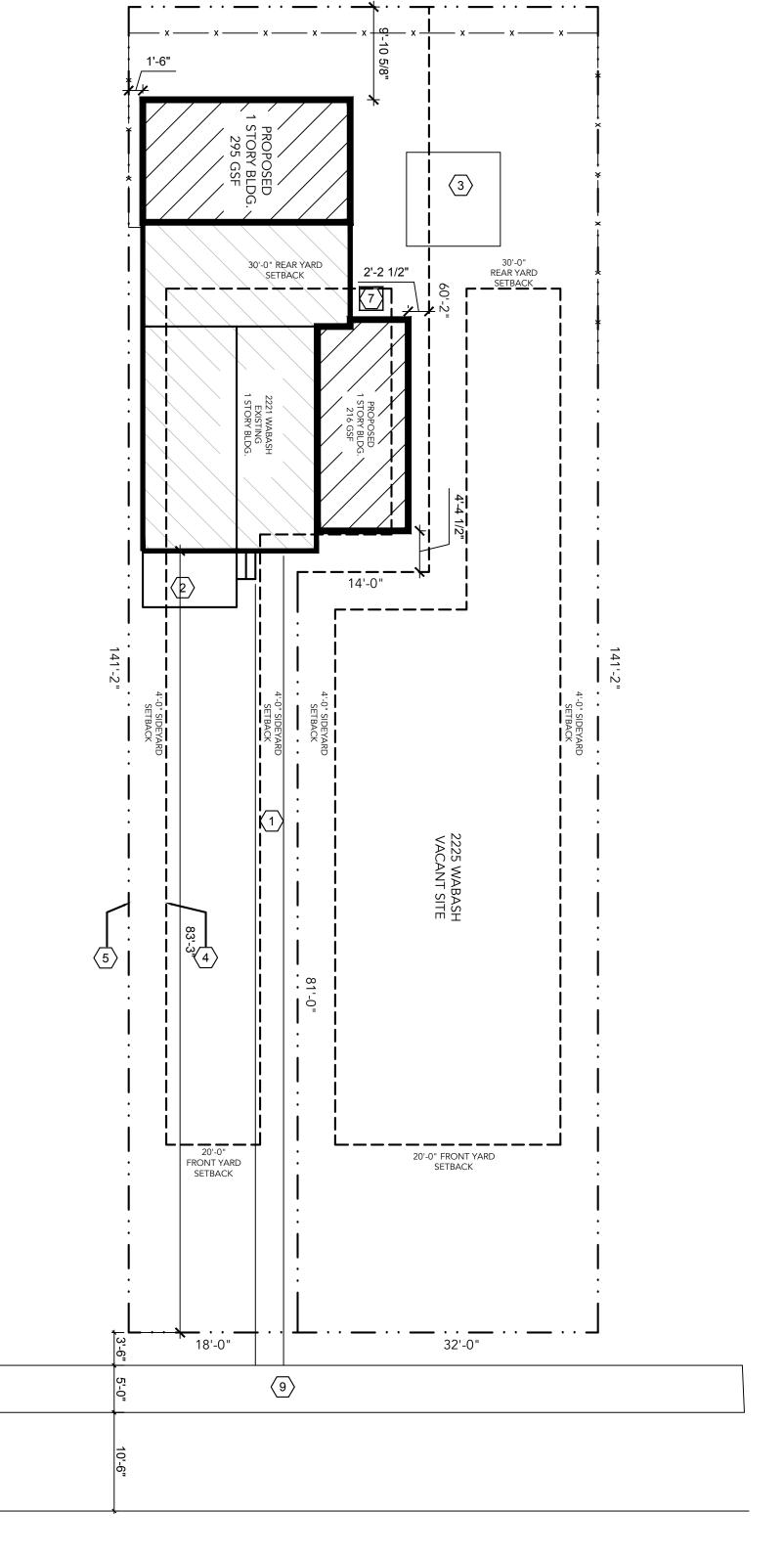
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Sheet Title :
SPECIAL INSPECTION
REQUIREMENTS

Project No. : 2019006

Sheet No.:
TS1.3





32'-0"

18'-0"

WABASH ST.



# **GENERAL SITE PLAN NOTES:**

- 1. PAVEMENT SHALL BE OF THE TYPE, THICKNESS AND CROSS SECTION AS INDICATED ON THE PLANS AND AS FOLLOWS:
- 2. CONCRETE: PORTLAND CEMENT TYPE IA (AIR-ENTRAINED) WITH A MINIMUM CEMENT CONTENT OF SIX SACKS PER CUBIC YARD, MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3,500 PSI AND A SLUMP OF 1 1/2 TO 3 INCHES.
- 3. ASPHALT: BASE COURSE MDOT BITUMINOUS MIXTURE NO. 1100L, 20AA; SURFACE COURSE MDOT BITUMINOUS MIXTURE NO. 1100T, 20AA; ASPHALT CEMENT PENETRATION GRADE 85-100, BOND COAT MDOT SS-1H EMULSION AT 0.10 GALLON PER SQUARE YARD; MAXIMUM 2 INCH LIFT.
- 4. PAVEMENT BASE SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY (MODIFIED PROCTOR) PRIOR TO PLACEMENT OF PROPOSED PAVEMENT. EXISTING SUB-BASE SHALL BE PROOF-ROLLED IN THE PRESENCE OF THE ENGINEER TO DETERMINE STABILITY.
- 5. ALL CONCRETE PAVEMENT, DRIVEWAYS, CURB & GUTTER, ETC., SHALL BE SPRAY CURED WITH WHITE MEMBRANE CURING COMPOUND IMMEDIATELY FOLLOWING FINISHING OPERATION.
- ALL CONCRETE PAVEMENT JOINTS SHALL BE FILLED WITH HOT POURED RUBBERIZED ASPHALT JOINT SEALING COMPOUND IMMEDIATELY AFTER SAWCUT OPERATION. FEDERAL SPECIFICATION SS-S164.
- 7. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH THE CURRENT STANDARDS AND SPECIFICATIONS OF THE MUNICIPALITY AND THE MICHIGAN DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR CONSTRUCTION, CURRENT EDITION.
- 8. ALL TOP OF CURB ELEVATIONS, AS SHOWN ON THE PLANS, ARE CALCULATED FOR A 6" CONCRETE CURB UNLESS OTHERWISE NOTED.
- 9. ALL SIDEWALK RAMPS, CONFORMING TO PUBLIC ACT NO. 8, 1993, SHALL BE INSTALLED AS INDICATED ON THE PLANS.
- 10. CONSTRUCTION OF A NEW OR RECONSTRUCTED DRIVE APPROACH CONNECTING TO AN EXISTING STATE OR COUNTY ROADWAY SHALL BE ALLOWED ONLY AFTER AN APPROVED PERMIT HAS BEEN SECURED FROM THE AGENCY HAVING JURISDICTION OVER SAID ROADWAY.
- 11. FOR ANY WORK WITHIN THE PUBLIC RIGHT-OF-WAY, THE CONTRACTOR SHALL PAY FOR AND SECURE ALL NECESSARY PERMITS AND LIKEWISE ARRANGE FOR ALL INSPECTION.
- 12. EXISTING TOPSOIL, VEGETATION AND ORGANIC MATERIALS SHALL BE STRIPPED AND REMOVED FROM PROPOSED PAVEMENT AREA PRIOR TO PLACEMENT OF BASE MATERIALS
- 13. EXPANSION JOINTS SHOULD BE INSTALLED AT THE END OF ALL INTERSECTION RADII.
- 14. SIDEWALK RAMPS, CONFORMING TO PUBLIC ACT NO. 8, 1973, SHALL BE INSTALLED AS SHOWN AT ALL STREET INTERSECTIONS AND AT ALL BARRIER FREE PARKING AREAS AS INDICATED ON THE PLANS.
- 15. ALL PAVEMENT AREAS SHALL BE PROOF-ROLLED UNDER THE SUPERVISION OF A GEOTECHNICAL ENGINEER PRIOR TO THE PLACEMENT OF BASE MATERIALS AND PAVING MATERIALS.
- 16. FILL AREAS SHALL BE MACHINE COMPACTED IN UNIFORM LIFTS NOT EXCEEDING 9 INCHES THICK TO 98% OF THE MAXIMUM DENSITY (MODIFIED PROCTOR) PRIOR TO PLACEMENT OF PROPOSED PAVEMENT.
- 17. ALL LANDSCAPED AREAS TO BE PROVIDED W/ SPRINKLER SYSTEMS FOR 100% COVERAGE.

