

Coolidge Terminal Replacement Project
Environmental Assessment
January 2023

APPENDIX B

TRAFFIC STUDY



INNOVATIVE IDEAS
EXCEPTIONAL DESIGN
UNMATCHED CLIENT SERVICE

OFFICE MEMORANDUM

DATE: August 9, 2022
TO: Donna C. Rice, LEED Green Associate
Detroit Building Authority
FROM: Charles H. Fawcett, P.E., PTOE
SUBJECT: DDOT Coolidge Bus Maintenance Facility – Traffic Impact Study

INTRODUCTION

This memorandum presents the methodology, analysis, findings, and recommendations for the Traffic Impact Study (TIS) of the proposed Detroit Department of Transportation (DDOT) Coolidge Bus Operations and Maintenance Facility. The bus facility is proposed to be located on Schaefer Highway between Schoolcraft Road and Lyndon Street. The facility is designed to be built with an initial capacity of 144 buses with the potential to be expanded in the future to accommodate 216 buses at full build-out.

The purpose of this TIS is to determine the expected traffic impacts of the proposed bus facility on nearby intersections. The study area was determined to include the intersections identified in Figure 1.

The facility is expected to be built and operational in 2025. At opening, the facility is expected to operate at its initial capacity, accommodating 144 buses. For the purposes of this study, it is assumed that the facility will be expanded to full capacity within 20 years of opening – by 2045. This study analyzed Existing conditions at the study intersections, opening year 2025 conditions including the No-Build conditions, and Initial Build conditions; and the horizon year 2045 Full Build conditions. Trafficware’s Synchro 11 (the latest version available) software program was used to perform intersection capacity analysis.

DATA COLLECTION

DLZ collected the 24-hour traffic counts on a typical weekday – Tuesday June 15, 2021. The turning movement counts were collected at the following 9 intersections which will collectively comprise the study area shown in Figure 1. Turning movement data reports are included in Appendix A.

1. Schaefer Highway and Lyndon Street
2. Schaefer Highway and Schoolcraft Road
3. Schaefer Highway and Grand River Avenue
4. Schaefer Highway and I-96 Service Road
5. Schaefer Highway and Jefferies Service Drive
6. Grand River Avenue and I-96 Service Road
7. Grand River and Jefferies Service Road

- 8. Schoolcraft Road and Ward Avenue
- 9. Meyers Road and Kendall Street

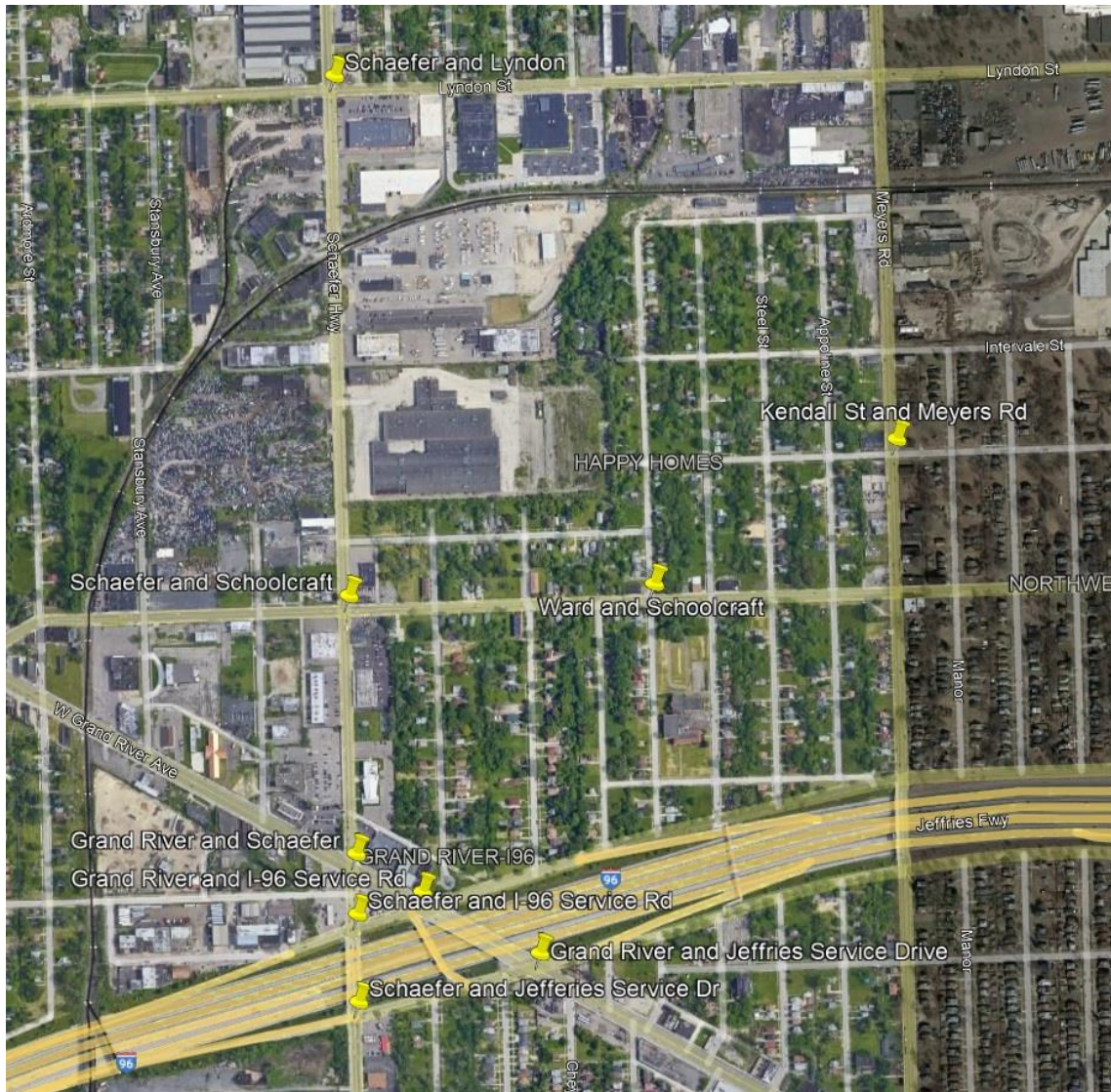


Figure 1 - Study Area Map

To determine the hours for capacity analysis, 24-hours of traffic data at the intersection of Schaefer Highway and Grand River Avenue were compared with the forecasted hourly site trips. (The development of forecasted site traffic is discussed below).

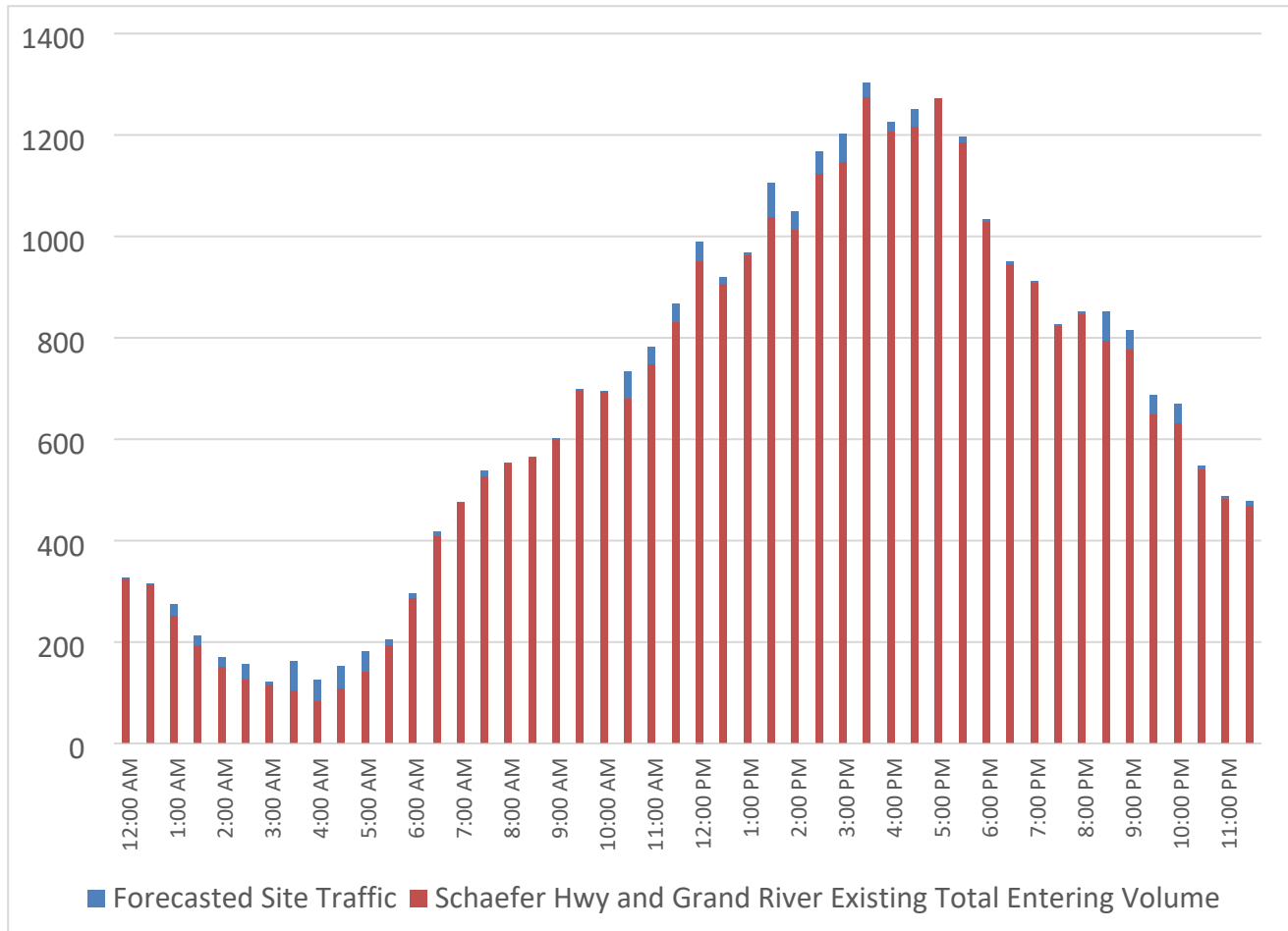


Figure 2 - Schaefer Highway and Grand River Avenue and Site Traffic Study

The traffic volume profile of the Schaefer Highway and Grand River Avenue intersection shows one peak period between approximately 3:30 PM and 5:30 PM. Site traffic during this period is expected to be minimal, however. Site traffic is expected to peak in early afternoon and late evening. Therefore, the peak hours for the purposes of this study were determined to be 1:30 PM – 2:30 PM (Midday) and 8:00 PM – 9:00 PM (Evening). Turning movement data at the remaining study intersections during these hours was used in the analysis described below and illustrated in Exhibit 1.

In addition to existing traffic counts, traffic signal timing information was obtained from the City of Detroit Traffic Engineering Department (TED) and from the Michigan Department of Transportation (MDOT) for their respective study area intersections controlled by traffic signals.

EXISTING CONDITIONS ANALYSIS

Synchro models were developed for each peak hour – Midday and Evening. The existing models were developed according to the roadway network, lane configurations, traffic control plans, and the existing traffic volumes, heavy vehicle percentages and peak hour factors. Existing lane configurations and traffic control are illustrated in Exhibit 2. Intersection capacity analyses were performed for the study intersections during the weekday Midday and Evening peak hours to determine their levels of service (LOS). LOS is a qualitative measure describing operational conditions within a traffic stream, generally in terms of such service measures as speed and travel time, freedom to maneuver, traffic interruptions, and comfort and convenience. Operational LOS reflects delays experienced by the motorist and is designated a letter grade of A through F. LOS A represents the best operating conditions and LOS F the worst. LOS D or better is considered acceptable for future intersections in this study. The Highway Capacity Manual (HCM) defines level of service for signalized and unsignalized intersections as a function of the average vehicle control delay in seconds per vehicle (sec) as summarized in Table 1 below.

Table 1 - LOS Criteria for Control Delay at Intersections

Level of Service Grade	Signalized Intersection (sec)	Unsignalized Intersection (sec)
A	≤10	≤10
B	10-20	10-15
C	20-35	15-25
D	35-55	25-35
E	55-80	35-50
F	≥80	≥50

The Existing condition Synchro models were used to determine the intersection LOS and average delay per vehicle. Those results are summarized in Table 2. The Existing conditions analysis found that all movements at the study intersections operate acceptable levels of service of D or better. Synchro intersection capacity analysis reports are in Appendix B.

Table 2 - Existing Conditions Capacity Analysis Results

Level of Service Summary – Midday Peak Hour													
Intersection	LOS/Delay (s/veh)												
	Lane Group												Overall
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Schaefer Hwy & Lyndon St	28.6 C	36.3 D		38.8 D	30.0 C		10.1 B	18.5 B		5.3 A	9.0 A		19.4 B
Schaefer Hwy & Schoolcraft Rd	25.5 C	22.4 C		25.5 C	22.2 C		7.3 A	23.5 C		6.4 A	22.6 C		22.3 C
Schaefer Hwy & Grand River Ave	10.5 B	7.1 A		8.7 A	6.5 A		26.2 C	37.4 D	6.9 A	53.5 D	28.8 C		17.7 B
Schaefer Hwy & I-96 Service Rd				30.9 C	25.3 C		4.6 A	2.6 A			10.1 B		10.3 B
Schaefer Hwy & Jeffries Service Dr		26.0 C						10.7 B		5.6 A	5.0 A		14.9 B
Grand River Ave & I-96 Service Rd					23.8 C	11.2 B	52.2 D	1.2 A			7.0 A	1.1 A	11.9 B
Grand River Ave & Jeffries Service Dr		31.4 C						3.9 A			2.3 A		11.2 B
Left Turn Bridge & Jeffries Service Dr		3.2 A								0.1 A			2.3 A
Schoolcraft Rd & Ward Ave	7.6 A	0.0 A		7.6 A	0.0 A			9.5 A			10.3 B		0.9 A
Meyers Rd & Kendall St		12.0 B			14.1 B		7.9 A			8.0 A			1.1 A

Level of Service Summary – Evening Peak Hour													
Intersection	LOS/Delay (s/veh)												
	Lane Group												Overall
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Schaefer Hwy & Lyndon St	22.3 C	18.7 B		23.8 C	22.8 C		5.1 A	9.4 A		5.1 A	8.3 A		12.4 B
Schaefer Hwy & Schoolcraft Rd	24.1 C	25.9 C		24.1 C	23.3 C		7.1 A	19.2 B		7.4 A	18.4 B		20.0 B
Schaefer Hwy & Grand River Ave	8.9 A	6.9 A		8.1 A	5.8 A		24.6 C	36.4 D	10.4 B	40.8 D	28.7 C		16.6 B
Schaefer Hwy & I-96 Service Rd				33.1 C	27.8 C		2.8 A	2.0 A			8.3 A		9.7 A
Schaefer Hwy & Jeffries Service Dr		25.6 C						8.8 A		45.5 D	3.7 A		14.6 B
Grand River Ave & I-96 Service Rd					25.1 C	11.7 B	52.1 A	0.6 A			6.9 D	0.8 A	12.0 B
Grand River Ave & Jeffries Service Dr		31.5 C						3.8 A			2.2 A		11.0 B
Left Turn Bridge & Jeffries Service Dr		2.0 A								0.1 A			1.4 A
Schoolcraft Rd & Ward Ave	7.7 A	0.0 A		0.0 A				0.0 A			9.5 A		0.5 A
Meyers Rd & Kendall St		11.9 B			14.1 B		7.8 A			8.0 A			1.1 A

NO BUILD CONDITIONS ANALYSIS

The Year 2025 No-Build scenario Synchro models were developed based on 1% annual background traffic growth rate at all intersections, the existing roadway geometries and signal timing plans. The year 2025 forecasted turning movement volumes used in the No Build analysis are illustrated in Exhibit 3. The analysis found that one movement at the intersection of Shaefer Highway and Grand River Avenue is expected to operate at LOS E in this future scenario without the proposed bus facility. This movement is addressed in the Build scenario with traffic signal timing modifications. The No Build condition capacity analysis results are summarized below in Table 3. Synchro intersection capacity analysis reports are included in Appendix B.

Table 3 – 2025 No Build Condition Capacity Analysis Results

Level of Service Summary – Midday Peak Hour													
Intersection	LOS/Delay (s/veh)												
	Lane Group												Overall
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Schaefer Hwy & Lyndon St	28.5 C	36.6 D		39.9 D	30.2 C		7.2 A	17.8 B		5.4 A	9.2 A		19.2 B
Schaefer Hwy & Schoolcraft Rd	25.7 C	23.4 C		25.7 C	22.6 C		7.4 A	24.5 C		6.5 A	23.2 C		23.0 C
Schaefer Hwy & Grand River Ave	11.0 B	7.4 A		8.9 A	6.8 A		26.0 C	37.1 D	6.6 A	58.6 E	28.6 C		18.0 B
Schaefer Hwy & I-96 Service Rd				30.4 C	24.9 C		5.1 A	2.8 A			10.9 B		10.6 B
Schaefer Hwy & Jeffries Service Dr		26.2 C						11.3 B		5.9 A	5.3 A		15.2 B
Grand River Ave & I-96 Service Rd					24.0 C	11.1 B	52.9 D	1.2 A			7.0 A	1.2 A	12.0 B
Grand River Ave & Jeffries Service Dr		31.9 C						4.0 A			2.3 A		11.4 B
Left Turn Bridge & Jeffries Service Dr		3.5 A								0.1 A			2.5 A
Schoolcraft Rd & Ward Ave	7.6 A	0.0 A		7.6 A	0.0 A			9.6 A			10.4 B		0.9 A
Meyers Rd & Kendall St		12.2 B			14.5 B		7.9 A			8.0 A			1.1 A

Level of Service Summary – Evening Peak Hour													
Intersection	LOS/Delay (s/veh)												
	Lane Group												
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Overall
Schaefer Hwy & Lyndon St	22.2 C	18.8 B		23.8 C	23.0 C		5.2 A	9.6 A		5.2 A	8.5 A		12.6 B
Schaefer Hwy & Schoolcraft Rd	24.1 C	26.0 C		24.1 C	23.6 C		7.1 A	19.7 B		7.5 A	18.7 B		20.3 C
Schaefer Hwy & Grand River Ave	9.0 A	7.1 A		8.2 A	6.0 A		24.3 C	36.6 D	10.4 B	43.1 D	28.6 C		16.8 B
Schaefer Hwy & I-96 Service Rd				33.1 C	28.3 C		2.9 A	2.1 A			8.9 A		10.3 B
Schaefer Hwy & Jeffries Service Dr		25.6 C						9.2 A		46.0 D	3.9 A		14.8 B
Grand River Ave & I-96 Service Rd					25.1 C	11.6 B	53.5 D	0.7 A			6.9 A	0.8 A	12.1 B
Grand River Ave & Jeffries Service Dr		31.3 C						3.8 A			2.2 A		11.0 B
Left Turn Bridge & Jeffries Service Dr		2.0 A								0.1 A			1.4 A
Schoolcraft Rd & Ward Ave	7.7 A	0.0 A		0.0 A				0.0 A			9.5 A		0.5 A
Meyers Rd & Kendall St		10.6 B			10.2 B		7.8 A			7.5 A			0.9 A

TRIP GENERATION, DISTRIBUTION AND ASSIGNMENT

The proposed development is an operations and maintenance facility for DDOT’s public bus transit routes. The facility has been designed to accommodate a total of 216 buses. It is expected that at opening in 2025, the facility will operate at an Initial Build capacity and is assumed to be operating at Full Build capacity within 20 years. Therefore, trip generation, distribution and assignment of site traffic was performed for the opening year 2025 Initial Build Scenario and forecasted horizon year of 2045 Full Build Scenario.

Access to the site is proposed on Schaefer Highway with two driveways – one for buses and one for employees. An emergency access driveway is proposed on the east side of the site aligned with the intersection of Ward and Kendall Avenues. This driveway will be gated and used in emergency situations only. No site traffic was assigned to the emergency access driveway. The proposed site plan is shown below in Figure 3.