## SITE PLAN KEY NOTES:

(TYPICAL THIS SHEET ONLY)

(1) EXISTING BRICK WALK WAY

(2) EXISTING FRONT PORCH(3) EXISTING CONC. PAD

4 LINE OF SET BACK BY ORDINANCE

5 PROPERTY LINE

6 SIDEWALK

 $\overline{\langle 7 \rangle}$  AIR CONDITIONING CONDENSER

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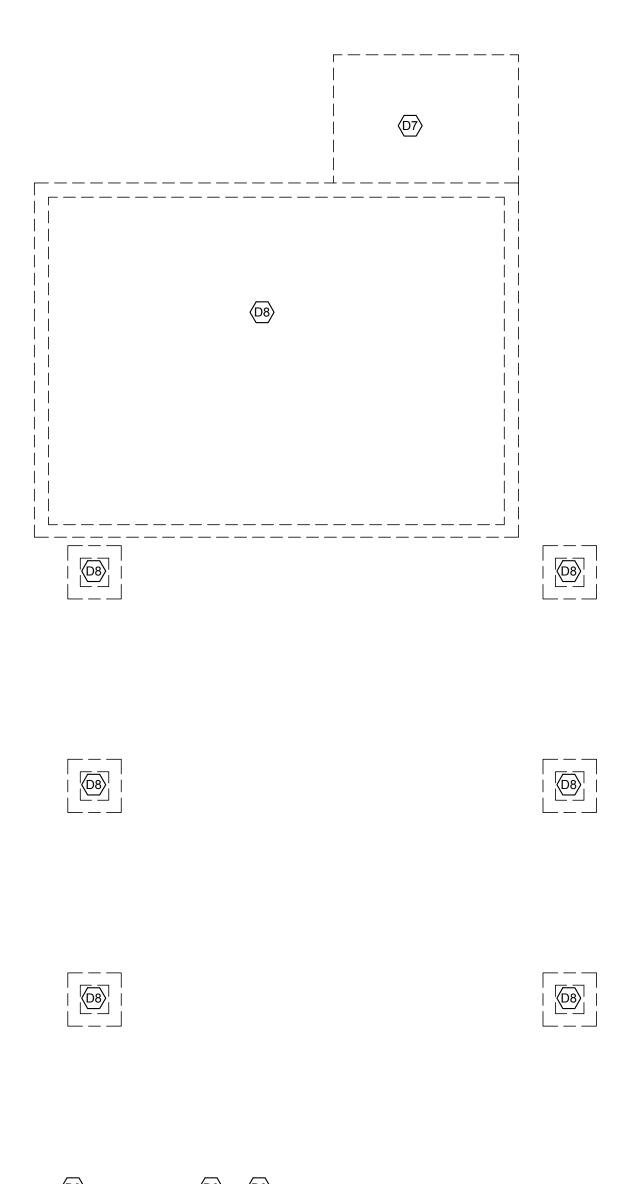
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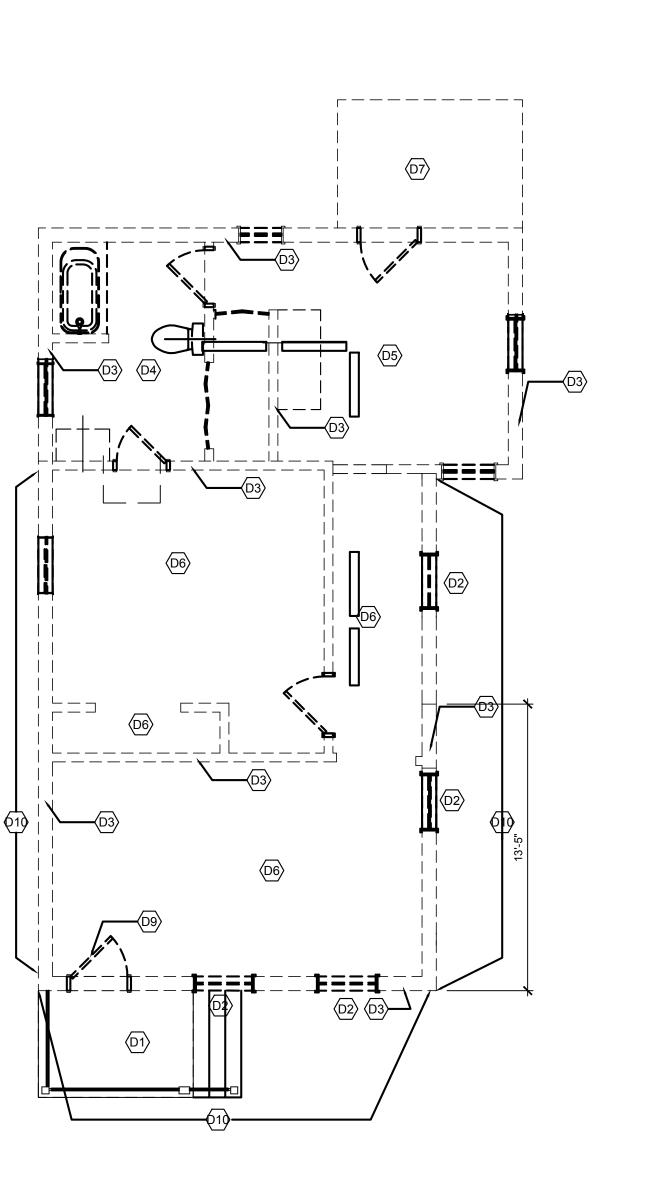
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ARCHITECTURAL
SITE PLAN

Project No. : 2019006

Sheet No.:
SP1.1











# GENERAL DEMOLITION PLAN NOTES:

- ALL DEMOLITION WORK REQUIRED IS NOT NECESSARILY LIMITED TO WHAT IS SHOWN ON THE DEMOLITION PLANS. THE INTENT IS TO REMOVE ALL MECHANICAL, ELECTRICAL, AND ARCHITECTURAL ITEMS AS REQUIRED TO FACILITATE NEW CONSTRUCTION.
- COORDINATE SCOPE AND EXTENT OF DEMOLITION WORK WITH NEW WORK PLANS AND DETAILS.
- 3. ALL WALLS, DOORS, FRAMES, AND RELATED HARDWARE ASSEMBLIES DESIGNATED AS "TO BE REMOVED" (SHOWN AS DASHED LINES) SHALL BE COMPLETELY REMOVED AND DISPOSED OF AS DESIGNATED BY OWNER/TENANT. ALL EXISTING WALLS NOT DESIGNATED FOR DEMOLITION SHALL BE PROTECTED FROM DAMAGE AND REMAIN "AS-IS".
- 4. ALL EQUIPMENT, DOORS, FRAMES, RELATED HARDWARE, AND DESIGNATED ITEMS TO BE SALVAGED SHALL BE REMOVED, PROTECTED FROM DAMAGE, AND STORED FOR REUSE.
- 5. CLEAN AND REPAIR ALL EXISTING FLOOR FINISHES AS NECESSARY.
- 6. ALL DEMOLITION WORK SHALL BE PERFORMED IN A NEAT AND WORKMANLIKE MANNER. ALL SURFACES ADJACENT TO AND ABUTTING TO THOSE DESIGNATED "TO BE REMOVED" SHALL BE LEFT WITH A SMOOTH AND FLUSH APPEARANCE.
- 7. THE CONTRACTOR SHALL EXERCISE ALL REQUISITE CARE NECESSARY TO ENSURE THAT ALL EQUIPMENT, MATERIALS, FINISHES AND ASSEMBLIES WHICH ARE NOT BEING REMOVED ARE PROTECTED FROM DAMAGE DURING DEMOLITION AND SUBSEQUENT CONSTRUCTION OPERATIONS.
- 8. REFER TO MECHANICAL AND ELECTRICAL DEMOLITION DRAWINGS AND SPECIFICATIONS FOR ADDITIONAL DEMOLITION INFORMATION.
- 9. GENERAL PRECAUTIONS SHALL BE TAKEN AS NECESSARY TO HOLD ALL DISRUPTION, DUST, DIRT, NOISE, AND DEBRIS TO A MINIMUM.
- 10. THE CONTRACTOR SHALL COORDINATE DEMOLITION WORK WITH OWNER TO ENSURE THAT IMPACTS ON THE BALANCE OF THE BUILDING ARE HELD TO A MINIMUM.
- 11. PREPARE ALL SURFACES TO RECEIVE THE NEW WORK AND FINISHES OF THE CONTRACT.
- 12. THE CONTRACTOR SHALL DESIGN, PROVIDE, INSTALL AND MAINTAIN ANY AND ALL TEMPORARY BRACING AS REQUIRED TO ENSURE THE STABILITY OF THE BUILDING ASSEMBLY AND/OR ANY SYSTEMS AND/OR SUB-ASSEMBLIES AND/OR SYSTEMS APPURTENANT THERETO UNTIL SAID ASSEMBLY AND/OR SUB-ASSSEMBLIES ARE COMPLETE, SELF-SUPPORTING AND/OR STABLE.

## **DEMOLITION PLAN KEY NOTES:**

(TYPICAL THIS SHEET ONLY)

FRONT PORCH TO REMOVED. SALVAGE RAILING AND SUPPORT POSTS FOR REINSTALLATION.

(D2) REMOVE WINDOW COMPLETE

- (D3) REMOVE WALL COMPLETE, INCLUDING ELECTRICAL TO PANEL
- (D4) REMOVE BATHROOM COMPLETE, INCLUDING ELECTRICAL AND PLUMBING
- (D5) REMOVEKITCHEN COMPLETE, INCLUDING ELECTRICAL AND PLUMBING
- REMOVE INTERIOR DRYWALL/PLASTER AND FLOOR FINISHES COMPLETE, ALL HVAC AND ELECTRICAL
- (D7) REMOVE REAR PORCH COMPLETE, INCLUDING FOUNDATIONS
- (D8) REMOVE FOUNDATION COMPLETE
- EXISTING HARDWARE/LOCK SET TO BE REMOVE. DOOR SLAB TO SALVAGED FOR REINSTALLATION, PREP TO RECEIVE NEW HINGES/LOCKSET,
- (11) REMOVE SIDING COMPLETE AND SALVAGE FOR RE-USE