City traffic engineering department officials requested that a consolidation of the two proposed driveways be explored. Therefore, two site access scenarios were analyzed, one according to the site plan and one assuming one consolidated driveway. Both are discussed in the Build Conditions Analysis Section below.



Figure 3 - Proposed Site Plan

Trip Generation

The design team modeled the daily operations at the facility on an hourly basis which included the number of employees and buses coming in and leaving site each hour. PDF versions of the Shift Analyses for Opening Day 2025 Initial Build Scenario of 144 buses and the 2045 Full Build Scenario of 216 buses are included in Appendix C. Detroit DOT provided the existing bus block chart that is shown in Figure 4 Site trips were generated based on this Shift Analysis and Bus Block chart. The Shift Analyses were used to forecast employee site trips. The bus block chart was used to forecast bus site trips.

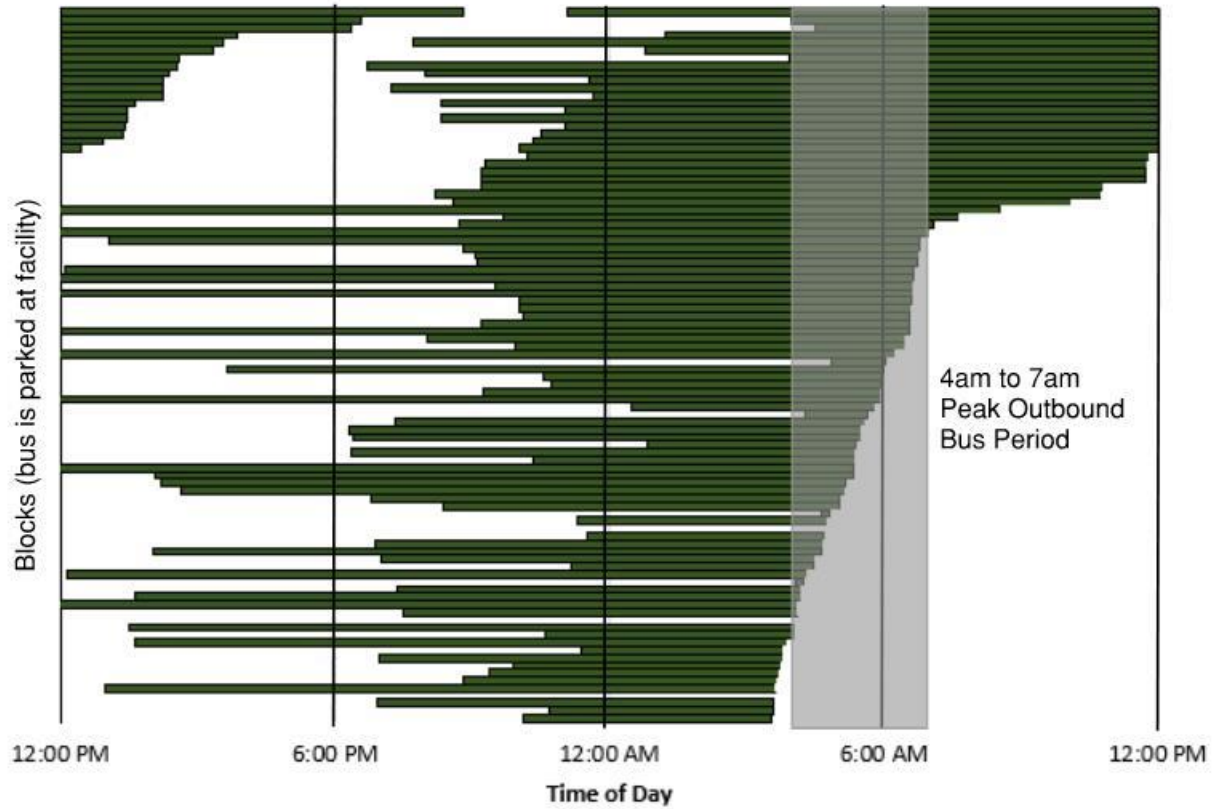


Figure 4 - Bus Block Schedule

The bus block chart shows that there are 91 buses using the Gilbert facility that will be moved over to Coolidge facility. It is assumed that 91 buses will be served at Coolidge in 2025. In year 2045 when the bus operators increase about 10% the buses will increase at the same pace.

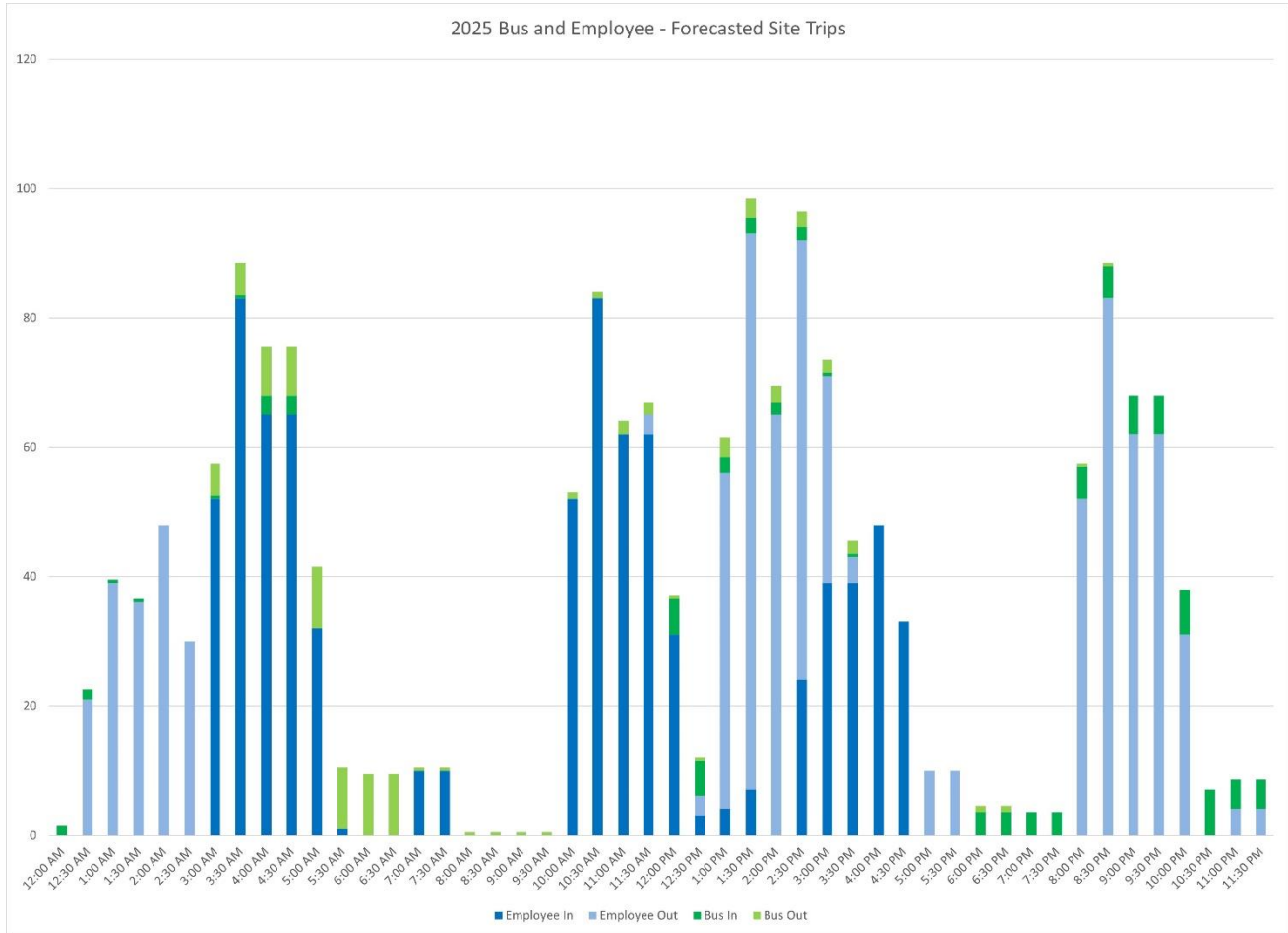


Figure 5 - Forecasted Site Trips

The forecasted employee and bus trips by hour are combined in the chart in Figure 5.

The hour with the highest intensity of trips was found to be the hour during the mid-day shift change between 1:30 PM to 2:30 PM. The second highest hour is during the evening shift change between 8:00 PM and 9:00 PM. These are the two hours that are expected to have largest impact on traffic within the study area and, therefore, are the hours analyzed in this study. It is noted that neither of the site peak hours occur during the commuter peak period that occurs from about 3:30 PM to 5:30 PM. The forecasted employee and bus trips forecasted during these two peak hours is shown in Table 4 below.

Table 4 - Trip Generation

Description	2025 Mid-Day Peak Hour 1:30 PM to 2:30 PM			2025 Evening Peak 8:00 PM to 9:00 PM		
	IN	OUT	Total	IN	OUT	Total
Employees	7	86	93	0	52	52
Buses	5	6	11	10	1	11
Total Trips	12	92	104	10	53	63

Description	2045 Mid-Day Peak Hour 1:30 PM to 2:30 PM			2045 Evening Peak 8:00 PM to 9:00 PM		
	IN	OUT	Total	IN	OUT	Total
Employees	39	96	135	17	53	70
Buses	6	7	13	12	1	13
Total Trips	45	103	148	29	54	83

Distribution and Assignment

An existing distribution model was created for the study area intersections based on existing peak hour traffic volumes. The distribution model was used to distribute employee traffic volumes. Generally, about 20% of employee traffic is expected to arrive and depart to the north from the site and about 80% to the south. Employee site traffic was assigned to each study intersection according to this model for each of the two peak hours.

The distribution of buses to and from the site was developed based on the expected routes that this facility is expected to serve. For the purposes of this analysis, it was assumed that DDOT routes would be served equally between this facility and the Shoemaker facility located on the east side of the city with the Coolidge facility serving routes on the west side. Approximately 20 routes were selected as serving the west side and assumed to operate out of the Coolidge facility. Outbound buses were distributed to the starting points of these 20 routes and assigned to study intersections accordingly. Inbound buses were distributed from the route endpoints and assigned to study intersections as well. The distribution of buses to and from their respective routes resulted in approximately 45% of buses traveling to and from the north from the site and 55% to and from the south.

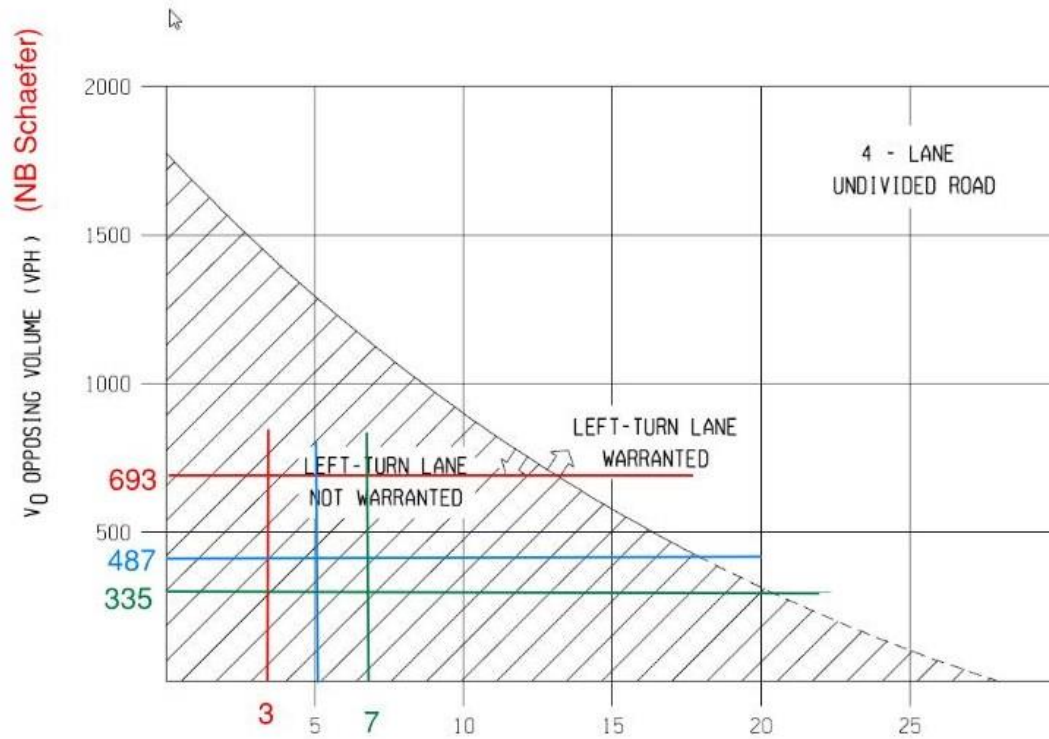
The resulting turning movement volumes expected to be associated with the site trips at the study intersections and the site driveways are illustrated in Exhibits 4-1 and 4-2.

BUILD CONDITIONS ANALYSIS

Forecasted site traffic was added to the forecasted No Build traffic volumes at each study intersection and the two proposed site driveways to arrive at expected total future traffic volumes in the 2025 and 2045 analysis years. The total forecasted turning movement volumes for the Build Condition are illustrated in Exhibits 5-1 and 5-2. Heavy Vehicle percentages for each movement at each study intersection were adjusted according to the bus trip assignments at those intersections. Capacity analysis at each intersection was performed for each of the Mid-Day and Evening peak hours. For the purposes of this analysis, it was assumed that traffic signal timings would be updated to accommodate expected traffic volumes within the next 20 years. In the No Build scenario, some movements at intersections along Grand River Avenue were expected to operate at LOS E in the future without the proposed bus facility. Traffic signal timing improvements were found to improve operations at these intersections even with the addition of site traffic.

We reviewed the need for southbound left turn lanes into each of the two proposed site driveways. This analysis was done using MDOT's Geometric Design Guidance section 1.1.5 Traffic Volume Guidelines for Left-Turn Lanes and the chart for four-lane undivided roads. Northbound opposing traffic and southbound left turning traffic forecasted at each driveway were plotted on the chart. The chart for the bus driveway is shown in Figure 6 and for the employee driveway is shown in Figure 7. The analysis found that a southbound left turn lane is not expected to be warranted at the proposed bus access points nor at the employee driveway during the midday and nighttime peak hours. We looked further at other time periods. The incoming bus volume is at the highest between 9:30 -10:30 PM. In 2045 there are 7 buses that make the SB left turn. However, the northbound opposing traffic is lower than 400 vph. This does not qualify for the warrant condition. The employee incoming traffic volume has a few peaks throughout the 24 hours. The highest southbound left turn volume is 19 vph from 3:30 AM – 4:30 AM, and from 11:30 AM – 12:30 PM. Only from 11:30 AM to 12:30 PM the volumes meet the left turn lane warrants. The second highest left turn volume is 14 vph from 13:30 PM to 1:30 PM at Employee Parking entrance, which does not meet the left turn lane warrant. Overall, there is only one hour at the employee entrance the left turn lane is warranted in 2045. Similarly, in the opening year 2025, there is only one hour that meets the left turn lane warrant at the employee entrance. Therefore, it is not recommended to have a left turn lane at either Coolidge facility entrances, but the traffic volumes be monitored for the future changes should more employee traffic takes the southbound left turn lane.

FOUR-LANE HIGHWAYS



V_L LEFT-TURNING (VPH) (at Bus Way)
 (2045 Mid-day)
 (2045 Night Time)
 (2045 9:30 PM - 10:30PM)

NOTE:

When $V_O < 400$ vph (dashed line), a Left-Turn Lane is Not Normally Warranted Unless The Advancing Volume (V_A) in The Same Direction as the Left-Turning Traffic Exceeds 400 vph ($V_A > 400$ vph).

Figure 6 - Southbound Left Turn Lane Analysis - Bus Driveway

FOUR-LANE HIGHWAYS

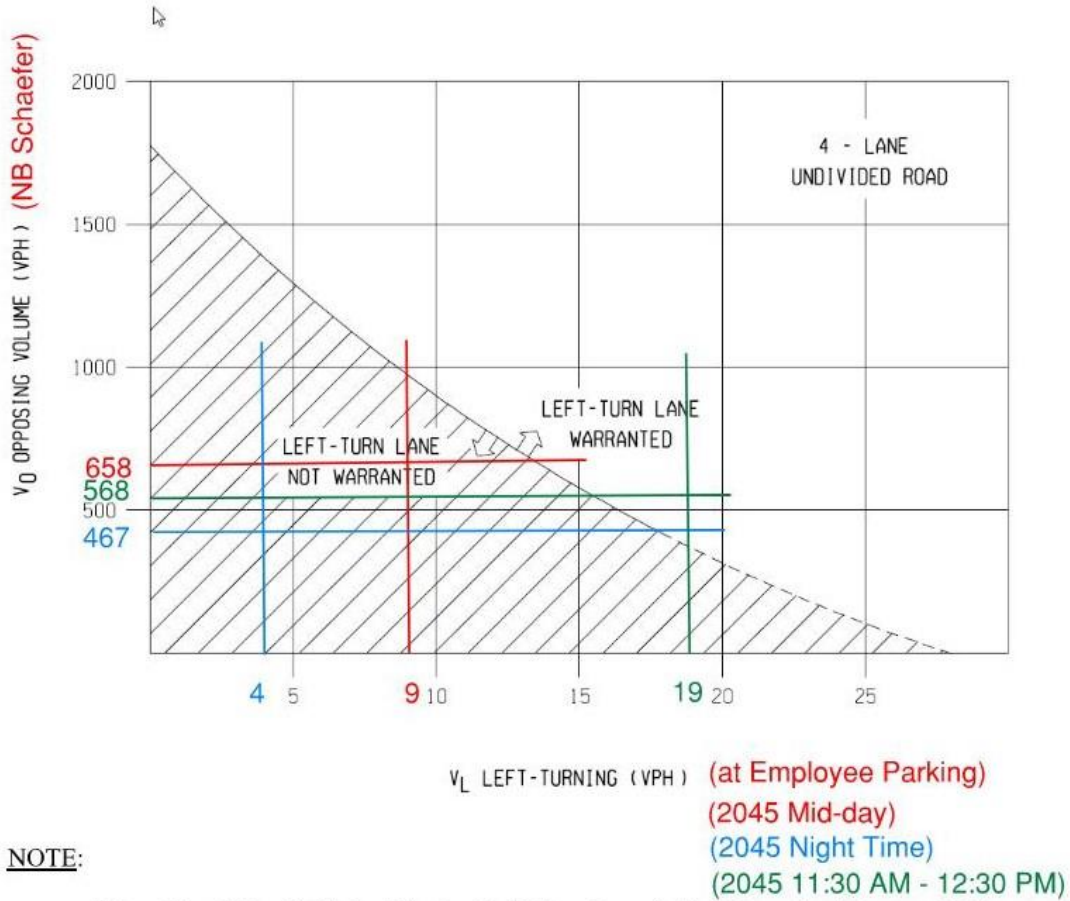


Figure 7 - Southbound Left Turn Lane Analysis - Employee Driveway

The Build Condition capacity analysis found that all movements at the study intersections are expected to continue to operate at acceptable LOS except at the site bus driveway in 2045. During the Mid-Day peak hour, the westbound left turn exiting the bus driveway is expected to operate at LOS E if controlled by a stop sign. This expected LOS is the result of the type of vehicle using the driveway since buses are slower and require larger gaps in traffic to enter a free-flowing roadway than passenger cars. It is noted that the total number of buses forecasted to exit the site and turn left during the Mid-Day peak hour during the 2045 Full Build scenario is four (4). It is recommended that this driveway operate as a stop-controlled approach to Schaefer Highway and that the operations be monitored for indications that alternative control may be needed as the population of on-site employees increases over the coming years.

The results of the Build Condition Capacity Analysis are included in Table 5 and Table 6. The lane configurations and traffic control resulting from the above analysis are illustrated in Exhibit 6.

Table 5 – 2025 Initial Build Condition Capacity Analysis Results

Level of Service Summary – Midday Peak Hour													
Intersection	LOS/Delay (s/veh)												
	Lane Group												
	EB L	EB T	EB R	WB L	WB T	WB R	NB L	NB T	NB R	SB L	SB T	SB R	Overall
Schaefer Hwy & Lyndon St	28.3 C	36.0 D		39.3 D	29.6 C		4.0 A	4.5 A		5.9 A	9.1 A		13.9 B
Schaefer Hwy & Coolidge Bus Drive (Unsignalized)				29.0 D		12.6 B				11.8 B	0.0 A		0.1 A
Schaefer Hwy & Coolidge Employee Drive (Unsignalized)				20.2 C		10.3 B				8.6 A	0.0 A		1.4 A
Schaefer Hwy & Schoolcraft Rd	25.8 C	21.1 C		23.8 C	21.0 C		14.5 B	26.8 C		11.1 B	22.4 C		23.3 C
Schaefer Hwy & Grand River Ave	13.4 B	9.2 A		9.5 A	7.3 A		20.5 C	27.8 C	3.0 A	40.4 D	26.5 C		16.3 B
Schaefer Hwy & I-96 Service Rd				22.1 C	16.8 B		4.6 A	2.7 A			3.6 A		6.3 A
Schaefer Hwy & Jeffries Service Dr		26.3 C						17.6 B		6.7 A	5.0 A		17.3 B
Grand River Ave & I-96 Service Rd					24.0 C	11.2 B	38.0 D	0.9 A			7.4 A	1.5 A	11.4 B
Grand River Ave & Jeffries Service Dr		34.6 C						4.1 A			1.7 A		12.3 B
Left Turn Bridge & Jeffries Service Dr		3.1 A								1.3 A			2.6 A
Schoolcraft Rd & Ward Ave	7.6 A	0.0 A		7.6 A	0.0 A			10.0 B			10.4 B		1.0 A
Meyers Rd & Kendall St		12.2 B			14.5 B		7.9 A			8.0 A			1.1 A

Level of Service Summary – Evening Peak Hour													
Intersection	LOS/Delay (s/veh)												
	Lane Group												
	EB L	EB T	EB R	WB L	WB T	WB R	NB L	NB T	NB R	SB L	SB T	SB R	Overall
Schaefer Hwy & Lyndon St	22.2 C	18.5 B		23.9 C	22.8 C		5.2 A	8.4 A		5.3 A	8.5 A		12.1 B
Schaefer Hwy & Coolidge Bus Drive (Unsignalized)				20.5 C		0.0 A				10.5 B	0.1 A		0.1 A
Schaefer Hwy & Coolidge Employee Drive (Unsignalized)				14.5 B		9.5 A				0.0 A			0.8 A
Schaefer Hwy & Schoolcraft Rd	24.4 C	23.8 C		22.4 C	21.9 C		8.1 A	21.4 C		8.6 A	21.4 C		21.3 C
Schaefer Hwy & Grand River Ave	10.3 B	8.3 A		8.2 A	6.3 A		18.5 B	29.4 C	3.4 A	39.7 D	26.7 C		15.7 B
Schaefer Hwy & I-96 Service Rd				21.7 C	17.3 B		2.7 A	1.9 A			2.6 A		5.7 A
Schaefer Hwy & Jeffries Service Dr		25.5 C						13.7 B		43.5 D	3.6 A		16.4 B
Grand River Ave & I-96 Service Rd					25.1 C	11.7 B	40.3 D	0.9 A			11.3 B	6.1 A	13.2 B
Grand River Ave & Jeffries Service Dr		32.8 C						3.9 A			2.8 A		11.6 B
Left Turn Bridge & Jeffries Service Dr		2.6 A								0.5 A			2.0 A
Schoolcraft Rd & Ward Ave	8.7 A	0.0 A		0.0 A				0.0 A			12.4 B		0.5 A
Meyers Rd & Kendall St		11.0 B			11.2 B		7.8 A			7.8 A			0.8 A

Table 6 - 2045 Full Build Condition Capacity Analysis Results

Level of Service Summary – Midday Peak Hour													
Intersection	LOS/Delay (s/veh)												
	Lane Group												
	EB L	EB T	EB R	WB L	WB T	WB R	NB L	NB T	NB R	SB L	SB T	SB R	Overall
Schaefer Hwy & Lyndon St	27.2 C	36.4 D		47.9 D	29.3 C		4.1 A	4.4 A		7.8 A	11.6 B		15.2 B
Schaefer Hwy & Coolidge Bus Drive (Unsignalized)				42.6 E		13.9 B				13.4 B	0.1 A		0.3 A
Schaefer Hwy & Coolidge Employee Drive (Unsignalized)				30.7 D		11.0 B				9.2 A	0.1 A		2.0 A
Schaefer Hwy & Schoolcraft Rd	27.6 C	22.6 C		24.7 C	23.0 C		15.0 B	45.4 D		13.5 B	45.4 D		34.9 D
Schaefer Hwy & Grand River Ave	21.7 C	12.4 B		12.6 B	9.8 A		17.8 B	22.9 C	2.2 A	49.7 D	23.3 C		17.0 B
Schaefer Hwy & I-96 Service Rd				17.0 B	13.0 B		8.3 A	3.5 A			4.6 A		6.7 A
Schaefer Hwy & Jeffries Service Dr		27.0 C						29.1 C		12.0 B	6.6 A		22.3 C
Grand River Ave & I-96 Service Rd					23.7 C	10.1 B	42.0 D	1.2 A			9.6 A	3.0 A	12.1 B
Grand River Ave & Jeffries Service Dr		35.6 D						4.9 A			1.3 A		12.6 B
Left Turn Bridge & Jeffries Service Dr		4.5 A								3.8 A			4.3 A
Schoolcraft Rd & Ward Ave	7.7 A	0.0 A		7.7 A	0.0 A			10.3 B			11.0 B		1.1 A
Meyers Rd & Kendall St		13.9 B			17.4 C		8.1 A			8.3 A			1.4 A

Level of Service Summary – Evening Peak Hour													
Intersection	LOS/Delay (s/veh)												
	Lane Group												
	EB L	EB T	EB R	WB L	WB T	WB R	NB L	NB T	NB R	SB L	SB T	SB R	Overall
Schaefer Hwy & Lyndon St	21.5 C	19.1 B		23.6 C	23.4 C		6.1 A	9.5 A		6.3 A	9.7 A		13.0 B
Schaefer Hwy & Coolidge Bus Drive (Unsignalized)				25.8 D		0.0 A				11.3 B	0.1 A		0.1 A
Schaefer Hwy & Coolidge Employee Drive (Unsignalized)				17.3 C		9.9 A				8.4 A			0.9 A
Schaefer Hwy & Schoolcraft Rd	25.6 C	25.1 C		22.9 C	23.6 C		8.2 A	25.5 C		9.2 A	24.5 C		24.1 C
Schaefer Hwy & Grand River Ave	14.2 B	11.0 B		11.1 B	8.7 A		16.0 B	26.3 C	4.2 A	43.5 D	23.8 C		16.3 B
Schaefer Hwy & I-96 Service Rd				23.8 C	19.3 B		3.7 A	2.4 A			3.2 A		6.6 A
Schaefer Hwy & Jeffries Service Dr		27.9 C						17.5 B		48.7 D	4.6 A		19.2 B
Grand River Ave & I-96 Service Rd					24.7 C	10.6 B	37.6 D	0.5 A			7.5 A	1.1 A	11.3 B
Grand River Ave & Jeffries Service Dr		36.8 D						4.5 A			1.3 A		12.4 B
Left Turn Bridge & Jeffries Service Dr		2.6 A								0.1 A			2.1 A
Schoolcraft Rd & Ward Ave	9.2 A	0.0 A		0.0 A				0.0 A			13.9 B		0.6 A
Meyers Rd & Kendall St		12.0 B			12.2 B		7.9 A			8.0 A			0.9 A

Consolidated Access Point

At the request of the Detroit Traffic Engineering Department, consolidating the two driveways on Schaefer Highway with both buses and employee traffic using one driveway was considered as part of this analysis. An operational analysis was conducted which combined the bus and employee traffic volumes at a single entrance on Schaefer Highway. Based on the analysis conducted and described above, the proposed driveway intersection was assumed to be controlled by a traffic signal and a southbound left turn lane was assumed to be provided on Schaefer Highway. The Synchro analysis found that a consolidated driveway would be expected to operate at acceptable levels of service in this scenario.

Providing only one access point to the site for all bus, truck, employee and visitor traffic results in logistical and operational challenges internal the site, however. This site will also be secured with guard houses and gates for access. Separate guard houses must be provided for bus and truck traffic and for passenger car traffic to accommodate the different methods for gaining access (key cards, badges, paperwork, etc.), accommodate the different vehicle sizes, and ensure efficient ingress and egress.

The safest environment for passenger car, bus and truck traffic to mix is on the public roadway system that is designed for this variation in vehicle type and size and where drivers expect it. Once on site, drivers of passenger vehicles are not expecting to encounter large vehicles. Drivers of heavy vehicles expect that once on site, they have the space and freedom to maneuver without the expectation of encountering smaller vehicles. For these reasons, industrial facilities such as this are typically designed to separate access and circulation of passenger and heavy vehicle traffic to minimize the potential for conflicts and, thus, crashes between the two.

Separate access drives for the site on Schaefer Highway are expected to operate well as described in the Build Conditions analysis section above. Consolidating the access to one driveway is expected to result in conflicts and challenges detrimental to the operations of the proposed facility. Therefore, it is recommended that two access driveways be provided according to the proposed site plan in Figure 3.

Emergency Access Use

In certain emergency situations, it may be necessary for buses to utilize the Emergency Access driveway proposed on the east side of the site at the intersection of Ward and Kendall Avenues. This situation was analyzed as it pertains to its expected impact on the intersections of Ward Avenue with Schoolcraft Road and Kendall Avenue at Meyers Road. Both intersections operate with minor leg stop control, however, the Ward Avenue and Schoolcraft intersection was previously controlled by a traffic signal. The traffic signal was removed in 2017 or 2018. An analysis was performed assuming approximately 50 buses would exit the Emergency Access driveway during the Mid-Day peak hour and distribute according to the distribution model discussed above. The analysis found that in this situation, both intersections would continue to operate at acceptable levels of service of LOS C or better. Therefore, no mitigation measures are expected to be needed in the event that the Emergency Access needs to temporarily be used by buses.

CONCLUSIONS

DLZ analyzed the expected traffic impacts of the proposed Coolidge Bus Operations and Maintenance Facility on the area roadway network including Schaefer Highway between Lyndon Street and the Jeffries Freeway (I-96). The study resulted in the following recommendations:

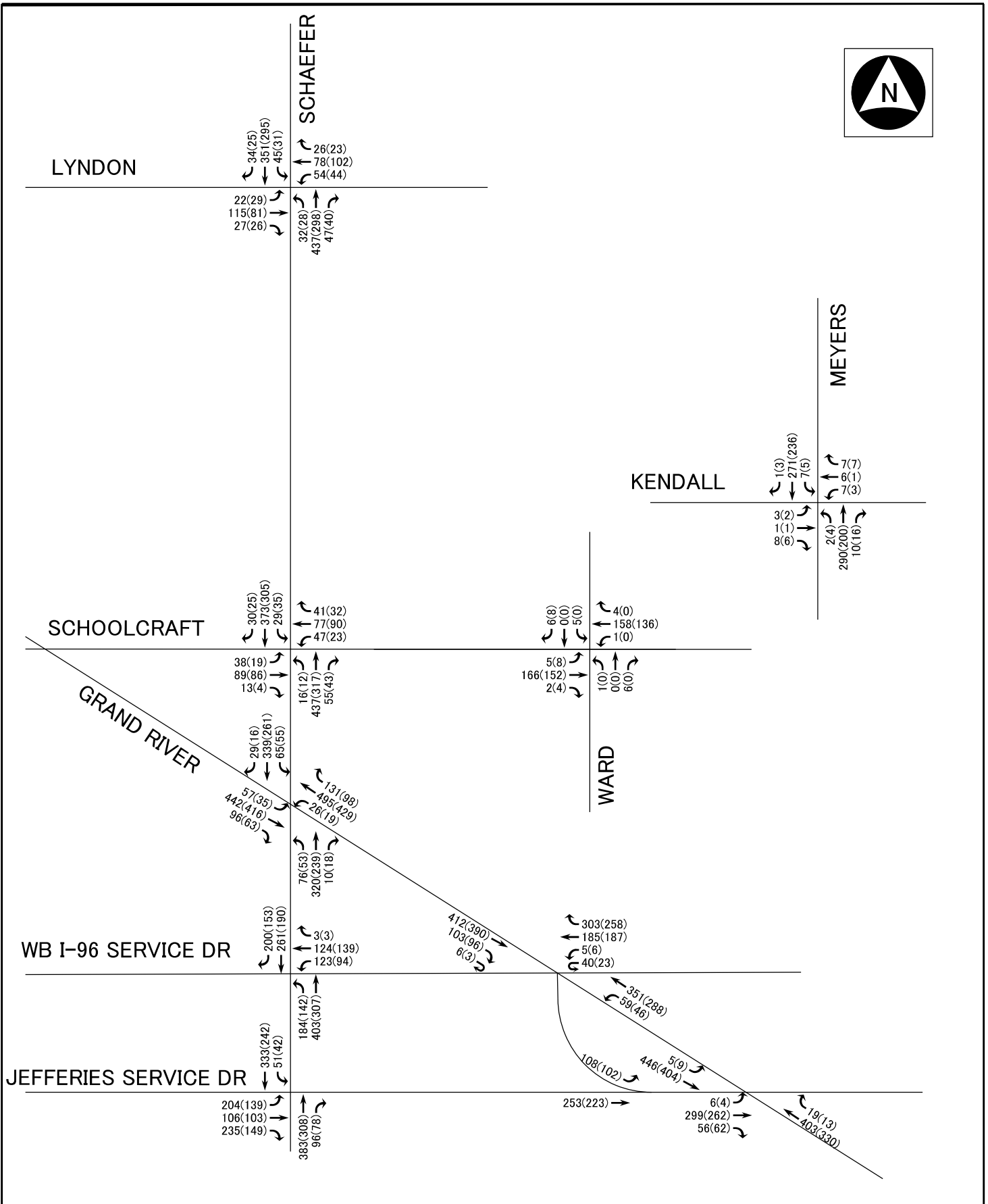
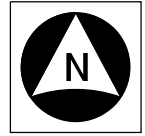
1. Provide separate site access points for buses and employees per the site plan, consolidating the access points into one is not recommended.
2. Westbound stop controls of the proposed site driveways for buses and employees at Schaefer Highway are recommended.
3. Monitor the westbound approach of the bus driveway during the Mid-Day peak hour near the horizon year as the intersection capacity is forecasted to approach LOS E.
4. Left turn lanes on Shaefer Highway were not found to be warranted for the Opening Year 2025 Initial Build Scenario based on forecasted traffic volumes and MDOT's Geometric Design Guidance. A southbound left turn lane may be warranted at the employee driveway in the year 2045 Full Build Scenario.
5. In emergency situations where buses must exit via the Emergency Access driveway, the intersections of Ward Avenue / Schoolcraft Road and Kendall Street / Meyers Road are expected to operate at acceptable levels of service with no mitigation measures needed.

With these recommendations, the study area intersections are expected to operate at acceptable levels of service with the proposed Coolidge Bus facility operating at full capacity in the year 2045. One possible exception is the proposed bus driveway during peak hours. Operations at this driveway should be monitored, and mitigation measures identified if needed, as the horizon year approaches.