# **DEMOLITION KEY:**

WALL TO REMAIN

WALL TO BE REMOVED

DOOR TO REMAIN

DOOR, FRAME AND HARDWARE TO BE REMOVED UNLESS NOTED OTHERWISE

DESIGNATED ITEM TO REMAIN

\_\_\_\_\_ DESIGNATED ITEM TO REMAIN
\_\_\_\_ \_\_\_ DESIGNATED ITEM TO BE REMOVED

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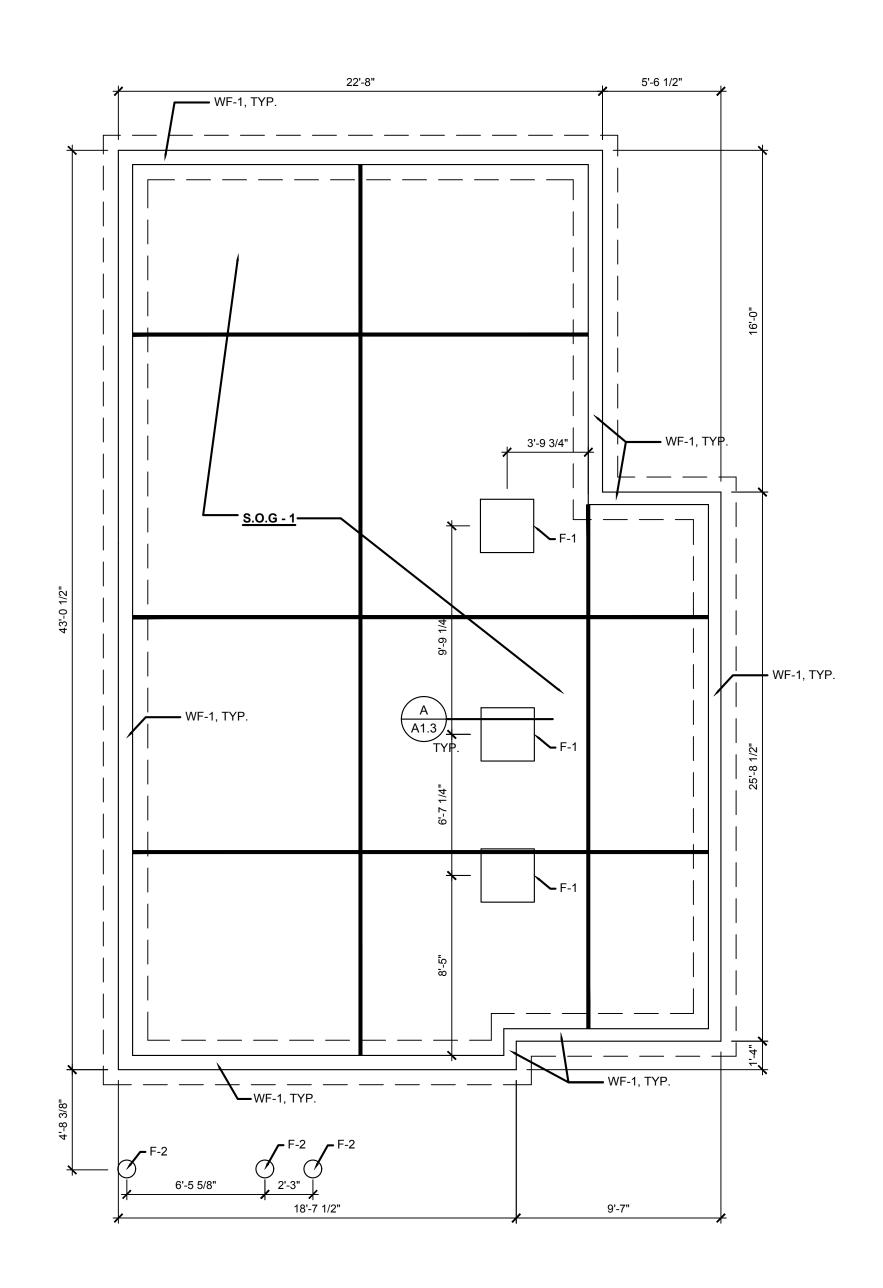
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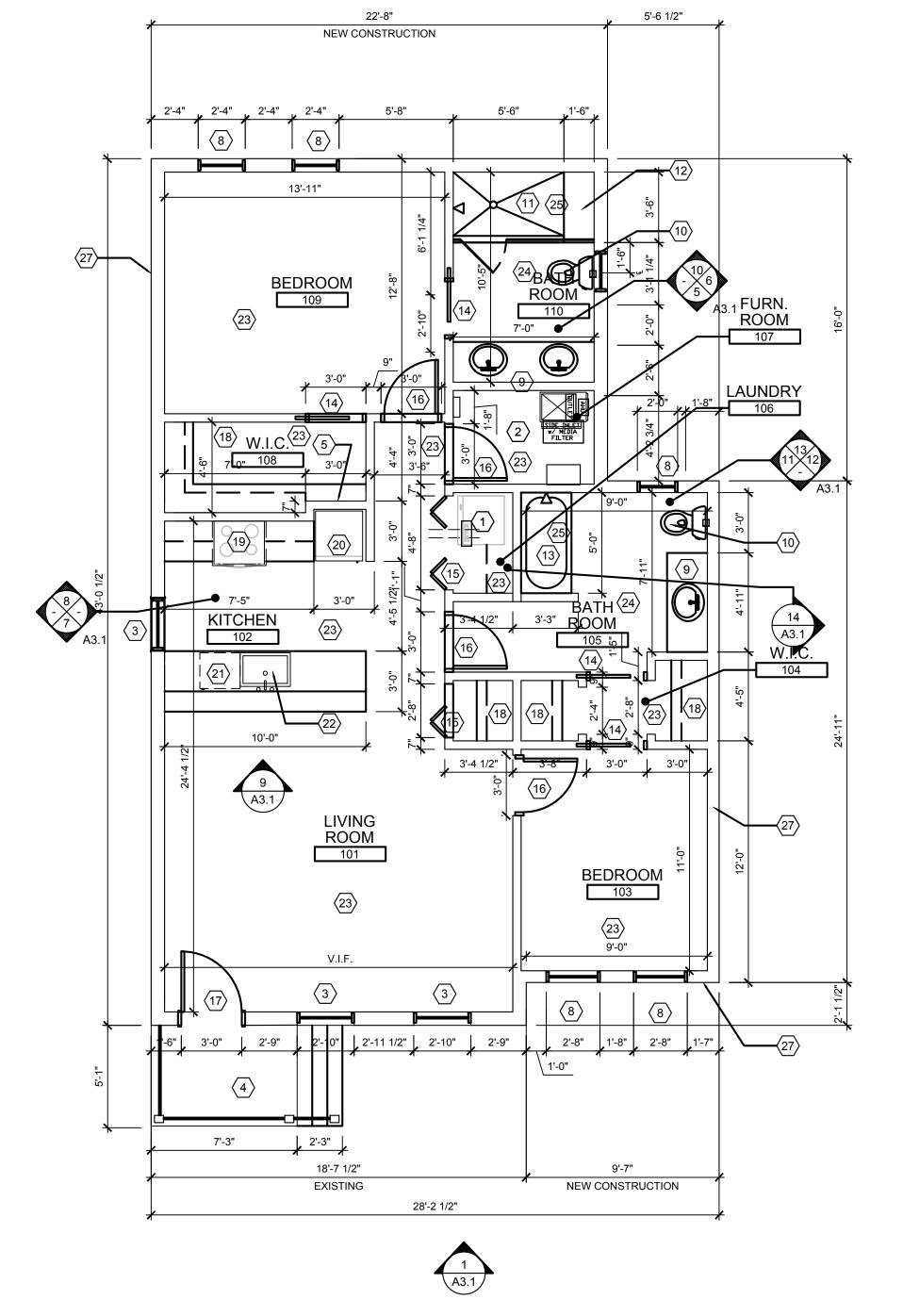
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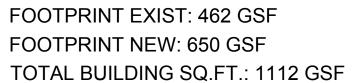
Sheet Title :
DEMOLITION
FLOOR PLAN

Project No. : 2019006

Sheet No.:
D1.1











## **GENERAL FLOOR PLAN NOTES:**

- 1. THIS DRAWING IS DIAGRAMMATIC AND SHOULD BE USED TO DETERMINE THE DESIGN INTENT. THE CONTRACTOR IS RESPONSIBLE FOR THE COMPLETE SET OF WORK AS INDICATED AND SHALL FIELD VERIFY ALL WORK, COORDINATE ALL DRAWINGS / NEW WORK AND SHALL NOTIFY ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES IN THE DOCUMENTS BEFORE PROCEEDING. FAILURE TO DO SO WILL RESULT IN THE CONTRACTOR TAKING FULL RESPONSIBILITY AND LIABILITY FOR SAID DISCREPANCIES.
- 2. ALL DIMENSIONS ARE SHOWN FROM FINISH FACE TO FINISH FACE OF PARTITION UNLESS OTHERWISE NOTED.
- 3. WALL THICKNESS' ARE NOMINAL NOT ACTUAL DIMENSIONS. SEE WALL SCHEDULE FOR ACTUAL DIMENSIONS.
- 4. ALL WORK SHALL BE DONE IN ACCORDANCE WITH ALL LOCAL, STATE, COUNTY CODE REGULATIONS, O.S.H.A., AND THE AMERICAN WITH DISABILITIES ACT (ADA). REFER TO THE CODE PLAN FOR MORE INFORMATION.
- PROVIDE POSITIVE SLOPE TO ALL FLOOR DRAINS WHILE KEEPING FLOOR LEVEL AT WALL BASE CONDITION.
- 6. ALL FIXTURES AND HARDWARE TO BE SATIN SILVER FINISH
- 7. ALL FLOOR/WALL BASES TO BE 1x4 PAINTED WHITE, EXCEPT AT LOCATIONS OF WALL TILE THEN PROVIDE TILE BASE.
- 8. ALL INTERIOR WALLS TO BE PAINTED WHITE, SATIN FINISH ON WALL AND FLAT FINISH ON CEILING