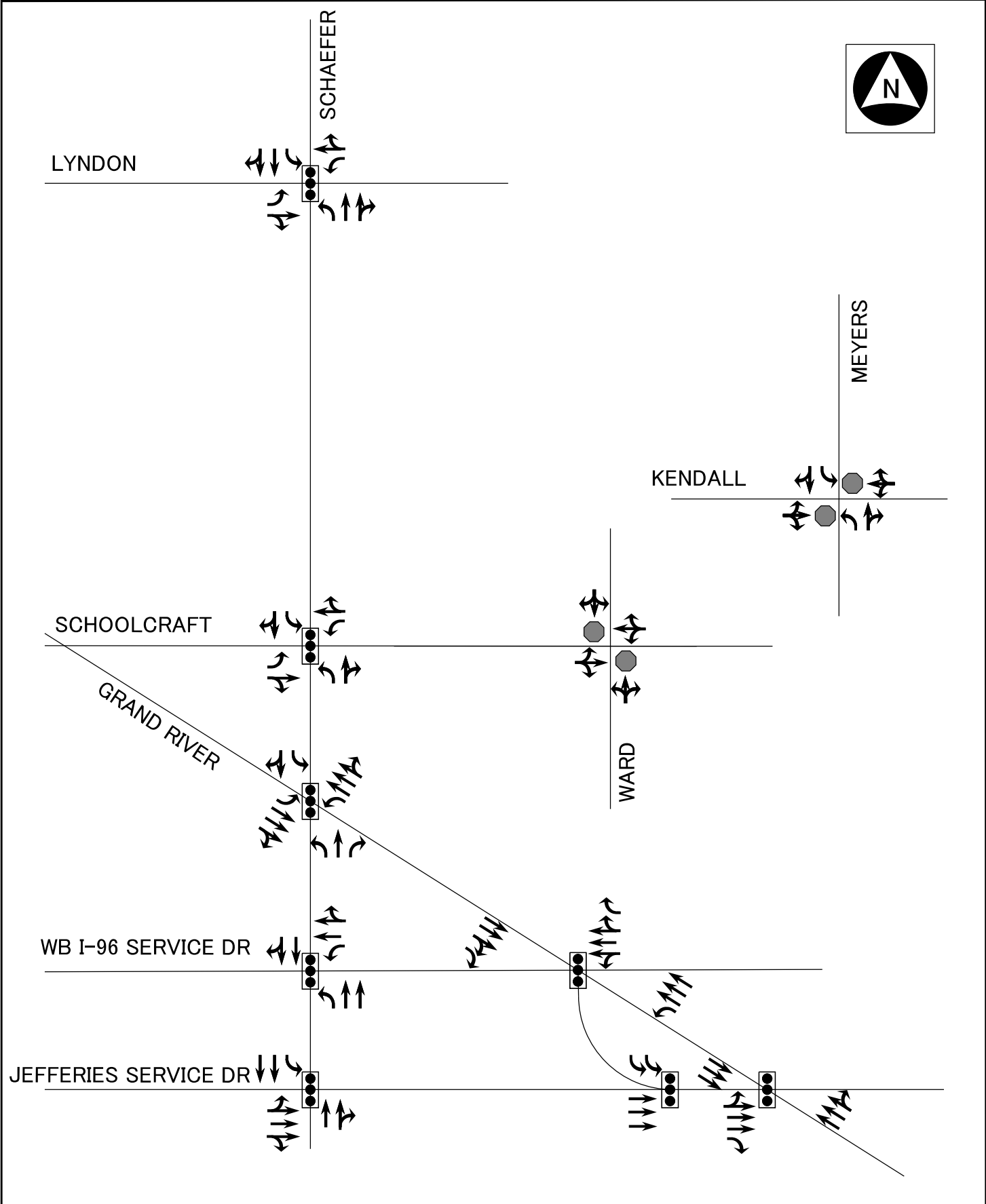
INNOVATIVE IDEAS
EXCEPTIONAL DESIGN
UNMATCHED CLIENT SERVICE

TRAFFIC VOLUME AND LANE CONFIGURATION EXHIBITS



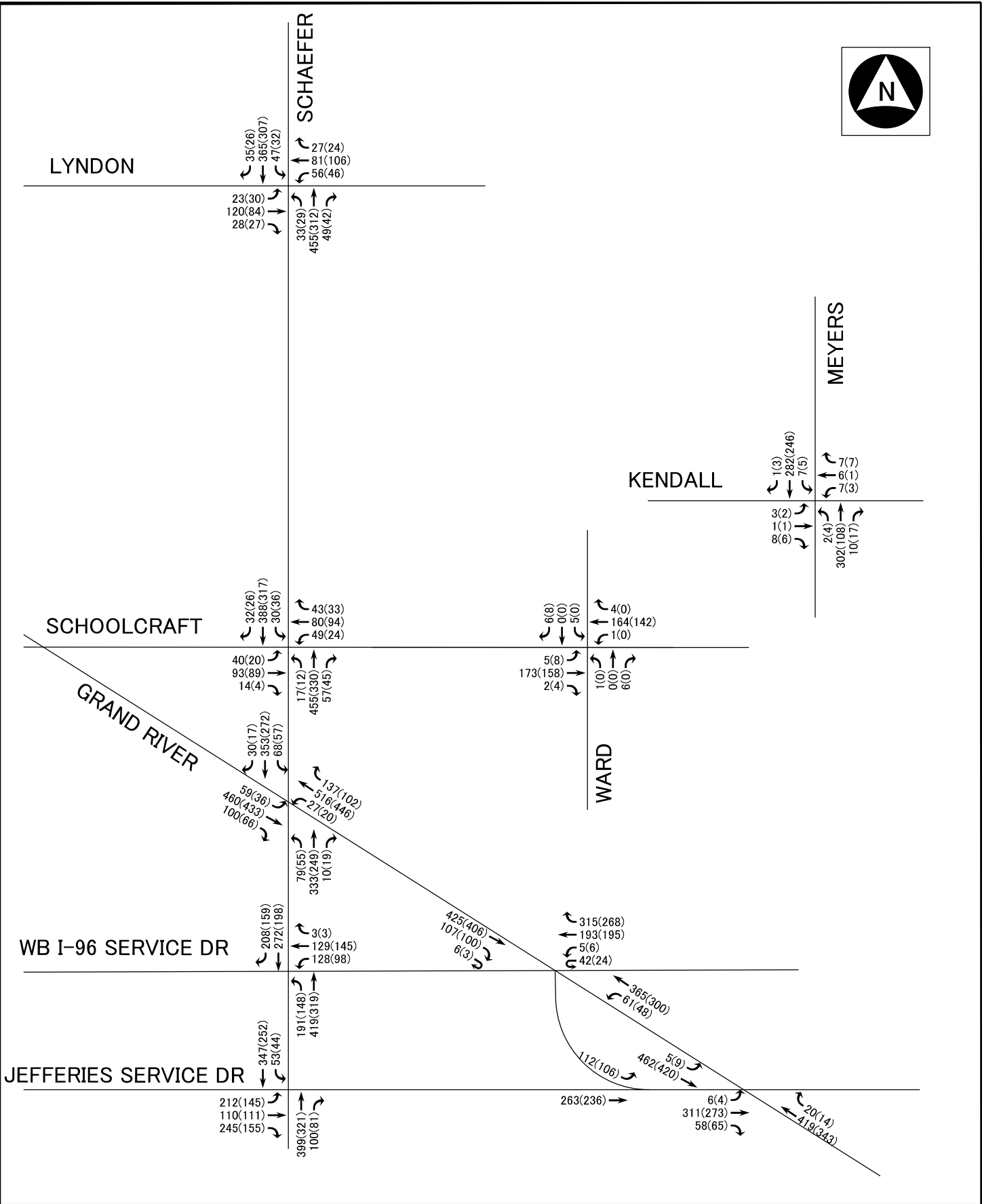
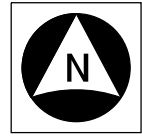
EXISTING
(2021)

EXHIBIT 1
PEAK HOUR
TURNING MOVEMENT VOLUMES
MID-DAY (EVENING)



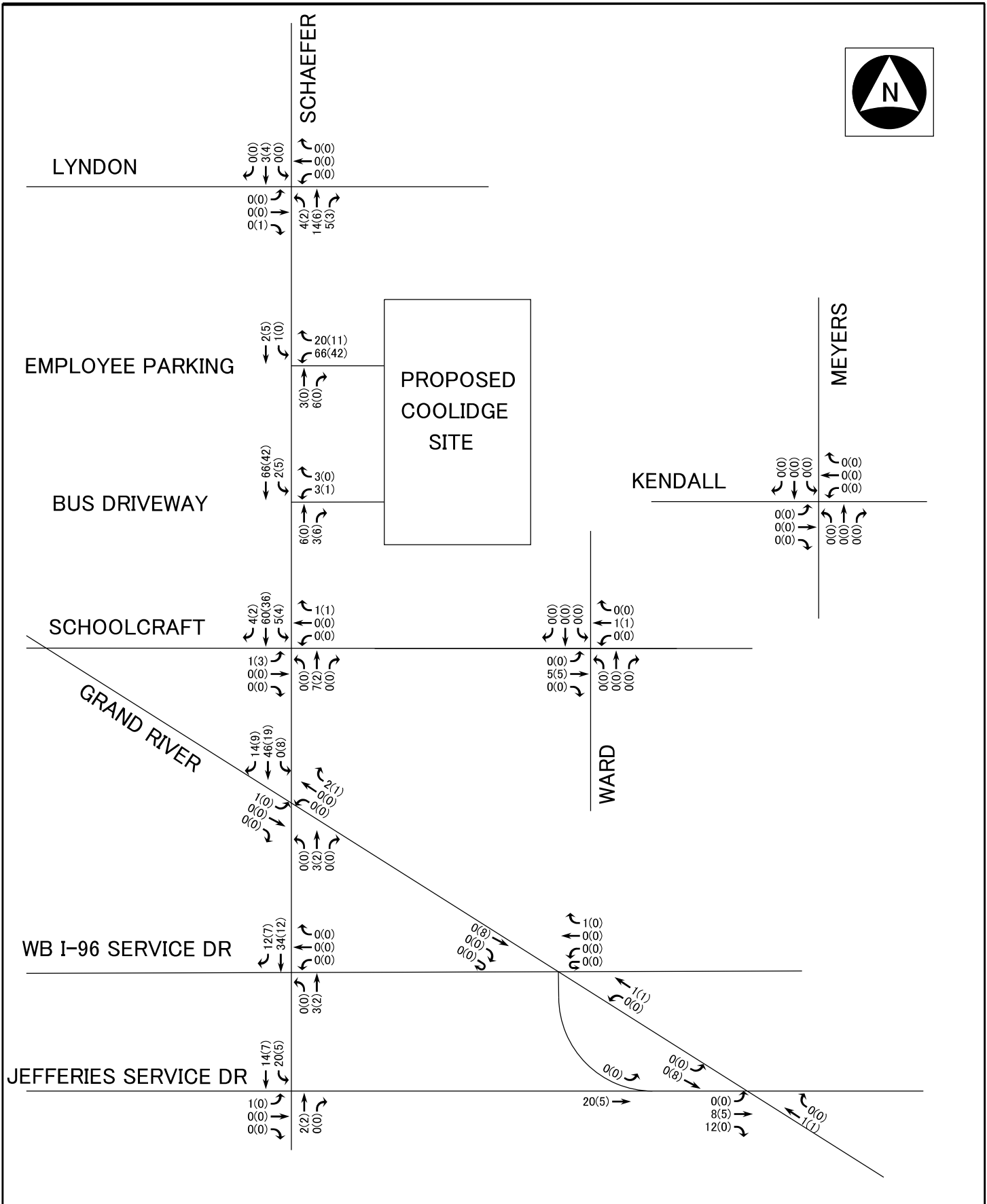
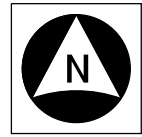
EXISTING
(2021)

EXHIBIT 2
LANE CONFIGURATION



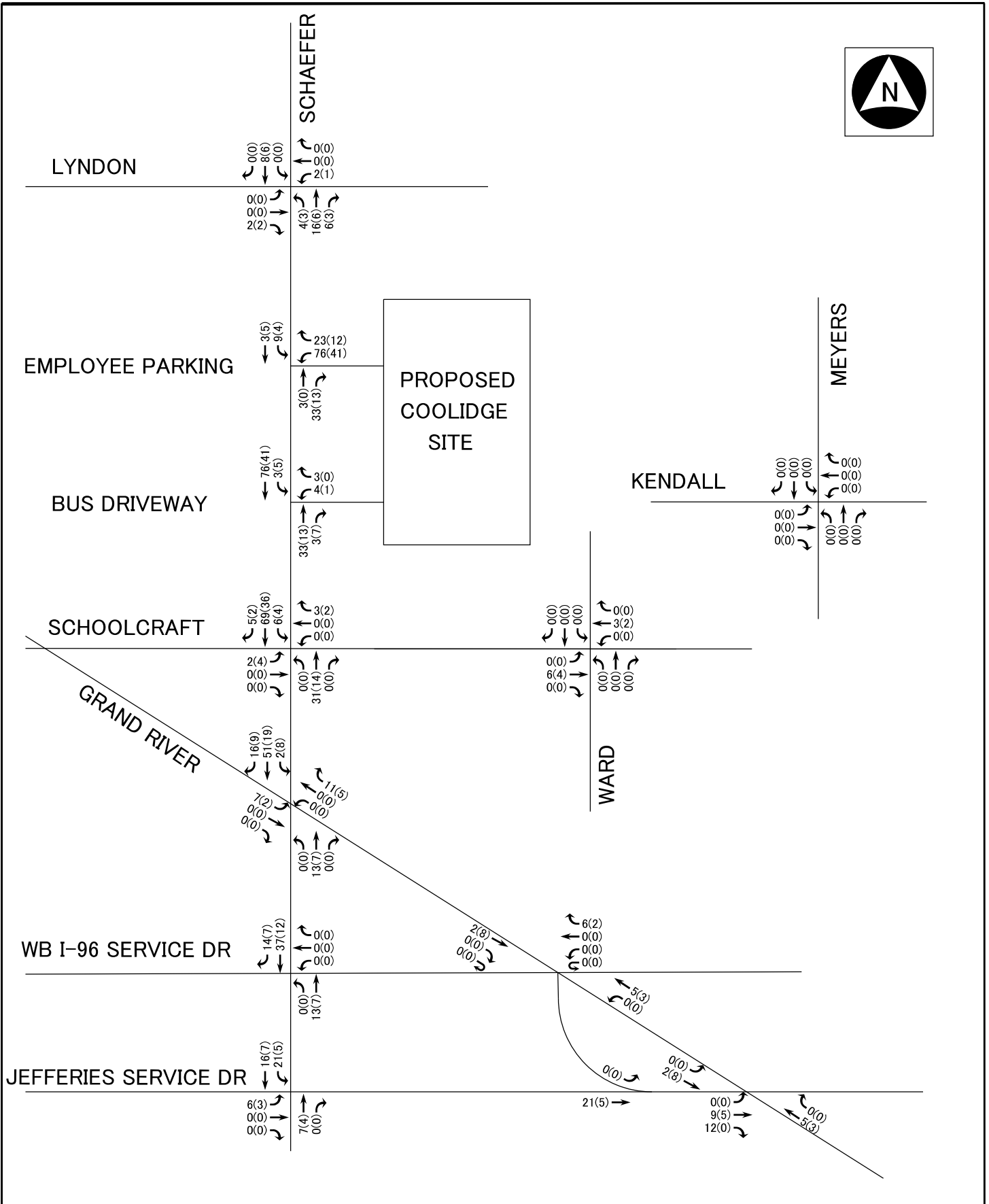
**PROJECTED NO-BUILD
(2025)**

**EXHIBIT 3
PEAK HOUR
TURNING MOVEMENT VOLUMES
MID-DAY (EVENING)**



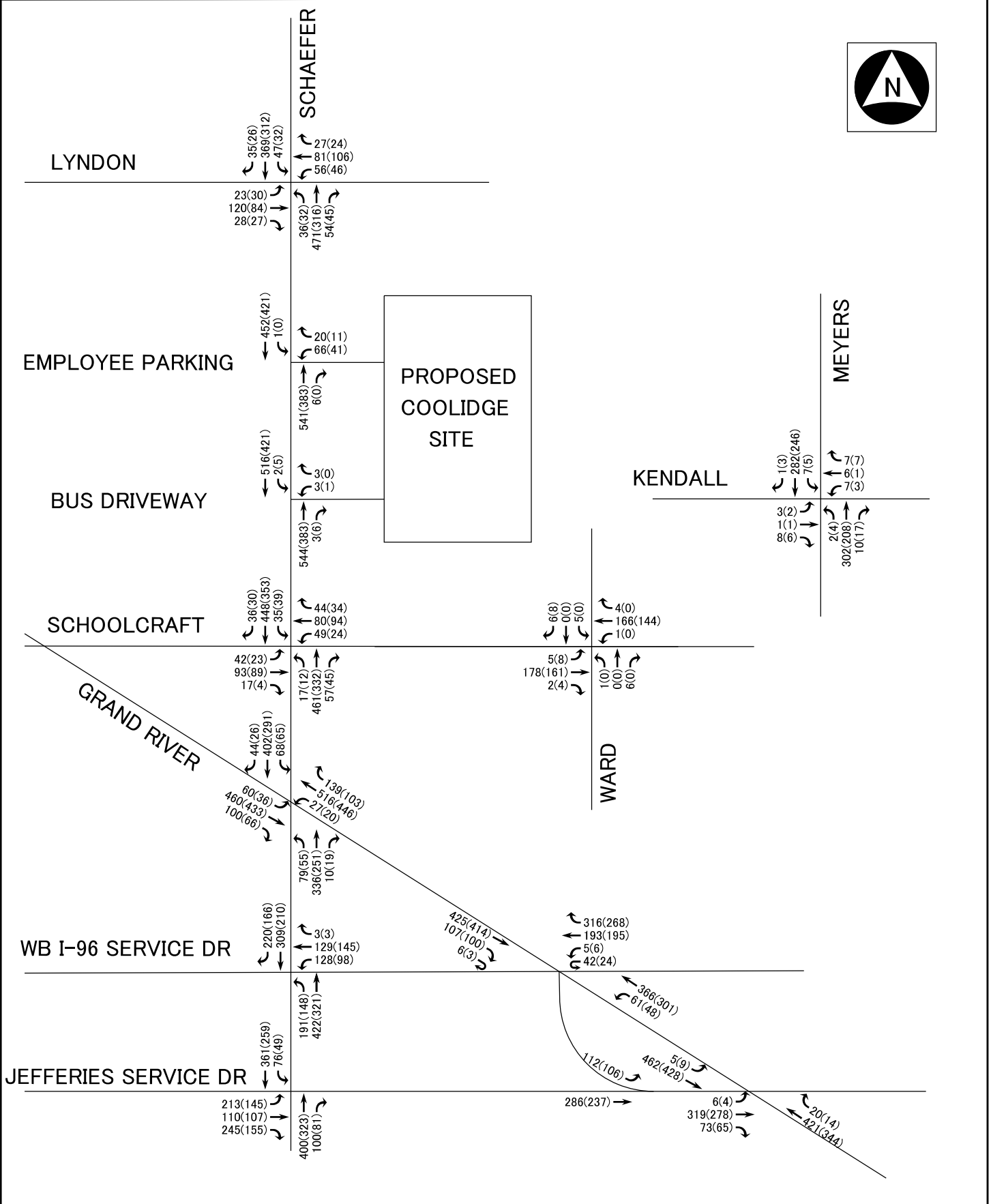
**PROPOSED SITE
(2025)**

**EXHIBIT 4-1
PEAK HOUR
TURNING MOVEMENT VOLUMES
MID-DAY (EVENING)**



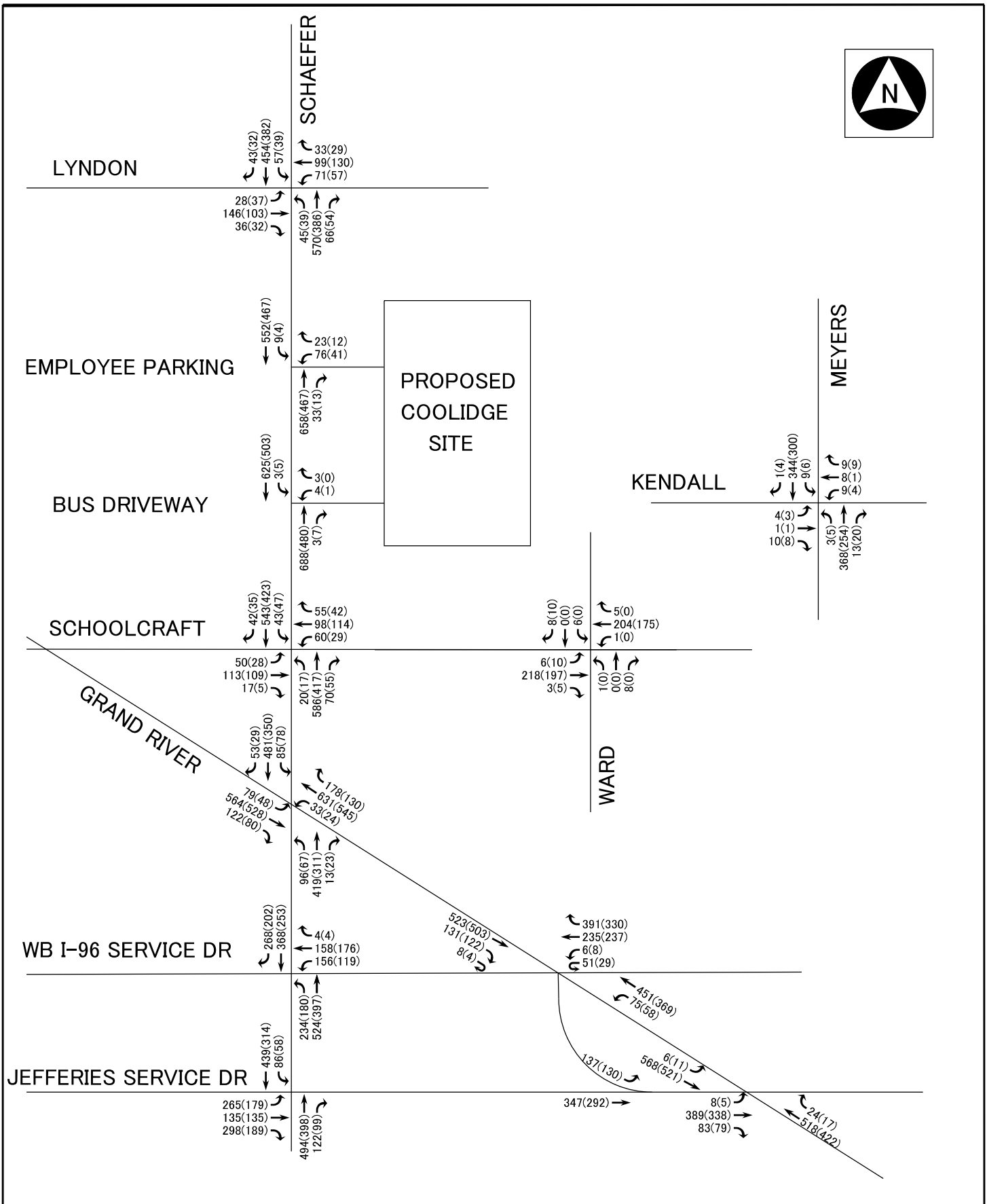
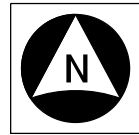
**PROPOSED SITE
(2045)**

EXHIBIT 4-2
PEAK HOUR
TURNING MOVEMENT VOLUMES
MID-DAY (EVENING)