# FLOOR PLAN KEY NOTES:

#### (TYPICAL THIS SHEET ONLY)

- STACKED WASHER/DRYER; PROVIDE GAS AND WATER CONNECTION(SUPPLY AND DRAIN). PROVIDE STORAGE SHELVES
- FURNACE AND ON-DEMAND HW SYSTEM , PROVIDE AC CONDENSER OUTSIDE AS REQ'D
- NEW WINDOW, PELLA ARCHITECTURAL WOOD SERIES OR SIMILAR. STYLE 2 OVER 2 GRID.
- FRONT PORCH, USE EXISTING SALVAGED MATERIALS, REPALCE WITH SIMILAR IF DAMAGED,
- 5 TENSION CABLES TO BE INSTALLED PRIOR TO WALL BACKFILL, 10 KIPOS
- 6 NOT USED
- $\overline{\langle 7 \rangle}$  NOT USED
- (1) NOT 0025
- 8 NEW WINDOW, PELLA ARCHITECTURAL WOOD SERIES OR SIMILAR. STYLE 2 OVER 2 GRID.
- 9 SHADOW GRAY QUARTZ COUNTER TOP WITH DROP IN SINK.
- (10) DUAL FLUSH TOILET, BY TOTO
- SHOWER, WHITE SHOWER PAN WITH SUBWAY TILE ON THREE SIDES, AND GLASS SHOWER ENCLOSURE,
- \$\langle 12 \rangle SHOWER SEAT FINISH W/ SUBWAY TILE
- $\langle 13 \rangle$  BATHTUB, PROVIDE GLASS SHOWER ENCLOSURE
- POCKET DOOR, SOLID CORE WOOD SLAB. STAIN TO MATCH MILANO DOOR VENEER
- (15) BI-FOLD DOOR, MFG: MILANO, FIN; GRAY OAK, STYLE; SL131-BFD-12
- SOLID CORE WOOD SLAB. STAIN TO MATCH MILANO DOOR VENEER. BATHROOM LOCK SET OR PRIVACY AS REQ'D.
- NEW 3-0x7'0 ENTRY DOOR, PROVIDE ENTRY SET
- (18) CLOSET WITH HANGER ROD AND SHELF
- (19) GAS STOVE, WITH MICROWAVE ABOVE IN CABINET, PROVIDE
- RE-CIRCULATION EXHAUST

  (20) REFRIGERATOR, PROVIDE WATER CONNECTION.
- 21) DISHWASHER
- SHADOW GRAY QUARTZ COUNTER TOP WITH STAINLESS STEEL SINK, W/ 1/2 HP GARBAGE DISPOSAL, PROVIDE GFCI IN KITCHEN AS REQUIRED, GARBAGE DISPOSAL TO HAVE AIR-SWITCH
- 23 FLOORING: AQUASEAL 24, 12MM BLUE SANDS PINE LAMINATE
- BATHROOM FLOORING: DALTILE YACHT CLUB, BRIDGE DECK YC02; SIZE
- SUBWAY TILE: DALTILE, COLOR WHEEL COLLECTION CLASSIC, FINISH WHITE; SIZE 3X6
- 26 INSTALL SHOE SHELVES ON WALL.
- 27 1 HR WALL, UL305 AT LOCATIONS WHERE WALL IS WITH IN 5' OR LESS OF THE PROPERTY LINE

# FOUNDATION SCHEDULE

WF-1: 24" WIDE x 12" THICK FOOTING, REINF. W/ (3) #5 T&B LONGITUDINAL. CONTINUOUS. STEM WALL 10" WIDE. SEE SECTIONS FOR REINFORCING

\* FOUNDATIONS ARE DESIGNED TO BEAR ON UNDISTURBED NATIVE/NATURAL SOILS ON ENG'R FILL HAVING A NET ALLOWABLE BEARING CAPACITY ≥ 2000 PSF V.I.F.

- **F-1:** 2'-0" x 2'-0" x 1'-4" THICK FOOTING w/(4) #5 BOT E.W.
- 16" DIA x 42" DEEP FOOTING w/(6) #5 VERTICAL + (3) #3 TIES EQ.

S.O.G - 1: 4" S.O.G. + 6x6-W2.4xW2.4 WWF PLACED 1" FROM TOP OF SLAB, ON VAPOR BARRIER, ON COMP. FILL@ 10'-0" MAX O.C., WWF LAP A MIN. 1 - COMPLETE GRID.

BACKFILL FOUNDATIONS UNIFORMLY AND WITH APPROVED BACKFILL MATERIALS COMPACTED TO 95% MIN. OF THE STANDARD PROCTOR.

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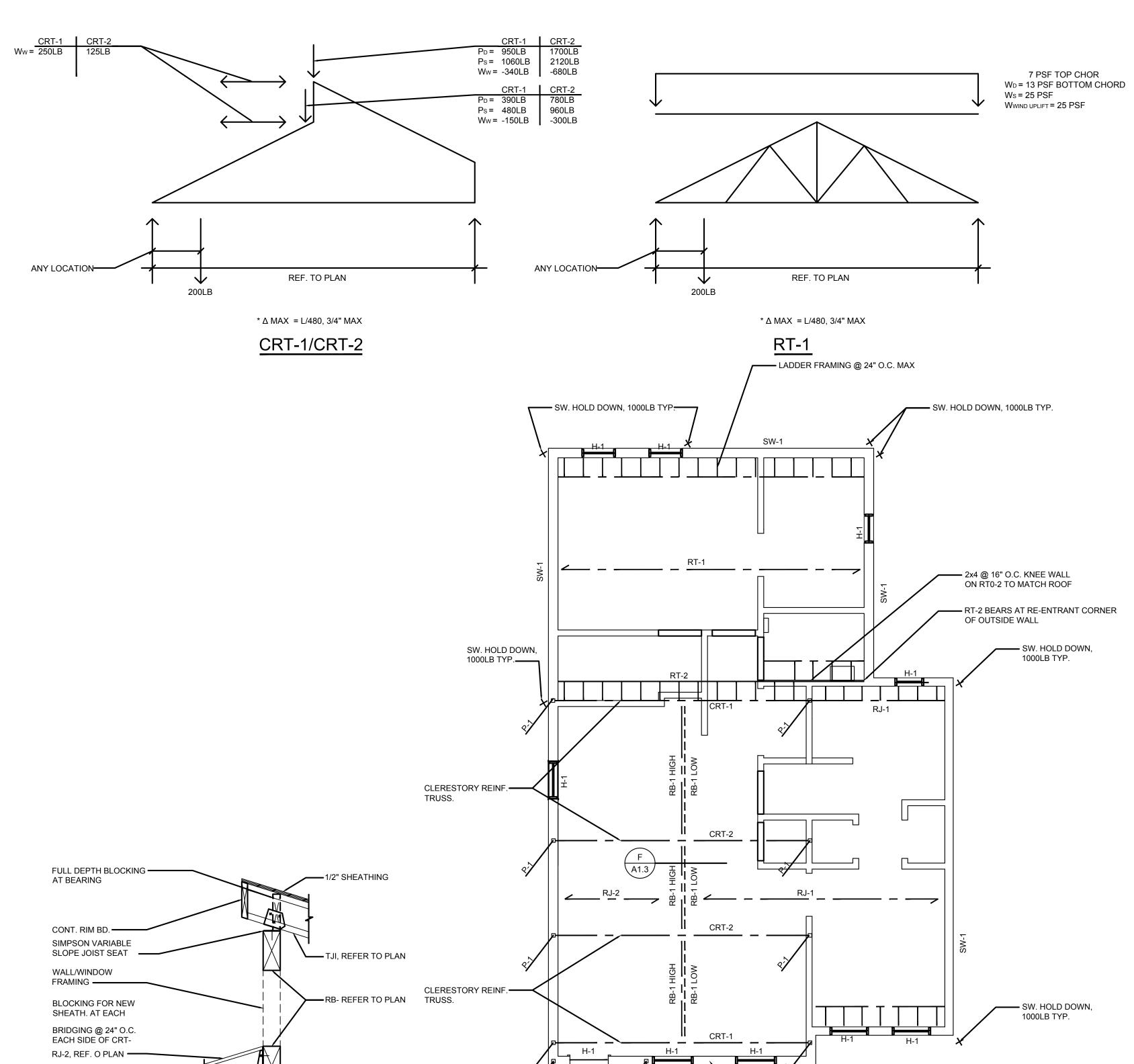
1ST FLOOR +

2ND FLOOR

PLANS

Project No. : 2019010

Sheet No.:



SIMPSON HANGER

STRUCTURAL DETAIL

A1.2/3 SCALE: 3/4" = 1'-0"

POST, REF. TO —

SIMPSON BASE -

DO NOT LOCATE C.J.— OVER FOOTING

S.O.G. REF. TO PLAN-

WWF, REF TO PLAN

VAPOR BARRIER

COMP. GRAN. FILL •

STRUCTURAL DETAIL

A1.2 SCALE: 3/4" = 1'-0"

—(2) #5 BARS EA. SIDE OF

-REINFORCEMENT, REF

—FOOTING, REF TO PLAN

FÓOTING, TYP.