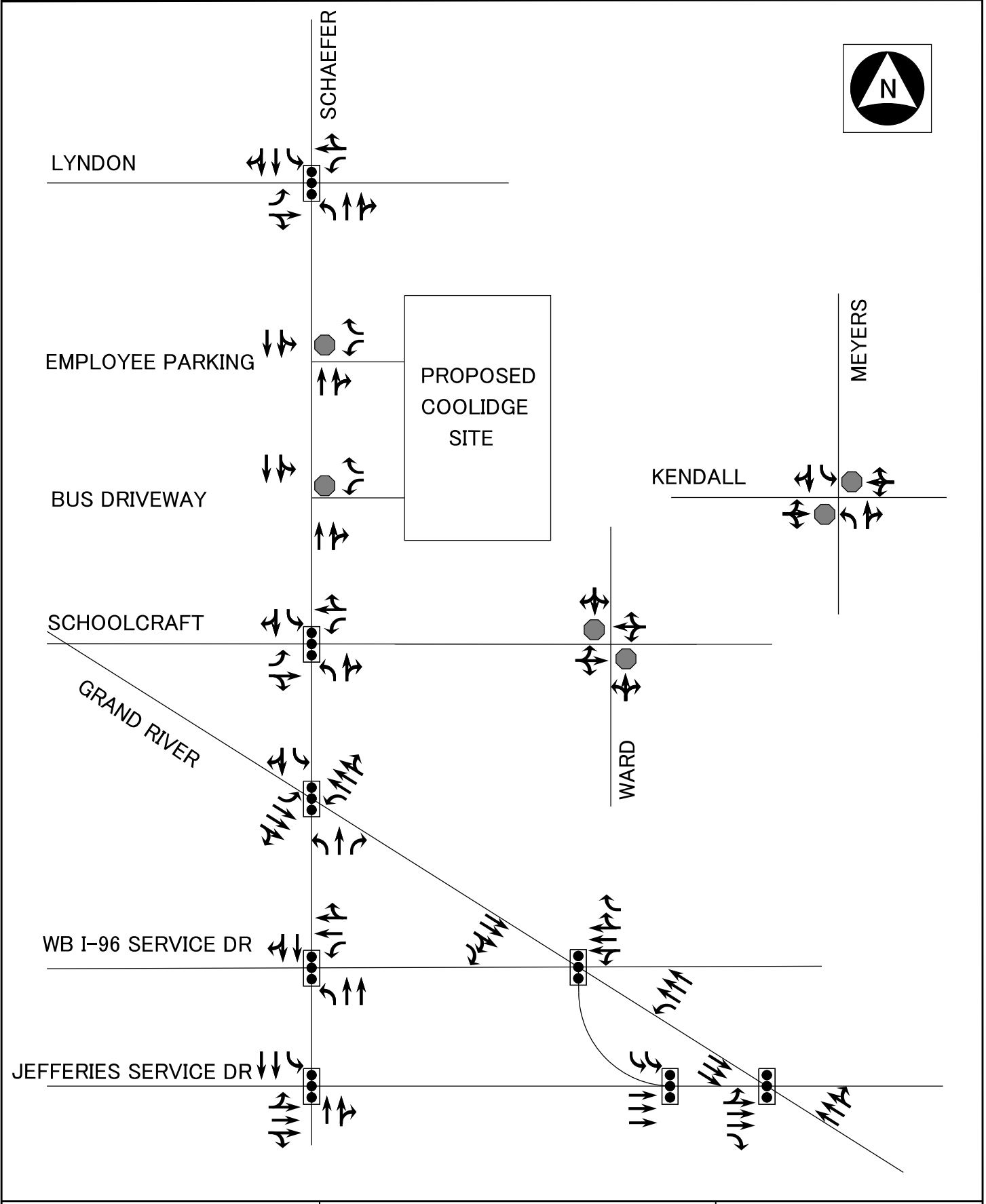
**PROJECTED BUILD
(2025)**

EXHIBIT 5-1
PEAK HOUR
TURNING MOVEMENT VOLUMES
MID-DAY (EVENING)



**PROJECTED BUILD
(2045)**

**EXHIBIT 5-2
PEAK HOUR
TURNING MOVEMENT VOLUMES
MID-DAY (EVENING)**



PROPOSED BUILD
(2025)

EXHIBIT 6
LANE CONFIGURATION



INNOVATIVE IDEAS
EXCEPTIONAL DESIGN
UNMATCHED CLIENT SERVICE

APPENDIX A
TRAFFIC TURNING MOVEMENT COUNTS

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schaefer Hwy & Lyndon St
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 1

Turning Movement Data

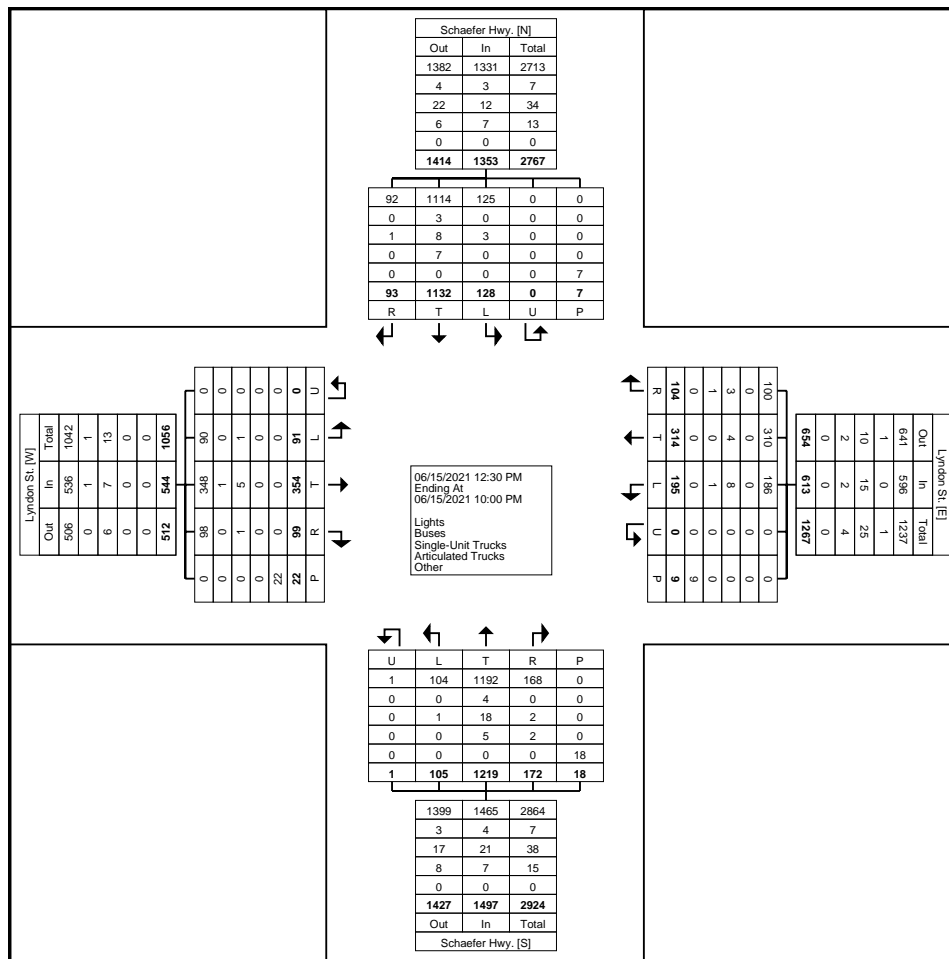
Start Time	Schaefer Hwy. Southbound						Lyndon St. Westbound						Schaefer Hwy. Northbound						Lyndon St. Eastbound						Int. Total
	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	
12:30 PM	5	70	12	0	1	87	7	17	14	0	0	38	11	75	4	0	1	90	6	23	6	0	2	35	250
12:45 PM	5	76	9	0	1	90	5	23	12	0	1	40	13	70	5	0	3	88	5	30	1	0	1	36	254
Hourly Total	10	146	21	0	2	177	12	40	26	0	1	78	24	145	9	0	4	178	11	53	7	0	3	71	504
1:00 PM	5	57	6	0	1	68	12	21	12	0	0	45	8	83	7	0	2	98	4	28	9	0	1	41	252
1:15 PM	2	85	12	0	1	99	10	18	17	0	0	45	17	83	16	0	0	116	6	20	11	0	0	37	297
1:30 PM	4	78	13	0	0	95	4	18	12	0	2	34	15	78	9	0	0	102	5	27	6	0	0	38	269
1:45 PM	10	80	11	0	0	101	5	14	12	0	0	31	16	91	12	0	0	119	9	21	6	0	0	36	287
Hourly Total	21	300	42	0	2	363	31	71	53	0	2	155	56	335	44	0	2	435	24	96	32	0	1	152	1105
2:00 PM	7	74	6	0	0	87	8	28	12	0	3	48	4	97	5	0	1	106	7	38	2	0	2	47	288
2:15 PM	13	92	15	0	0	120	9	18	18	0	0	45	12	105	6	0	1	123	6	29	8	0	4	43	331
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	20	166	21	0	0	207	17	46	30	0	3	93	16	202	11	0	2	229	13	67	10	0	6	90	619
8:00 PM	2	66	5	0	0	73	6	30	9	0	0	45	13	82	7	1	2	103	6	19	10	0	3	35	256
8:15 PM	12	73	13	0	2	98	6	21	13	0	0	40	13	78	3	0	2	94	9	20	7	0	2	36	268
8:30 PM	5	78	6	0	0	89	5	26	15	0	0	46	6	73	7	0	1	86	4	22	6	0	1	32	253
8:45 PM	6	68	7	0	0	81	6	25	7	0	1	38	8	65	10	0	1	83	7	20	6	0	0	33	235
Hourly Total	25	285	31	0	2	341	23	102	44	0	1	169	40	298	27	1	6	366	26	81	29	0	6	136	1012
9:00 PM	4	59	4	0	0	67	9	17	14	0	2	40	18	58	7	0	3	83	10	13	5	0	0	28	218
9:15 PM	6	58	2	0	1	66	4	14	12	0	0	30	7	57	4	0	1	68	8	16	1	0	2	25	189
9:30 PM	3	58	3	0	0	64	5	11	8	0	0	24	5	61	1	0	0	67	5	19	3	0	0	27	182
9:45 PM	4	60	4	0	0	68	3	13	8	0	0	24	6	63	2	0	0	71	2	9	4	0	4	15	178
Hourly Total	17	235	13	0	1	265	21	55	42	0	2	118	36	239	14	0	4	289	25	57	13	0	6	95	767
Grand Total	93	1132	128	0	7	1353	104	314	195	0	9	613	172	1219	105	1	18	1497	99	354	91	0	22	544	4007
Approach %	6.9	83.7	9.5	0.0	-	-	17.0	51.2	31.8	0.0	-	-	11.5	81.4	7.0	0.1	-	-	18.2	65.1	16.7	0.0	-	-	-
Total %	2.3	28.3	3.2	0.0	-	33.8	2.6	7.8	4.9	0.0	-	15.3	4.3	30.4	2.6	0.0	-	37.4	2.5	8.8	2.3	0.0	-	13.6	-
Lights	92	1114	125	0	-	1331	100	310	186	0	-	596	168	1192	104	1	-	1465	98	348	90	0	-	536	3928
% Lights	98.9	98.4	97.7	-	-	98.4	96.2	98.7	95.4	-	-	97.2	97.7	97.8	99.0	100.0	-	97.9	99.0	98.3	98.9	-	-	98.5	98.0
Buses	0	3	0	0	-	3	0	0	0	0	-	0	0	4	0	0	-	4	0	1	0	0	-	1	8
% Buses	0.0	0.3	0.0	-	-	0.2	0.0	0.0	0.0	-	-	0.0	0.0	0.3	0.0	0.0	-	0.3	0.0	0.3	0.0	-	-	0.2	0.2
Single-Unit Trucks	1	8	3	0	-	12	3	4	8	0	-	15	2	18	1	0	-	21	1	5	1	0	-	7	55
% Single-Unit Trucks	1.1	0.7	2.3	-	-	0.9	2.9	1.3	4.1	-	-	2.4	1.2	1.5	1.0	0.0	-	1.4	1.0	1.4	1.1	-	-	1.3	1.4
Articulated Trucks	0	7	0	0	-	7	1	0	1	0	-	2	2	5	0	0	-	7	0	0	0	0	-	0	16
% Articulated Trucks	0.0	0.6	0.0	-	-	0.5	1.0	0.0	0.5	-	-	0.3	1.2	0.4	0.0	0.0	-	0.5	0.0	0.0	0.0	-	-	0.0	0.4
Bicycles on Crosswalk	-	-	-	-	3	-	-	-	-	-	1	-	-	-	-	-	4	-	-	-	-	-	2	-	-
% Bicycles on Crosswalk	-	-	-	-	42.9	-	-	-	-	-	11.1	-	-	-	-	-	22.2	-	-	-	-	-	9.1	-	-
Pedestrians	-	-	-	-	4	-	-	-	-	-	8	-	-	-	-	-	14	-	-	-	-	-	20	-	-

% Pedestrians	-	-	-	-	57.1	-	-	-	-	88.9	-	-	-	-	77.8	-	-	-	-	90.9	-	-
---------------	---	---	---	---	------	---	---	---	---	------	---	---	---	---	------	---	---	---	---	------	---	---

Unit Number:

DLZ
8430 West Bryn Mawr Avenue, Suite 100
Chicago, Illinois, United States 60631
(773) 283-2600

Count Name: Schaefer Hwy & Lyndon St
Site Code: 1942-6994-50/0800/2000
Start Date: 06/15/2021
Page No: 3



Turning Movement Data Plot

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schaefer Hwy & Lyndon St
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 4

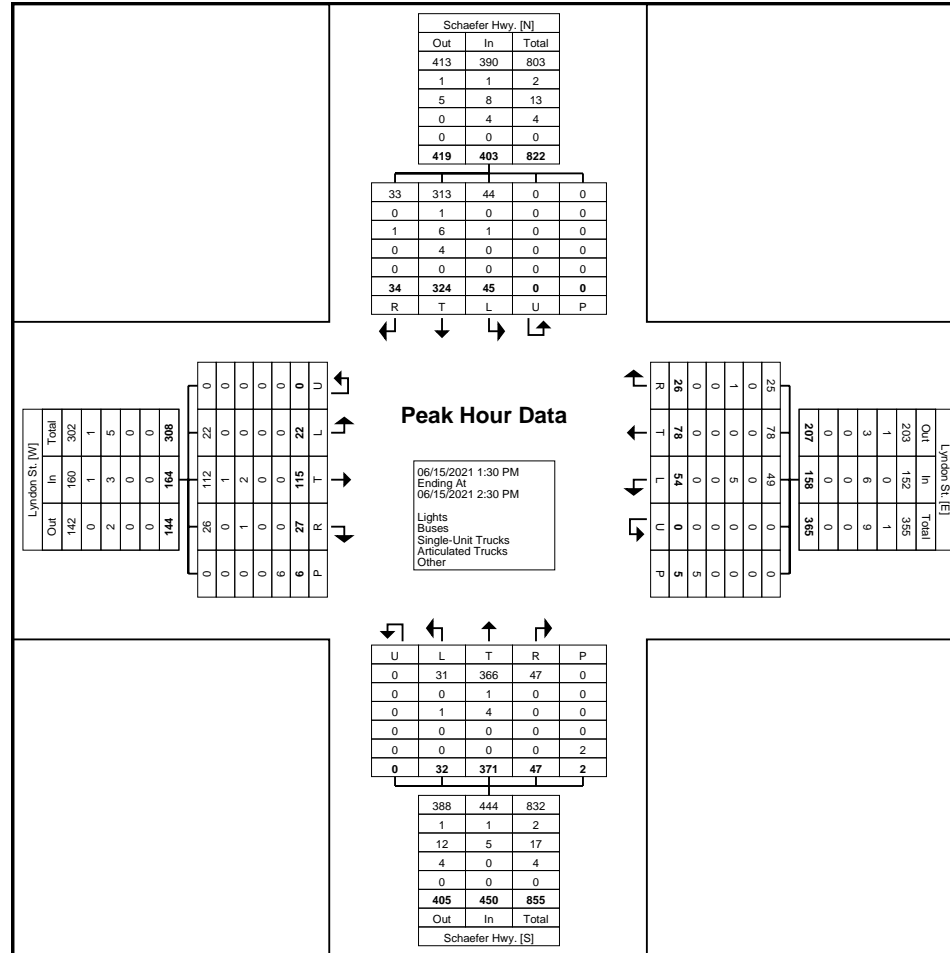
Turning Movement Peak Hour Data (1:30 PM)

Start Time	Schaefer Hwy. Southbound						Lyndon St. Westbound						Schaefer Hwy. Northbound						Lyndon St. Eastbound						Int. Total
	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	
1:30 PM	4	78	13	0	0	95	4	18	12	0	2	34	15	78	9	0	0	102	5	27	6	0	0	38	269
1:45 PM	10	80	11	0	0	101	5	14	12	0	0	31	16	91	12	0	0	119	9	21	6	0	0	36	287
2:00 PM	7	74	6	0	0	87	8	28	12	0	3	48	4	97	5	0	1	106	7	38	2	0	2	47	288
2:15 PM	13	92	15	0	0	120	9	18	18	0	0	45	12	105	6	0	1	123	6	29	8	0	4	43	331
Total	34	324	45	0	0	403	26	78	54	0	5	158	47	371	32	0	2	450	27	115	22	0	6	164	1175
Approach %	8.4	80.4	11.2	0.0	-	-	16.5	49.4	34.2	0.0	-	-	10.4	82.4	7.1	0.0	-	-	16.5	70.1	13.4	0.0	-	-	-
Total %	2.9	27.6	3.8	0.0	-	34.3	2.2	6.6	4.6	0.0	-	13.4	4.0	31.6	2.7	0.0	-	38.3	2.3	9.8	1.9	0.0	-	14.0	-
PHF	0.654	0.880	0.750	0.000	-	0.840	0.722	0.696	0.750	0.000	-	0.823	0.734	0.883	0.667	0.000	-	0.915	0.750	0.757	0.688	0.000	-	0.872	0.887
Lights	33	313	44	0	-	390	25	78	49	0	-	152	47	366	31	0	-	444	26	112	22	0	-	160	1146
% Lights	97.1	96.6	97.8	-	-	96.8	96.2	100.0	90.7	-	-	96.2	100.0	98.7	96.9	-	-	98.7	96.3	97.4	100.0	-	-	97.6	97.5
Buses	0	1	0	0	-	1	0	0	0	0	-	0	0	1	0	0	-	1	0	1	0	0	-	1	3
% Buses	0.0	0.3	0.0	-	-	0.2	0.0	0.0	0.0	-	-	0.0	0.0	0.3	0.0	-	-	0.2	0.0	0.9	0.0	-	-	0.6	0.3
Single-Unit Trucks	1	6	1	0	-	8	1	0	5	0	-	6	0	4	1	0	-	5	1	2	0	0	-	3	22
% Single-Unit Trucks	2.9	1.9	2.2	-	-	2.0	3.8	0.0	9.3	-	-	3.8	0.0	1.1	3.1	-	-	1.1	3.7	1.7	0.0	-	-	1.8	1.9
Articulated Trucks	0	4	0	0	-	4	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	4
% Articulated Trucks	0.0	1.2	0.0	-	-	1.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.3
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	-	0	-	-	-	-	-	1	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	20.0	-	-	-	-	-	0.0	-	-	-	-	-	16.7	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	4	-	-	-	-	-	2	-	-	-	-	-	5	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	80.0	-	-	-	-	-	100.0	-	-	-	-	-	83.3	-	-

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schaefer Hwy & Lyndon St
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 5



Turning Movement Peak Hour Data Plot (1:30 PM)

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schaefer Hwy & Lyndon St
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 6

Turning Movement Peak Hour Data (8:00 PM)

Start Time	Schaefer Hwy. Southbound						Lyndon St. Westbound						Schaefer Hwy. Northbound						Lyndon St. Eastbound						Int. Total
	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	
8:00 PM	2	66	5	0	0	73	6	30	9	0	0	45	13	82	7	1	2	103	6	19	10	0	3	35	256
8:15 PM	12	73	13	0	2	98	6	21	13	0	0	40	13	78	3	0	2	94	9	20	7	0	2	36	268
8:30 PM	5	78	6	0	0	89	5	26	15	0	0	46	6	73	7	0	1	86	4	22	6	0	1	32	253
8:45 PM	6	68	7	0	0	81	6	25	7	0	1	38	8	65	10	0	1	83	7	20	6	0	0	33	235
Total	25	285	31	0	2	341	23	102	44	0	1	169	40	298	27	1	6	366	26	81	29	0	6	136	1012
Approach %	7.3	83.6	9.1	0.0	-	-	13.6	60.4	26.0	0.0	-	-	10.9	81.4	7.4	0.3	-	-	19.1	59.6	21.3	0.0	-	-	-
Total %	2.5	28.2	3.1	0.0	-	33.7	2.3	10.1	4.3	0.0	-	16.7	4.0	29.4	2.7	0.1	-	36.2	2.6	8.0	2.9	0.0	-	13.4	-
PHF	0.521	0.913	0.596	0.000	-	0.870	0.958	0.850	0.733	0.000	-	0.918	0.769	0.909	0.675	0.250	-	0.888	0.722	0.920	0.725	0.000	-	0.944	0.944
Lights	25	284	30	0	-	339	23	102	44	0	-	169	39	295	27	1	-	362	26	81	29	0	-	136	1006
% Lights	100.0	99.6	96.8	-	-	99.4	100.0	100.0	100.0	-	-	100.0	97.5	99.0	100.0	100.0	-	98.9	100.0	100.0	100.0	-	-	100.0	99.4
Buses	0	1	0	0	-	1	0	0	0	0	-	0	0	1	0	0	-	1	0	0	0	0	-	0	2
% Buses	0.0	0.4	0.0	-	-	0.3	0.0	0.0	0.0	-	-	0.0	0.0	0.3	0.0	0.0	-	0.3	0.0	0.0	0.0	-	-	0.0	0.2
Single-Unit Trucks	0	0	1	0	-	1	0	0	0	0	-	0	1	1	0	0	-	2	0	0	0	0	-	0	3
% Single-Unit Trucks	0.0	0.0	3.2	-	-	0.3	0.0	0.0	0.0	-	-	0.0	2.5	0.3	0.0	0.0	-	0.5	0.0	0.0	0.0	-	-	0.0	0.3
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	1	0	0	-	1	0	0	0	0	-	0	1
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.3	0.0	0.0	-	0.3	0.0	0.0	0.0	-	-	0.0	0.1
Bicycles on Crosswalk	-	-	-	-	1	-	-	-	-	-	0	-	-	-	-	-	2	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	50.0	-	-	-	-	-	0.0	-	-	-	-	-	33.3	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	1	-	-	-	-	-	1	-	-	-	-	-	4	-	-	-	-	-	6	-	-
% Pedestrians	-	-	-	-	50.0	-	-	-	-	-	100.0	-	-	-	-	-	66.7	-	-	-	-	-	100.0	-	-

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schafer Hwy & Schoolcraft
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 1

Turning Movement Data

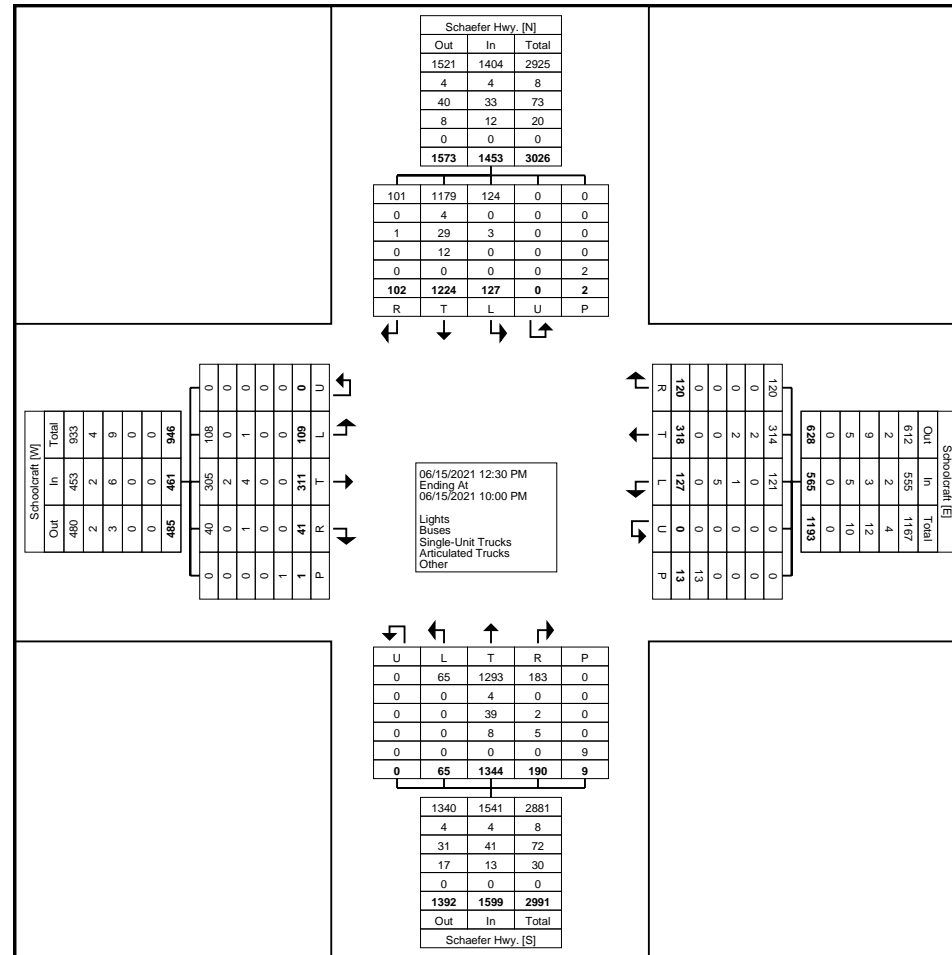
Start Time	Schafer Hwy. Southbound						Schoolcraft Westbound						Schafer Hwy. Northbound						Schoolcraft Eastbound						Int. Total
	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	
12:30 PM	2	80	3	0	0	85	8	12	5	0	1	25	7	82	6	0	1	95	3	20	10	0	0	33	238
12:45 PM	7	88	8	0	0	103	5	26	7	0	1	38	18	92	6	0	0	116	2	21	4	0	0	27	284
Hourly Total	9	168	11	0	0	188	13	38	12	0	2	63	25	174	12	0	1	211	5	41	14	0	0	60	522
1:00 PM	5	74	5	0	1	84	2	17	13	0	1	32	8	99	8	0	0	115	9	21	14	0	0	44	275
1:15 PM	14	94	9	0	0	117	10	18	5	0	0	33	18	101	7	0	2	126	2	25	4	0	0	31	307
1:30 PM	6	96	5	0	0	107	8	20	11	0	1	39	17	99	7	0	0	123	6	17	11	0	0	34	303
1:45 PM	6	87	5	0	0	98	8	27	13	0	0	48	16	104	2	0	0	122	2	20	6	0	0	28	296
Hourly Total	31	351	24	0	1	406	28	82	42	0	2	152	59	403	24	0	2	486	19	83	35	0	0	137	1181
2:00 PM	11	88	8	0	0	107	13	17	14	0	3	44	11	108	6	0	0	125	3	23	6	0	0	32	308
2:15 PM	7	102	11	0	0	120	12	13	9	0	0	34	11	112	1	0	0	124	2	21	15	0	0	38	316
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	18	190	19	0	0	227	25	30	23	0	3	78	22	220	7	0	0	249	5	44	21	0	0	70	624
8:00 PM	3	70	7	0	0	80	9	28	3	0	1	40	5	82	4	0	0	91	3	34	6	0	0	43	254
8:15 PM	9	75	9	0	0	93	9	20	5	0	2	34	14	85	4	0	3	103	1	11	9	0	0	21	251
8:30 PM	10	65	10	0	1	85	7	24	8	0	1	39	15	73	3	0	2	91	0	18	2	0	0	20	235
8:45 PM	3	60	9	0	0	72	7	18	7	0	2	32	9	69	1	0	1	79	0	23	2	0	1	25	208
Hourly Total	25	270	35	0	1	330	32	90	23	0	6	145	43	309	12	0	6	364	4	86	19	0	1	109	948
9:00 PM	4	66	13	0	0	83	8	25	12	0	0	45	8	71	4	0	0	83	4	16	4	0	0	24	235
9:15 PM	4	71	5	0	0	80	8	18	5	0	0	31	10	52	3	0	0	65	1	13	8	0	0	22	198
9:30 PM	9	57	10	0	0	76	2	18	5	0	0	25	15	57	2	0	0	74	3	16	3	0	0	22	197
9:45 PM	2	51	10	0	0	63	4	17	5	0	0	26	8	58	1	0	0	67	0	12	5	0	0	17	173
Hourly Total	19	245	38	0	0	302	22	78	27	0	0	127	41	238	10	0	0	289	8	57	20	0	0	85	803
Grand Total	102	1224	127	0	2	1453	120	318	127	0	13	565	190	1344	65	0	9	1599	41	311	109	0	1	461	4078
Approach %	7.0	84.2	8.7	0.0	-	-	21.2	56.3	22.5	0.0	-	-	11.9	84.1	4.1	0.0	-	-	8.9	67.5	23.6	0.0	-	-	-
Total %	2.5	30.0	3.1	0.0	-	35.6	2.9	7.8	3.1	0.0	-	13.9	4.7	33.0	1.6	0.0	-	39.2	1.0	7.6	2.7	0.0	-	11.3	-
Lights	101	1179	124	0	-	1404	120	314	121	0	-	555	183	1293	65	0	-	1541	40	305	108	0	-	453	3953
% Lights	99.0	96.3	97.6	-	-	96.6	100.0	98.7	95.3	-	-	98.2	96.3	96.2	100.0	-	-	96.4	97.6	98.1	99.1	-	-	98.3	96.9
Buses	0	4	0	0	-	4	0	2	0	0	-	2	0	4	0	0	-	4	0	2	0	0	-	2	12
% Buses	0.0	0.3	0.0	-	-	0.3	0.0	0.6	0.0	-	-	0.4	0.0	0.3	0.0	-	-	0.3	0.0	0.6	0.0	-	-	0.4	0.3
Single-Unit Trucks	1	29	3	0	-	33	0	2	1	0	-	3	2	39	0	0	-	41	1	4	1	0	-	6	83
% Single-Unit Trucks	1.0	2.4	2.4	-	-	2.3	0.0	0.6	0.8	-	-	0.5	1.1	2.9	0.0	-	-	2.6	2.4	1.3	0.9	-	-	1.3	2.0
Articulated Trucks	0	12	0	0	-	12	0	0	5	0	-	5	5	8	0	0	-	13	0	0	0	0	-	0	30
% Articulated Trucks	0.0	1.0	0.0	-	-	0.8	0.0	0.0	3.9	-	-	0.9	2.6	0.6	0.0	-	-	0.8	0.0	0.0	0.0	-	-	0.0	0.7
Bicycles on Crosswalk	-	-	-	-	1	-	-	-	-	-	6	-	-	-	-	-	1	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	50.0	-	-	-	-	-	46.2	-	-	-	-	-	11.1	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	1	-	-	-	-	-	7	-	-	-	-	-	8	-	-	-	-	-	1	-	-

% Pedestrians	-	-	-	-	50.0	-	-	-	-	53.8	-	-	-	-	88.9	-	-	-	-	100.0	-	-
---------------	---	---	---	---	------	---	---	---	---	------	---	---	---	---	------	---	---	---	---	-------	---	---

Unit Number:

DLZ
8430 West Bryn Mawr Avenue, Suite 100
Chicago, Illinois, United States 60631
(773) 283-2600

Count Name: Schafer Hwy & Schoolcraft
Site Code: 1942-6994-50/0800/2000
Start Date: 06/15/2021
Page No: 3



Turning Movement Data Plot

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schafer Hwy & Schoolcraft
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 4

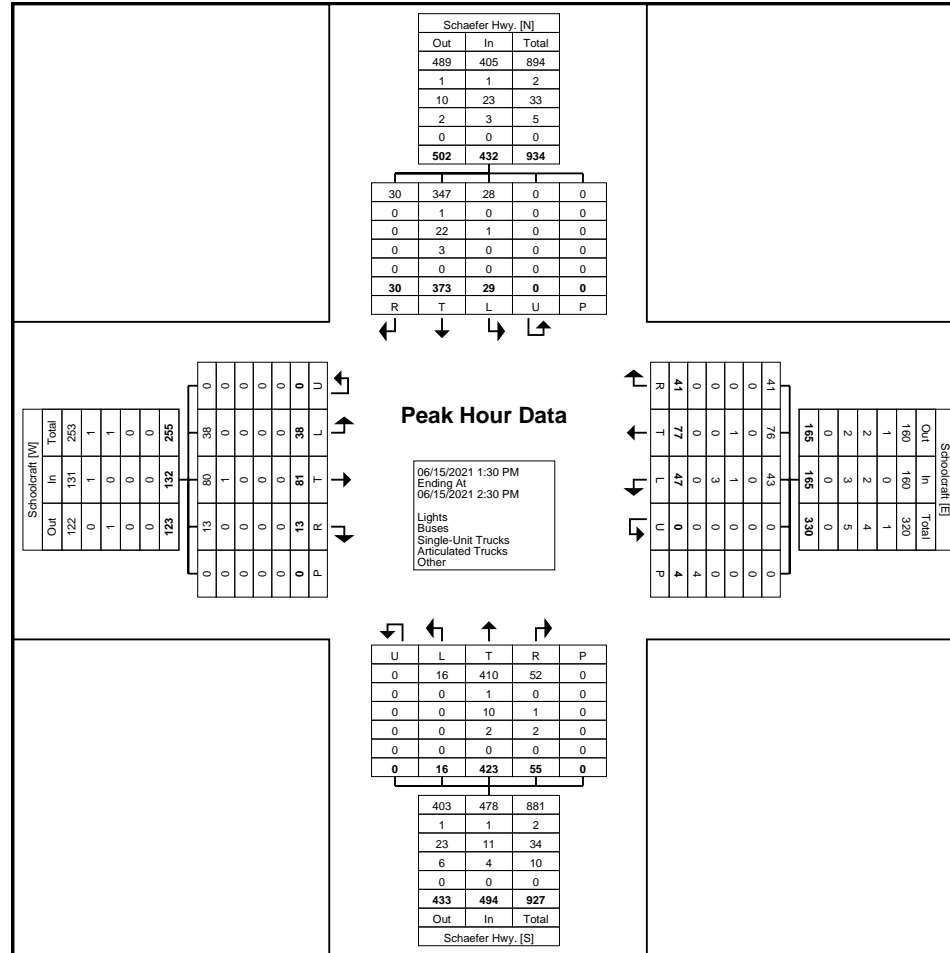
Turning Movement Peak Hour Data (1:30 PM)

Start Time	Schaefer Hwy. Southbound						Schoolcraft Westbound						Schaefer Hwy. Northbound						Schoolcraft Eastbound						Int. Total
	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	
1:30 PM	6	96	5	0	0	107	8	20	11	0	1	39	17	99	7	0	0	123	6	17	11	0	0	34	303
1:45 PM	6	87	5	0	0	98	8	27	13	0	0	48	16	104	2	0	0	122	2	20	6	0	0	28	296
2:00 PM	11	88	8	0	0	107	13	17	14	0	3	44	11	108	6	0	0	125	3	23	6	0	0	32	308
2:15 PM	7	102	11	0	0	120	12	13	9	0	0	34	11	112	1	0	0	124	2	21	15	0	0	38	316
Total	30	373	29	0	0	432	41	77	47	0	4	165	55	423	16	0	0	494	13	81	38	0	0	132	1223
Approach %	6.9	86.3	6.7	0.0	-	-	24.8	46.7	28.5	0.0	-	-	11.1	85.6	3.2	0.0	-	-	9.8	61.4	28.8	0.0	-	-	-
Total %	2.5	30.5	2.4	0.0	-	35.3	3.4	6.3	3.8	0.0	-	13.5	4.5	34.6	1.3	0.0	-	40.4	1.1	6.6	3.1	0.0	-	10.8	-
PHF	0.682	0.914	0.659	0.000	-	0.900	0.788	0.713	0.839	0.000	-	0.859	0.809	0.944	0.571	0.000	-	0.988	0.542	0.880	0.633	0.000	-	0.868	0.968
Lights	30	347	28	0	-	405	41	76	43	0	-	160	52	410	16	0	-	478	13	80	38	0	-	131	1174
% Lights	100.0	93.0	96.6	-	-	93.8	100.0	98.7	91.5	-	-	97.0	94.5	96.9	100.0	-	-	96.8	100.0	98.8	100.0	-	-	99.2	96.0
Buses	0	1	0	0	-	1	0	0	0	0	-	0	0	1	0	0	-	1	0	1	0	0	-	1	3
% Buses	0.0	0.3	0.0	-	-	0.2	0.0	0.0	0.0	-	-	0.0	0.0	0.2	0.0	-	-	0.2	0.0	1.2	0.0	-	-	0.8	0.2
Single-Unit Trucks	0	22	1	0	-	23	0	1	1	0	-	2	1	10	0	0	-	11	0	0	0	0	-	0	36
% Single-Unit Trucks	0.0	5.9	3.4	-	-	5.3	0.0	1.3	2.1	-	-	1.2	1.8	2.4	0.0	-	-	2.2	0.0	0.0	0.0	-	-	0.0	2.9
Articulated Trucks	0	3	0	0	-	3	0	0	3	0	-	3	2	2	0	0	-	4	0	0	0	0	-	0	10
% Articulated Trucks	0.0	0.8	0.0	-	-	0.7	0.0	0.0	6.4	-	-	1.8	3.6	0.5	0.0	-	-	0.8	0.0	0.0	0.0	-	-	0.0	0.8
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	2	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	50.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	2	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	50.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schafer Hwy & Schoolcraft
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 5



Turning Movement Peak Hour Data Plot (1:30 PM)

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schafer Hwy & Schoolcraft
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 6

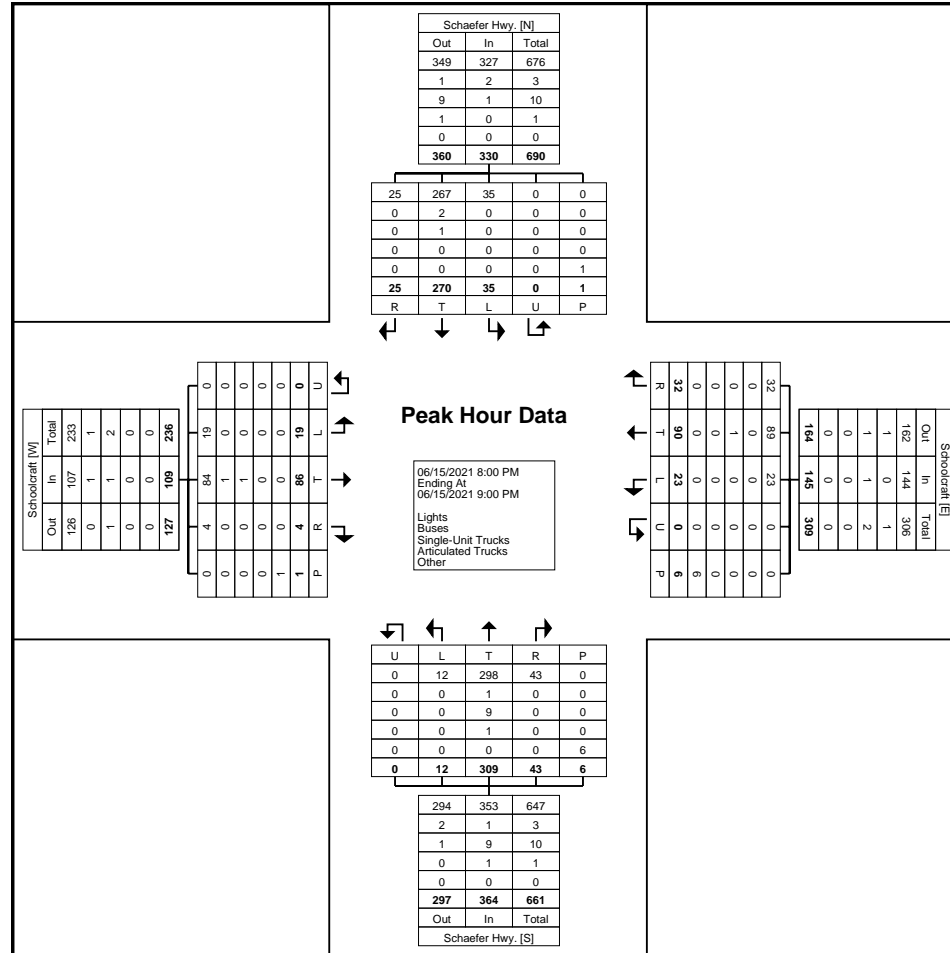
Turning Movement Peak Hour Data (8:00 PM)