TO PLAN

PLAN.

PLATE

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JOIST BRIDGING TO BE 1X3 NOMINAL LUMBER IN AN X-CONFIGURATION

6. TRUSS BRIDGING TO BE DESIGNED BY TRUSS ENG'R

 GABLE END TRUSSES TO BE DESIGNED FOR H/600 MAX WIND LOAD DEFLECTION.

## FRAMING SCHEDULE

2J-1: 11-7/8" TJI 230 @ 16" O.C. + 1/2" SHEATHING, BRIDGING @ THIRD POINTS

**RJ-2:** 2X8 @ 16" O.C. + 1/2" SHEATHING, BRIDGING AT MID-SPAN

**RB-1:** 9-1/4" x 3-1/2" PSL 2.0E

WOOD ROOF TRUSS @ 16" O.C. + 1/2" SHEATHING, BRIDGING AT SPAN/3, REF TO LOAD DIAGRAM

WOOD ROOF TRUSS + 1/2" SHEATHING, BRIDGING AT SPAN/3
POINTS REF. TO RT-1 LOAD DIAGRAM AND AD 5 PSF DEAD

WOOD ROOF TRUSS + 1/2" SHEATHING, BRIDGING AT SPAN/3 POINTS REF. TO RT-1 LOAD DIAGRAM

P-1:
(3) 2x6 ADD ADD'L AS REQ'D FOR FULL BRG. OF CRT-1/CRT-2
\* PROVIDE UPLIFT ANCHORS TO FOUNDATION & CRT-1/CRT-2
R=500LB/1000LB MIN.

P-2: 6x6 PRESSURE TREATED WOOD POST W/ SIMPSON BASE PLATE

(3) 2X8 HEADER + OUTSIDE
(2) 2X6 CRIPPLE+ 8d @ 12" O.C. MAX TYP.
(2) 2X6 KING POST

<u>W-1:</u> 2x6 @ 16" O.C. W/ 1/2" SHEATHING, WOLM. SILL PLATES

(3) 2x6 CHORDS 2x6 @ 16" O.C. FIELD

1/2" SHEATHING 8d NAILING OR STAPLES @ 6" O.C. PERIMETER, 12" O.C. FIELD

<u>CRT-1:</u> CLEARSTORY ROOF TRUSS, REF TO LOAD DIAGRAM

CRT-2: CLEARSTORY ROOF TRUSS, REF TO LOAD DIAGRAM

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Sheet Title :
ROOF FRAMING PLANS
AND DETAILS

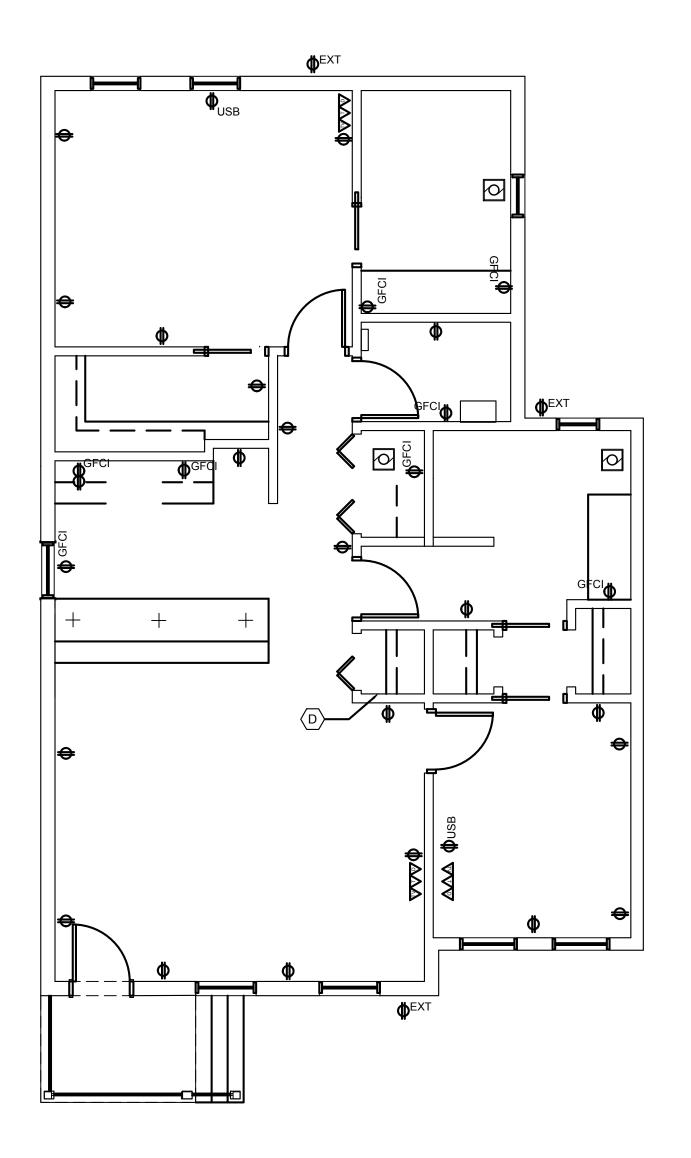
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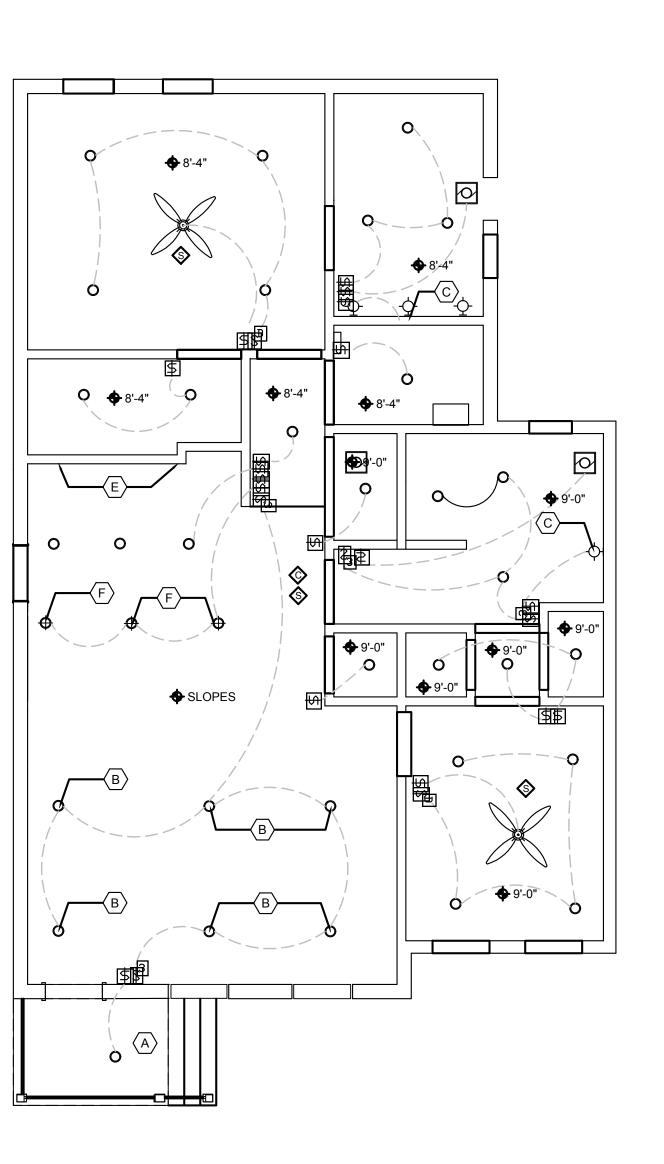
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- RT-3 INFILL AS REQ'D - LADDER FRAMING NOT