Start Time	Schaefer Hwy. Southbound						Schoolcraft Westbound						Schaefer Hwy. Northbound						Schoolcraft Eastbound						Int. Total
	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	
8:00 PM	3	70	7	0	0	80	9	28	3	0	1	40	5	82	4	0	0	91	3	34	6	0	0	43	254
8:15 PM	9	75	9	0	0	93	9	20	5	0	2	34	14	85	4	0	3	103	1	11	9	0	0	21	251
8:30 PM	10	65	10	0	1	85	7	24	8	0	1	39	15	73	3	0	2	91	0	18	2	0	0	20	235
8:45 PM	3	60	9	0	0	72	7	18	7	0	2	32	9	69	1	0	1	79	0	23	2	0	1	25	208
Total	25	270	35	0	1	330	32	90	23	0	6	145	43	309	12	0	6	364	4	86	19	0	1	109	948
Approach %	7.6	81.8	10.6	0.0	-	-	22.1	62.1	15.9	0.0	-	-	11.8	84.9	3.3	0.0	-	-	3.7	78.9	17.4	0.0	-	-	-
Total %	2.6	28.5	3.7	0.0	-	34.8	3.4	9.5	2.4	0.0	-	15.3	4.5	32.6	1.3	0.0	-	38.4	0.4	9.1	2.0	0.0	-	11.5	-
PHF	0.625	0.900	0.875	0.000	-	0.887	0.889	0.804	0.719	0.000	-	0.906	0.717	0.909	0.750	0.000	-	0.883	0.333	0.632	0.528	0.000	-	0.634	0.933
Lights	25	267	35	0	-	327	32	89	23	0	-	144	43	298	12	0	-	353	4	84	19	0	-	107	931
% Lights	100.0	98.9	100.0	-	-	99.1	100.0	98.9	100.0	-	-	99.3	100.0	96.4	100.0	-	-	97.0	100.0	97.7	100.0	-	-	98.2	98.2
Buses	0	2	0	0	-	2	0	0	0	0	-	0	0	1	0	0	-	1	0	1	0	0	-	1	4
% Buses	0.0	0.7	0.0	-	-	0.6	0.0	0.0	0.0	-	-	0.0	0.0	0.3	0.0	-	-	0.3	0.0	1.2	0.0	-	-	0.9	0.4
Single-Unit Trucks	0	1	0	0	-	1	0	1	0	0	-	1	0	9	0	0	-	9	0	1	0	0	-	1	12
% Single-Unit Trucks	0.0	0.4	0.0	-	-	0.3	0.0	1.1	0.0	-	-	0.7	0.0	2.9	0.0	-	-	2.5	0.0	1.2	0.0	-	-	0.9	1.3
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	1	0	0	-	1	0	0	0	0	-	0	1
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.3	0.0	-	-	0.3	0.0	0.0	0.0	-	-	0.0	0.1
Bicycles on Crosswalk	-	-	-	-	1	-	-	-	-	-	3	-	-	-	-	-	1	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	100.0	-	-	-	-	-	50.0	-	-	-	-	-	16.7	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	3	-	-	-	-	-	5	-	-	-	-	-	1	-	-
% Pedestrians	-	-	-	-	0.0	-	-	-	-	-	50.0	-	-	-	-	-	83.3	-	-	-	-	-	100.0	-	-

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schafer Hwy & Schoolcraft
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 7



Turning Movement Peak Hour Data Plot (8:00 PM)

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schaefer Hwy & I-96 Service Rd
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 1

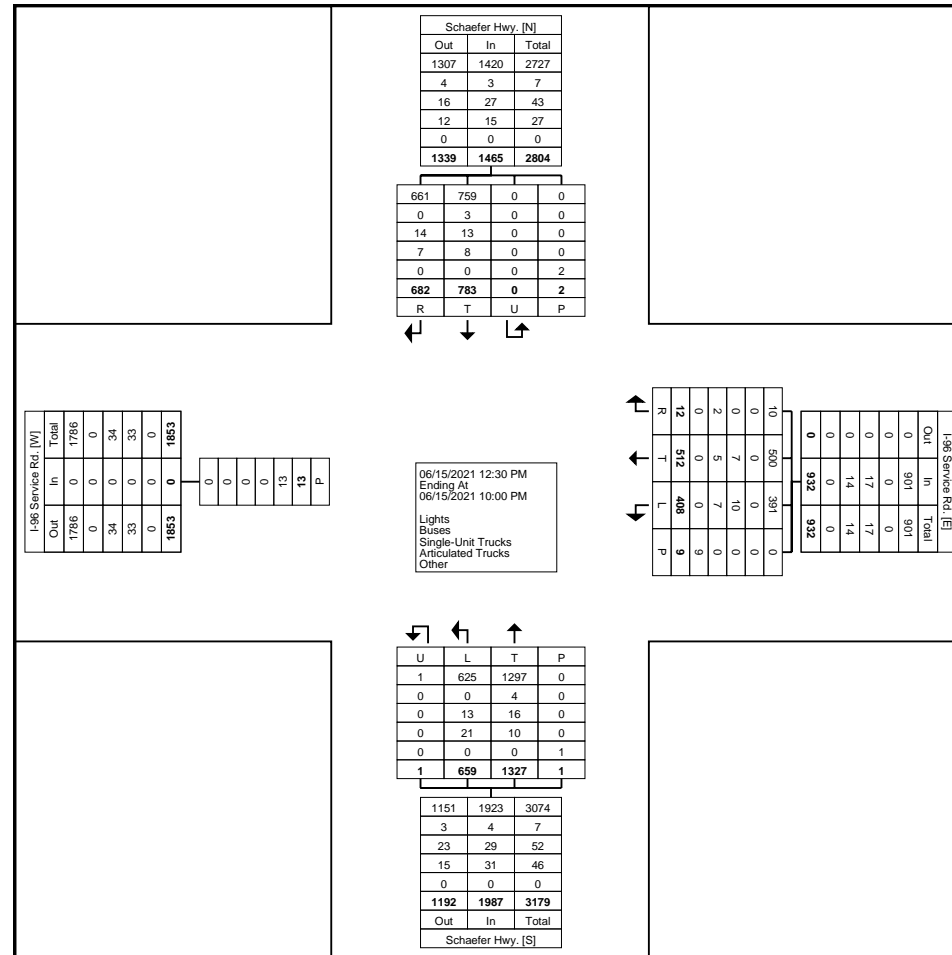
Turning Movement Data

Start Time	Schaefer Hwy. Southbound					I-96 Service Rd. Westbound					Schaefer Hwy. Northbound					I-96 Service Rd. Eastbound		Int. Total
	Right	Thru	U-Turn	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Thru	Left	U-Turn	Peds	App. Total	Peds	App. Total	
12:30 PM	44	42	0	0	86	0	48	29	1	77	77	48	0	0	125	0	0	288
12:45 PM	59	51	0	0	110	3	35	29	0	67	100	50	0	0	150	1	0	327
Hourly Total	103	93	0	0	196	3	83	58	1	144	177	98	0	0	275	1	0	615
1:00 PM	36	49	0	0	85	1	35	19	0	55	92	51	0	0	143	1	0	283
1:15 PM	48	60	0	0	108	0	34	28	1	62	90	45	0	0	135	0	0	305
1:30 PM	54	63	0	1	117	0	26	31	2	57	99	43	0	0	142	1	0	316
1:45 PM	42	54	0	0	96	0	25	33	1	58	111	55	0	0	166	0	0	320
Hourly Total	180	226	0	1	406	1	120	111	4	232	392	194	0	0	586	2	0	1224
2:00 PM	52	60	0	0	112	0	25	20	0	45	92	42	0	0	134	3	0	291
2:15 PM	52	63	0	0	115	3	36	39	0	78	101	43	1	0	145	0	0	338
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	104	123	0	0	227	3	61	59	0	123	193	85	1	0	279	3	0	629
8:00 PM	39	38	0	0	77	2	32	24	1	58	76	38	0	1	114	2	0	249
8:15 PM	47	44	0	0	91	0	33	17	0	50	101	41	0	0	142	1	0	283
8:30 PM	36	52	0	0	88	0	30	31	0	61	69	30	0	0	99	0	0	248
8:45 PM	31	41	0	0	72	1	27	22	2	50	61	33	0	0	94	1	0	216
Hourly Total	153	175	0	0	328	3	122	94	3	219	307	142	0	1	449	4	0	996
9:00 PM	40	46	0	0	86	0	33	23	0	56	63	39	0	0	102	0	0	244
9:15 PM	33	52	0	0	85	0	40	24	0	64	60	29	0	0	89	3	0	238
9:30 PM	36	32	0	1	68	0	35	23	1	58	74	36	0	0	110	0	0	236
9:45 PM	33	36	0	0	69	2	18	16	0	36	61	36	0	0	97	0	0	202
Hourly Total	142	166	0	1	308	2	126	86	1	214	258	140	0	0	398	3	0	920
Grand Total	682	783	0	2	1465	12	512	408	9	932	1327	659	1	1	1987	13	0	4384
Approach %	46.6	53.4	0.0	-	-	1.3	54.9	43.8	-	-	66.8	33.2	0.1	-	-	-	-	-
Total %	15.6	17.9	0.0	-	33.4	0.3	11.7	9.3	-	21.3	30.3	15.0	0.0	-	45.3	-	0.0	-
Lights	661	759	0	-	1420	10	500	391	-	901	1297	625	1	-	1923	-	0	4244
% Lights	96.9	96.9	-	-	96.9	83.3	97.7	95.8	-	96.7	97.7	94.8	100.0	-	96.8	-	-	96.8
Buses	0	3	0	-	3	0	0	0	-	0	4	0	0	-	4	-	0	7
% Buses	0.0	0.4	-	-	0.2	0.0	0.0	0.0	-	0.0	0.3	0.0	0.0	-	0.2	-	-	0.2
Single-Unit Trucks	14	13	0	-	27	0	7	10	-	17	16	13	0	-	29	-	0	73
% Single-Unit Trucks	2.1	1.7	-	-	1.8	0.0	1.4	2.5	-	1.8	1.2	2.0	0.0	-	1.5	-	-	1.7
Articulated Trucks	7	8	0	-	15	2	5	7	-	14	10	21	0	-	31	-	0	60
% Articulated Trucks	1.0	1.0	-	-	1.0	16.7	1.0	1.7	-	1.5	0.8	3.2	0.0	-	1.6	-	-	1.4
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	4	-	-	-	-	0	-	3	-	-
% Bicycles on Crosswalk	-	-	-	0.0	-	-	-	-	44.4	-	-	-	-	0.0	-	23.1	-	-
Pedestrians	-	-	-	2	-	-	-	-	5	-	-	-	-	1	-	10	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	55.6	-	-	-	-	100.0	-	76.9	-	-

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schaefer Hwy & I-96 Service Rd
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 2



Turning Movement Data Plot

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schaefer Hwy & I-96 Service Rd
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 3

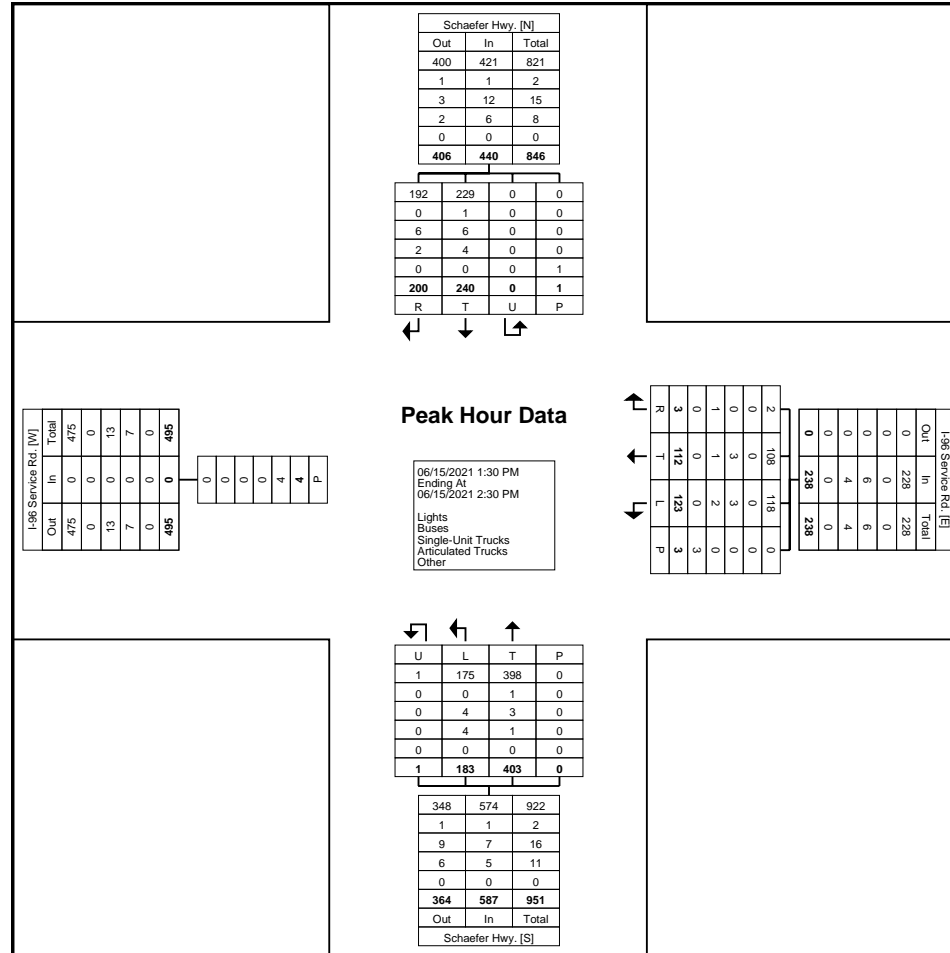
Turning Movement Peak Hour Data (1:30 PM)

Start Time	Schaefer Hwy. Southbound					I-96 Service Rd. Westbound					Schaefer Hwy. Northbound					I-96 Service Rd. Eastbound		Int. Total
	Right	Thru	U-Turn	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Thru	Left	U-Turn	Peds	App. Total	Peds	App. Total	
1:30 PM	54	63	0	1	117	0	26	31	2	57	99	43	0	0	142	1	0	316
1:45 PM	42	54	0	0	96	0	25	33	1	58	111	55	0	0	166	0	0	320
2:00 PM	52	60	0	0	112	0	25	20	0	45	92	42	0	0	134	3	0	291
2:15 PM	52	63	0	0	115	3	36	39	0	78	101	43	1	0	145	0	0	338
Total	200	240	0	1	440	3	112	123	3	238	403	183	1	0	587	4	0	1265
Approach %	45.5	54.5	0.0	-	-	1.3	47.1	51.7	-	-	68.7	31.2	0.2	-	-	-	-	-
Total %	15.8	19.0	0.0	-	34.8	0.2	8.9	9.7	-	18.8	31.9	14.5	0.1	-	46.4	-	0.0	-
PHF	0.926	0.952	0.000	-	0.940	0.250	0.778	0.788	-	0.763	0.908	0.832	0.250	-	0.884	-	0.000	0.936
Lights	192	229	0	-	421	2	108	118	-	228	398	175	1	-	574	-	0	1223
% Lights	96.0	95.4	-	-	95.7	66.7	96.4	95.9	-	95.8	98.8	95.6	100.0	-	97.8	-	-	96.7
Buses	0	1	0	-	1	0	0	0	-	0	1	0	0	-	1	-	0	2
% Buses	0.0	0.4	-	-	0.2	0.0	0.0	0.0	-	0.0	0.2	0.0	0.0	-	0.2	-	-	0.2
Single-Unit Trucks	6	6	0	-	12	0	3	3	-	6	3	4	0	-	7	-	0	25
% Single-Unit Trucks	3.0	2.5	-	-	2.7	0.0	2.7	2.4	-	2.5	0.7	2.2	0.0	-	1.2	-	-	2.0
Articulated Trucks	2	4	0	-	6	1	1	2	-	4	1	4	0	-	5	-	0	15
% Articulated Trucks	1.0	1.7	-	-	1.4	33.3	0.9	1.6	-	1.7	0.2	2.2	0.0	-	0.9	-	-	1.2
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	3	-	-	-	-	0	-	0	-	-
% Bicycles on Crosswalk	-	-	-	0.0	-	-	-	-	100.0	-	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	1	-	-	-	-	0	-	-	-	-	0	-	4	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	0.0	-	-	-	-	-	-	100.0	-	-

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schaefer Hwy & I-96 Service Rd
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 4



Turning Movement Peak Hour Data Plot (1:30 PM)

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schaefer Hwy & I-96 Service Rd
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 5

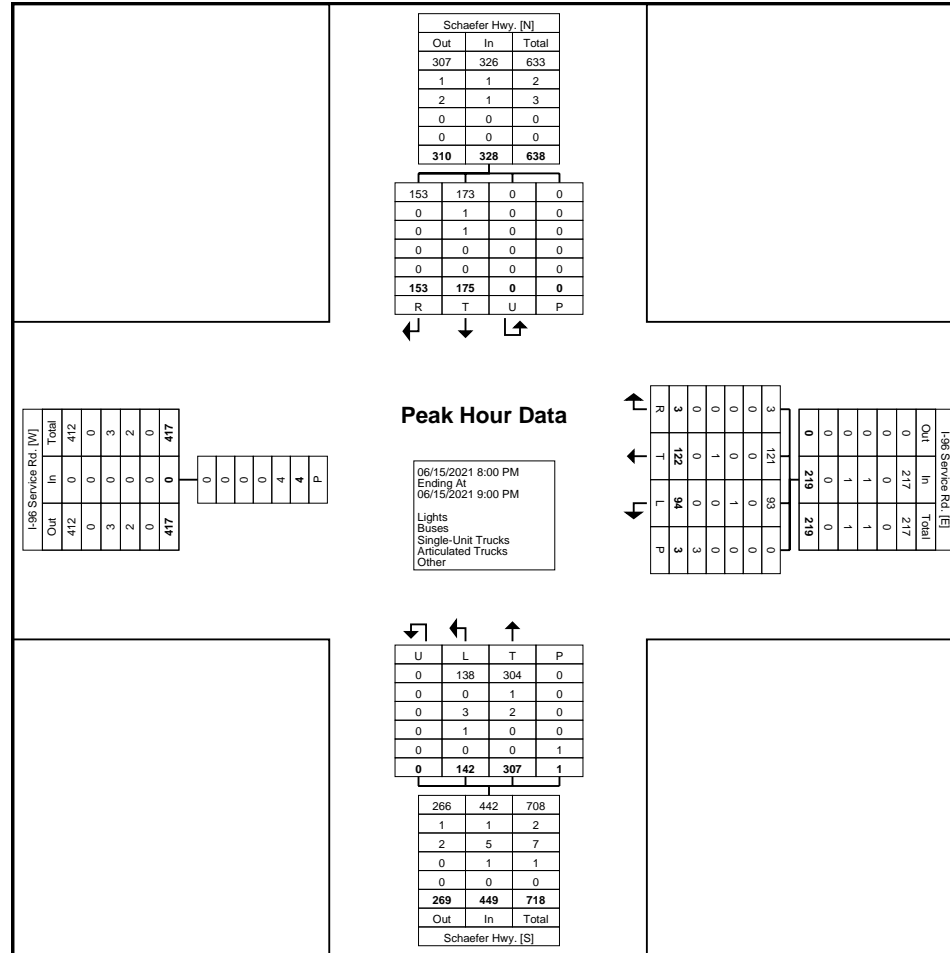
Turning Movement Peak Hour Data (8:00 PM)