SHOWN FOR CLARITY





# GENERAL REFLECTED CEILING PLAN NOTES:

- 1. SWITCH SYMBOL INDICATES THE LOCATION FOR SWITCHING ALL FIXTURES WITHIN THAT ROOM UNLESS NOTED OTHERWISE. REFER TO ELECTRICAL DRAWINGS FOR FURTHER INFORMATION AS REQUIRED.
- 2. CONTRACTOR TO CENTER ALL CEILING MOUNTED ITEMS (i.e., RECESSED LIGHT FIXTURES, SMOKE DETECTORS, FIRE SUPPRESSION HEADS) WITHIN THE ASSOCIATED CEILING TILE AS SHOWN. COORDINATE FINAL LOCATION WITH THE APPROPRIATE MECHANICAL, ELECTRICAL, FIRE ALARM, AND FIRE SUPPRESSION DRAWINGS AS REQUIRED.
- 3. REFER TO WALL TYPES FOR WALLS THAT PENETRATE CEILINGS.
- 4. REFER TO MECHANICAL HVAC PLANS FOR DIFFUSER / GRILLE SIZES.
- 5. FOR LIGHT FIXTURE TYPES SEE ELECTRICAL LIGHTING PLANS.
- REFER TO DIMENSIONS ON REFLECTED CEILING PLAN TO LOCATE / LAYOUT CEILING GRID AND LIGHT FIXTURES. 7. ACCESS PANELS TO BE INDEPENDENTLY MOUNTED, DO NOT SUPPORT ON CEILING GRID. COORDINATE SIZE, QUANTITY AND LOCATIONS WITH ARCHITECTURAL AND MECHANICAL
- 8. ALL ELECTRICAL OUTLETS TO BE 18" AFF, UNLESS AT KITCHEN/BATHROOM COUNTER, CONTACTOR TO INSTALL GFCI AND SPACING BETWEEN OUTLETS PER MI, ELEC. CODE.

# REFLECTED CEILING PLAN KEY NOTES:

(TYPICAL THIS SHEET ONLY)

- A PAINT SND REPAIR UNDERSIDE OF PORCH SOFFIT AS REQ'D.
- B ADJUSTABLE CAN IN SLOPED CEILING
- (C) INSTALL WALL SCONES BETWEEN BATHROOM MIRRORS
- D INSTALL LEVITON 49605-14P NETWORK BOX. ALL INCOMING IT/TELECOMMUNICATION/DATA TO CONNECT THROUGH DATA BOX.
- E UNDER CABINET LED LIGHTING
- F PENDANT FIXTURE, COORDINATE BOTTOM OF FIXTURE W SLOPED CEILING

REFLECTED CEILING LEGEND						
SYMBOL	DESCRIPTION					
	GYPSUM BOARD CEILING AND/OR SOFFIT					
0	RECESSED DOWNLIGHT FIXTURE					
•	PENDANT LIGHT FIXTURE					
<b>©</b>	DOMED LIGHT FIXTURE					
X	CEILING FAN W/ LED LIGHTS					
Э	WALL SCONCE FIXTURE					
<b>\$</b>	SMOKE DETECTOR, INTER CONNECTED					
<b>\$</b>	CO2 DETECTOR					
<b>♦</b> HEIGHT	ELEVATION ABOVE FINISHED FLOOR					
Ф	POWER OUTLET					
	DATA RECEPTACLE (COMBINE W/DATA,TELE,TV IN ON RECEPTACLE)					
$\sqrt{1}$	TELEPHONE RECEPTACLE (COMBINE W/DATA,TELE,TV IN ON RECEPTACLE)					
$\nabla$	TV/COAX RECEPTACLE(COMBINE W/DATA,TELE,TV IN ON RECEPTACLE)					
Ø	EXHAUST FAN					
	LIGHT SWITCH W/ DIMMER					
	3 WAY SWITCH					
\$	WALL SWITCH					

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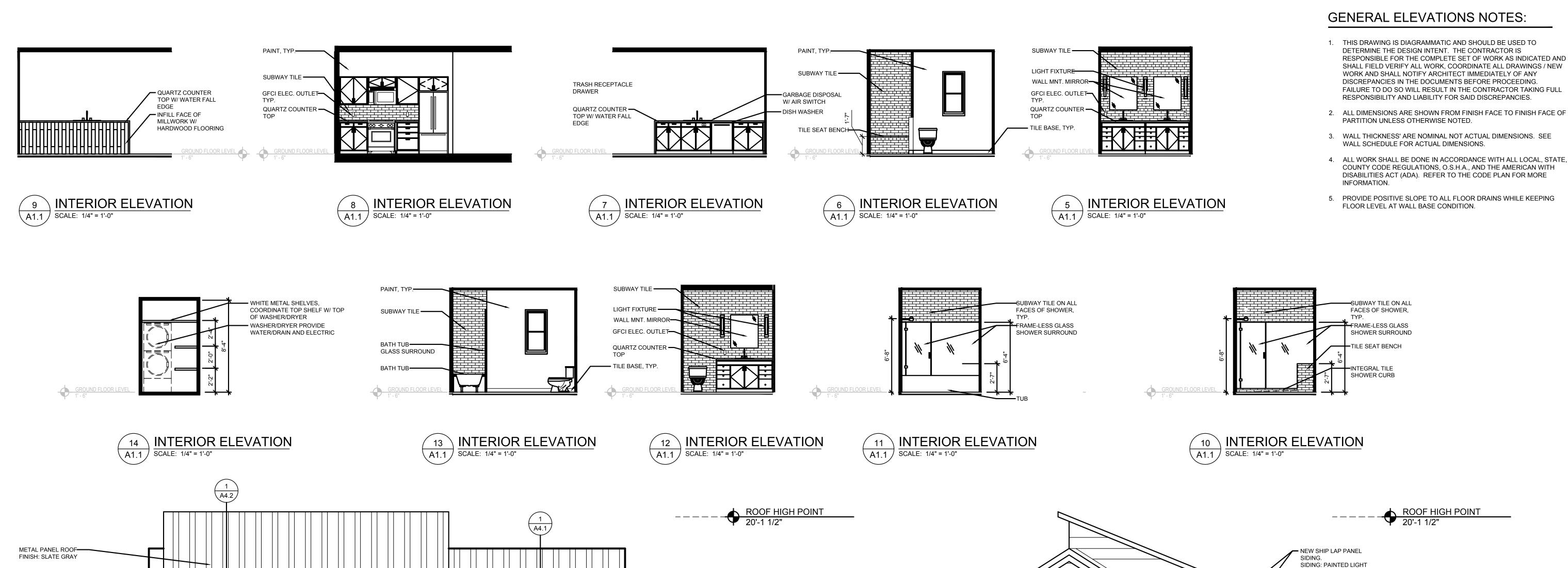
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REFLECTED
CEILING PLAN

2019006

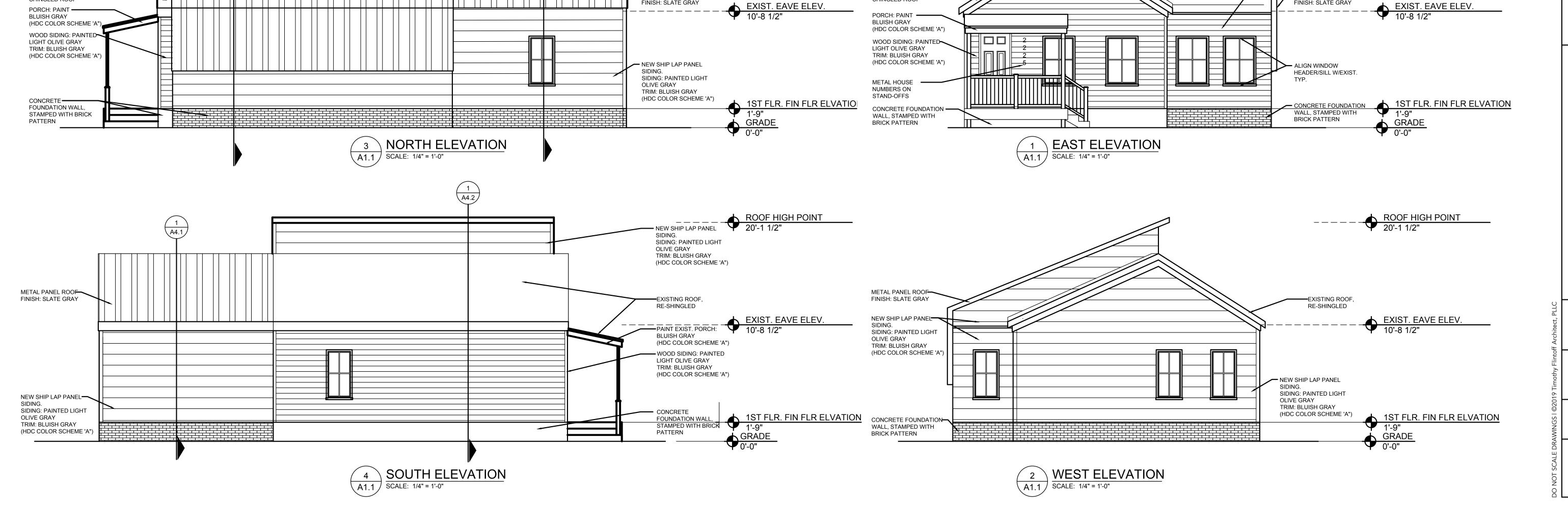


► METAL PANEL ROOF

FINISH: SLATE GRAY

ROOF PROFILE TO — MATCH ORIGINAL,

SHINGLED ROOF



ROOF PROFILE TO

MATCH ORIGINAL,

SHINGLED ROOF

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5. PROVIDE POSITIVE SLOPE TO ALL FLOOR DRAINS WHILE KEEPING

OLIVE GRAY

TRIM: BLUISH GRAY (HDC COLOR SCHEME 'A")

METAL PANEL ROOF,

FINISH: SLATE GRAY

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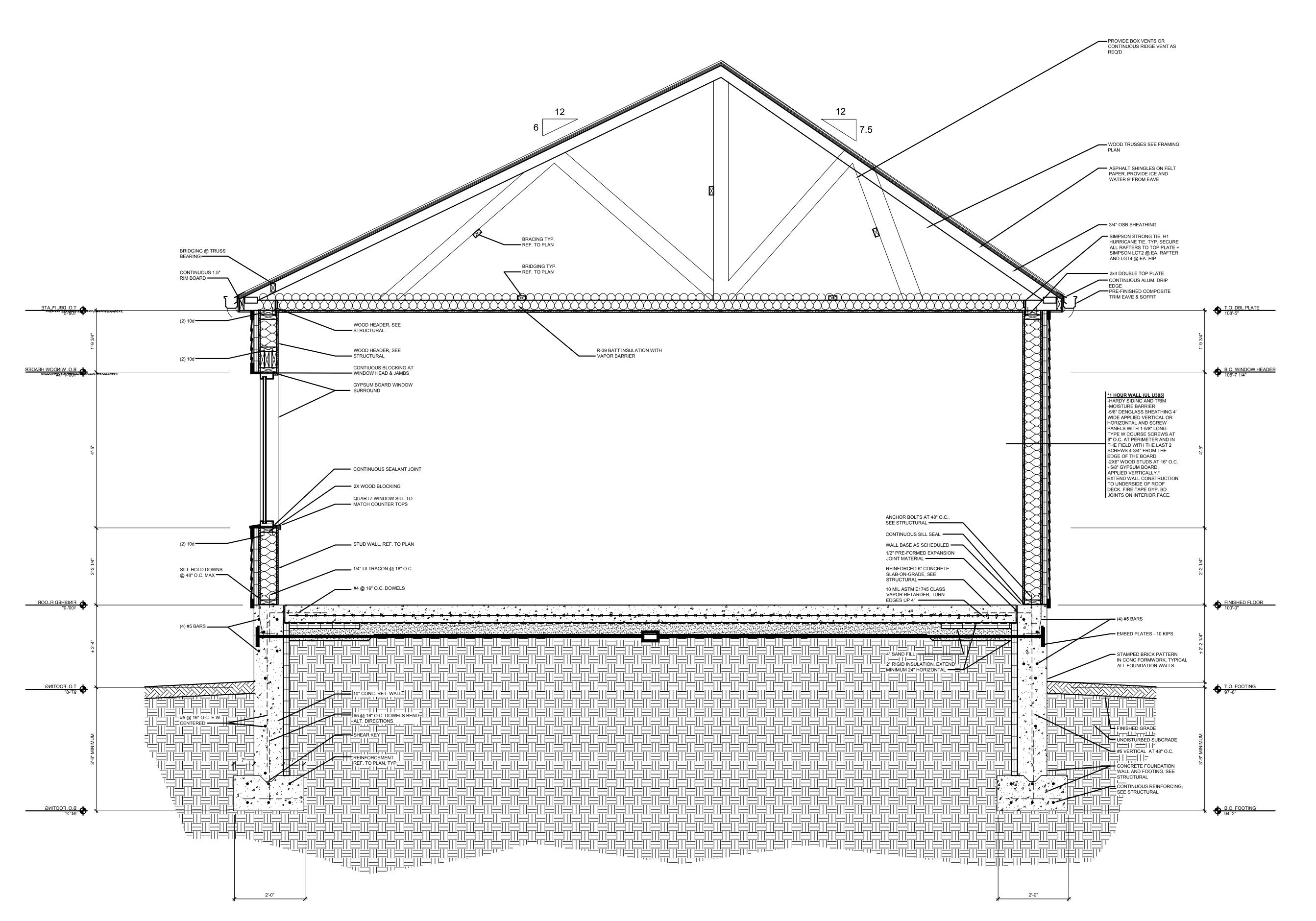
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EXTERIOR

RENDERING

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