Start Time	Schaefer Hwy. Southbound					I-96 Service Rd. Westbound					Schaefer Hwy. Northbound					I-96 Service Rd. Eastbound		Int. Total
	Right	Thru	U-Turn	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Thru	Left	U-Turn	Peds	App. Total	Peds	App. Total	
8:00 PM	39	38	0	0	77	2	32	24	1	58	76	38	0	1	114	2	0	249
8:15 PM	47	44	0	0	91	0	33	17	0	50	101	41	0	0	142	1	0	283
8:30 PM	36	52	0	0	88	0	30	31	0	61	69	30	0	0	99	0	0	248
8:45 PM	31	41	0	0	72	1	27	22	2	50	61	33	0	0	94	1	0	216
Total	153	175	0	0	328	3	122	94	3	219	307	142	0	1	449	4	0	996
Approach %	46.6	53.4	0.0	-	-	1.4	55.7	42.9	-	-	68.4	31.6	0.0	-	-	-	-	-
Total %	15.4	17.6	0.0	-	32.9	0.3	12.2	9.4	-	22.0	30.8	14.3	0.0	-	45.1	-	0.0	-
PHF	0.814	0.841	0.000	-	0.901	0.375	0.924	0.758	-	0.898	0.760	0.866	0.000	-	0.790	-	0.000	0.880
Lights	153	173	0	-	326	3	121	93	-	217	304	138	0	-	442	-	0	985
% Lights	100.0	98.9	-	-	99.4	100.0	99.2	98.9	-	99.1	99.0	97.2	-	-	98.4	-	-	98.9
Buses	0	1	0	-	1	0	0	0	-	0	1	0	0	-	1	-	0	2
% Buses	0.0	0.6	-	-	0.3	0.0	0.0	0.0	-	0.0	0.3	0.0	-	-	0.2	-	-	0.2
Single-Unit Trucks	0	1	0	-	1	0	0	1	-	1	2	3	0	-	5	-	0	7
% Single-Unit Trucks	0.0	0.6	-	-	0.3	0.0	0.0	1.1	-	0.5	0.7	2.1	-	-	1.1	-	-	0.7
Articulated Trucks	0	0	0	-	0	0	1	0	-	1	0	1	0	-	1	-	0	2
% Articulated Trucks	0.0	0.0	-	-	0.0	0.0	0.8	0.0	-	0.5	0.0	0.7	-	-	0.2	-	-	0.2
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	1	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	0.0	-	-	-	-	0.0	-	25.0	-	-
Pedestrians	-	-	-	0	-	-	-	-	3	-	-	-	-	1	-	3	-	-
% Pedestrians	-	-	-	-	-	-	-	-	100.0	-	-	-	-	100.0	-	75.0	-	-

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schaefer Hwy & I-96 Service Rd
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 6



Turning Movement Peak Hour Data Plot (8:00 PM)

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schaefer Hwy & Jefferies Service Dr
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 1

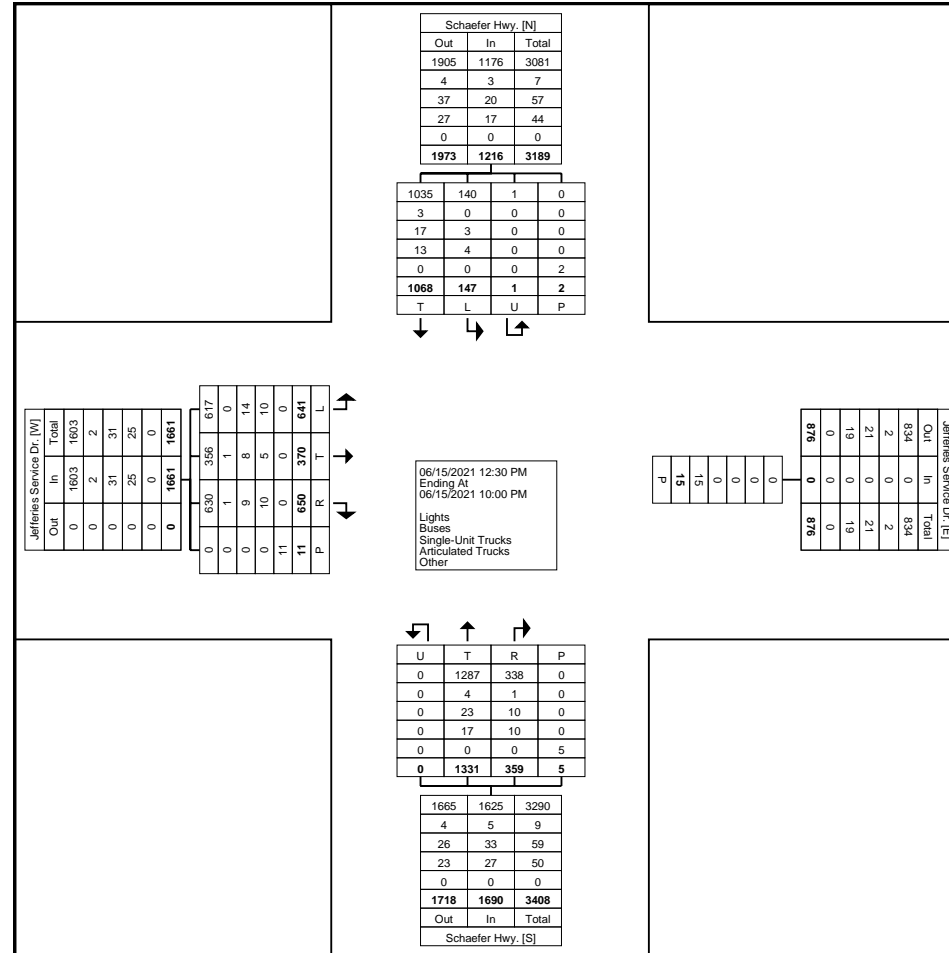
Turning Movement Data

Start Time	Schaefer Hwy. Southbound					Jefferies Service Dr. Westbound		Schaefer Hwy. Northbound					Jefferies Service Dr. Eastbound					Int. Total
	Thru	Left	U-Turn	Peds	App. Total	Peds	App. Total	Right	Thru	U-Turn	Peds	App. Total	Right	Thru	Left	Peds	App. Total	
12:30 PM	66	9	0	0	75	1	0	28	88	0	0	116	43	32	42	0	117	308
12:45 PM	73	5	1	1	79	1	0	31	107	0	1	138	33	21	43	2	97	314
Hourly Total	139	14	1	1	154	2	0	59	195	0	1	254	76	53	85	2	214	622
1:00 PM	64	5	0	0	69	0	0	27	93	0	0	120	34	16	47	1	97	286
1:15 PM	73	16	0	0	89	1	0	25	94	0	0	119	38	25	41	0	104	312
1:30 PM	78	16	0	1	94	2	0	27	82	0	2	109	76	21	53	3	150	353
1:45 PM	81	9	0	0	90	1	0	13	111	0	0	124	59	22	60	0	141	355
Hourly Total	296	46	0	1	342	4	0	92	380	0	2	472	207	84	201	4	492	1306
2:00 PM	73	13	0	0	86	3	0	26	85	0	1	111	52	12	45	1	109	306
2:15 PM	86	13	0	0	99	0	0	30	93	0	0	123	48	36	46	0	130	352
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	159	26	0	0	185	3	0	56	178	0	1	234	100	48	91	1	239	658
8:00 PM	57	8	0	0	65	2	0	27	78	0	0	105	36	27	36	0	99	269
8:15 PM	50	13	0	0	63	0	0	21	103	0	0	124	40	33	39	0	112	299
8:30 PM	77	11	0	0	88	0	0	15	62	0	0	77	41	21	37	0	99	264
8:45 PM	58	10	0	0	68	2	0	15	65	0	0	80	32	22	27	0	81	229
Hourly Total	242	42	0	0	284	4	0	78	308	0	0	386	149	103	139	0	391	1061
9:00 PM	63	3	0	0	66	0	0	17	68	0	1	85	32	23	31	1	86	237
9:15 PM	67	8	0	0	75	1	0	24	65	0	0	89	32	15	28	2	75	239
9:30 PM	52	7	0	0	59	1	0	15	76	0	0	91	32	25	32	0	89	239
9:45 PM	50	1	0	0	51	0	0	18	61	0	0	79	22	19	34	1	75	205
Hourly Total	232	19	0	0	251	2	0	74	270	0	1	344	118	82	125	4	325	920
Grand Total	1068	147	1	2	1216	15	0	359	1331	0	5	1690	650	370	641	11	1661	4567
Approach %	87.8	12.1	0.1	-	-	-	-	21.2	78.8	0.0	-	-	39.1	22.3	38.6	-	-	-
Total %	23.4	3.2	0.0	-	26.6	-	0.0	7.9	29.1	0.0	-	37.0	14.2	8.1	14.0	-	36.4	-
Lights	1035	140	1	-	1176	-	0	338	1287	0	-	1625	630	356	617	-	1603	4404
% Lights	96.9	95.2	100.0	-	96.7	-	-	94.2	96.7	-	-	96.2	96.9	96.2	96.3	-	96.5	96.4
Buses	3	0	0	-	3	-	0	1	4	0	-	5	1	1	0	-	2	10
% Buses	0.3	0.0	0.0	-	0.2	-	-	0.3	0.3	-	-	0.3	0.2	0.3	0.0	-	0.1	0.2
Single-Unit Trucks	17	3	0	-	20	-	0	10	23	0	-	33	9	8	14	-	31	84
% Single-Unit Trucks	1.6	2.0	0.0	-	1.6	-	-	2.8	1.7	-	-	2.0	1.4	2.2	2.2	-	1.9	1.8
Articulated Trucks	13	4	0	-	17	-	0	10	17	0	-	27	10	5	10	-	25	69
% Articulated Trucks	1.2	2.7	0.0	-	1.4	-	-	2.8	1.3	-	-	1.6	1.5	1.4	1.6	-	1.5	1.5
Bicycles on Crosswalk	-	-	-	0	-	4	-	-	-	-	0	-	-	-	-	2	-	-
% Bicycles on Crosswalk	-	-	-	0.0	-	26.7	-	-	-	-	0.0	-	-	-	-	18.2	-	-
Pedestrians	-	-	-	2	-	11	-	-	-	-	5	-	-	-	-	9	-	-
% Pedestrians	-	-	-	100.0	-	73.3	-	-	-	-	100.0	-	-	-	-	81.8	-	-

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schaefer Hwy & Jefferies Service Dr
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 2



Turning Movement Data Plot

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schaefer Hwy & Jefferies Service Dr
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 3

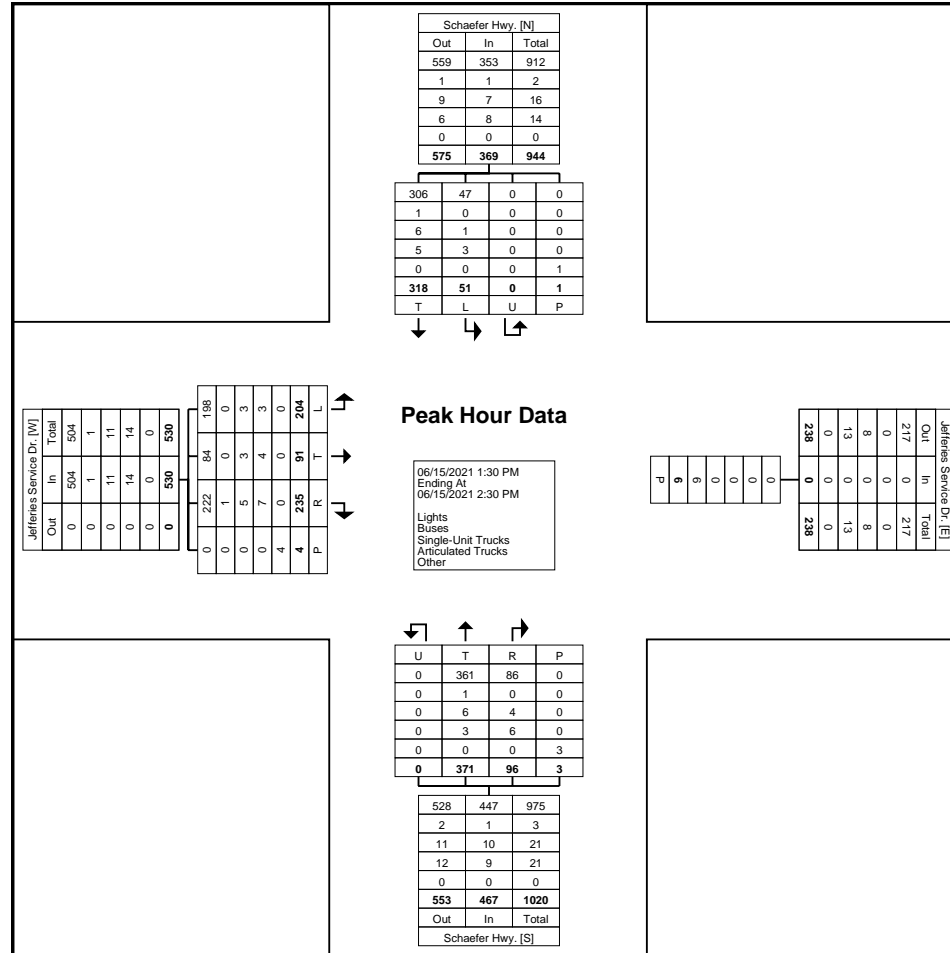
Turning Movement Peak Hour Data (1:30 PM)

Start Time	Schaefer Hwy. Southbound					Jefferies Service Dr. Westbound		Schaefer Hwy. Northbound					Jefferies Service Dr. Eastbound					Int. Total
	Thru	Left	U-Turn	Peds	App. Total	Peds	App. Total	Right	Thru	U-Turn	Peds	App. Total	Right	Thru	Left	Peds	App. Total	
1:30 PM	78	16	0	1	94	2	0	27	82	0	2	109	76	21	53	3	150	353
1:45 PM	81	9	0	0	90	1	0	13	111	0	0	124	59	22	60	0	141	355
2:00 PM	73	13	0	0	86	3	0	26	85	0	1	111	52	12	45	1	109	306
2:15 PM	86	13	0	0	99	0	0	30	93	0	0	123	48	36	46	0	130	352
Total	318	51	0	1	369	6	0	96	371	0	3	467	235	91	204	4	530	1366
Approach %	86.2	13.8	0.0	-	-	-	-	20.6	79.4	0.0	-	-	44.3	17.2	38.5	-	-	-
Total %	23.3	3.7	0.0	-	27.0	-	0.0	7.0	27.2	0.0	-	34.2	17.2	6.7	14.9	-	38.8	-
PHF	0.924	0.797	0.000	-	0.932	-	0.000	0.800	0.836	0.000	-	0.942	0.773	0.632	0.850	-	0.883	0.962
Lights	306	47	0	-	353	-	0	86	361	0	-	447	222	84	198	-	504	1304
% Lights	96.2	92.2	-	-	95.7	-	-	89.6	97.3	-	-	95.7	94.5	92.3	97.1	-	95.1	95.5
Buses	1	0	0	-	1	-	0	0	1	0	-	1	1	0	0	-	1	3
% Buses	0.3	0.0	-	-	0.3	-	-	0.0	0.3	-	-	0.2	0.4	0.0	0.0	-	0.2	0.2
Single-Unit Trucks	6	1	0	-	7	-	0	4	6	0	-	10	5	3	3	-	11	28
% Single-Unit Trucks	1.9	2.0	-	-	1.9	-	-	4.2	1.6	-	-	2.1	2.1	3.3	1.5	-	2.1	2.0
Articulated Trucks	5	3	0	-	8	-	0	6	3	0	-	9	7	4	3	-	14	31
% Articulated Trucks	1.6	5.9	-	-	2.2	-	-	6.3	0.8	-	-	1.9	3.0	4.4	1.5	-	2.6	2.3
Bicycles on Crosswalk	-	-	-	0	-	3	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	0.0	-	50.0	-	-	-	-	0.0	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	1	-	3	-	-	-	-	3	-	-	-	-	4	-	-
% Pedestrians	-	-	-	100.0	-	50.0	-	-	-	-	100.0	-	-	-	-	100.0	-	-

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schaefer Hwy & Jefferies Service Dr
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 4

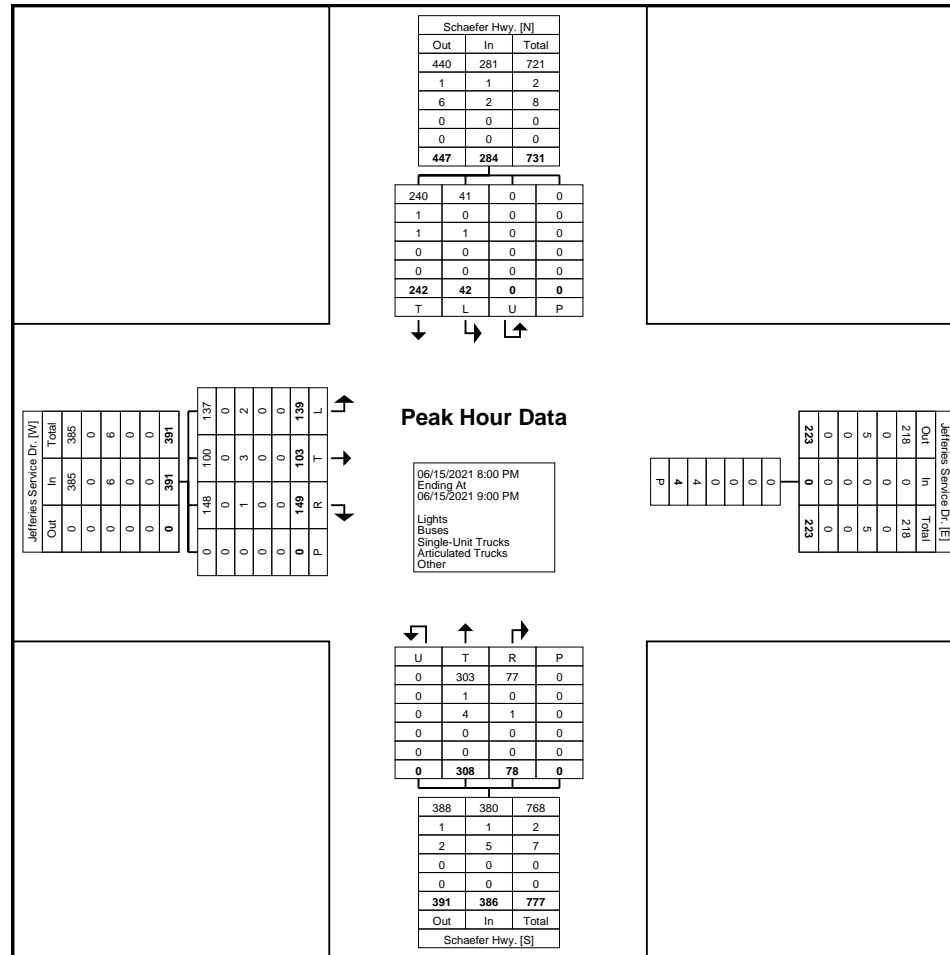


Turning Movement Peak Hour Data Plot (1:30 PM)

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Schaefer Hwy & Jefferies Service Dr
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 6



Turning Movement Peak Hour Data Plot (8:00 PM)

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Grand River Ave & Jefferies
 Service Dr
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 1

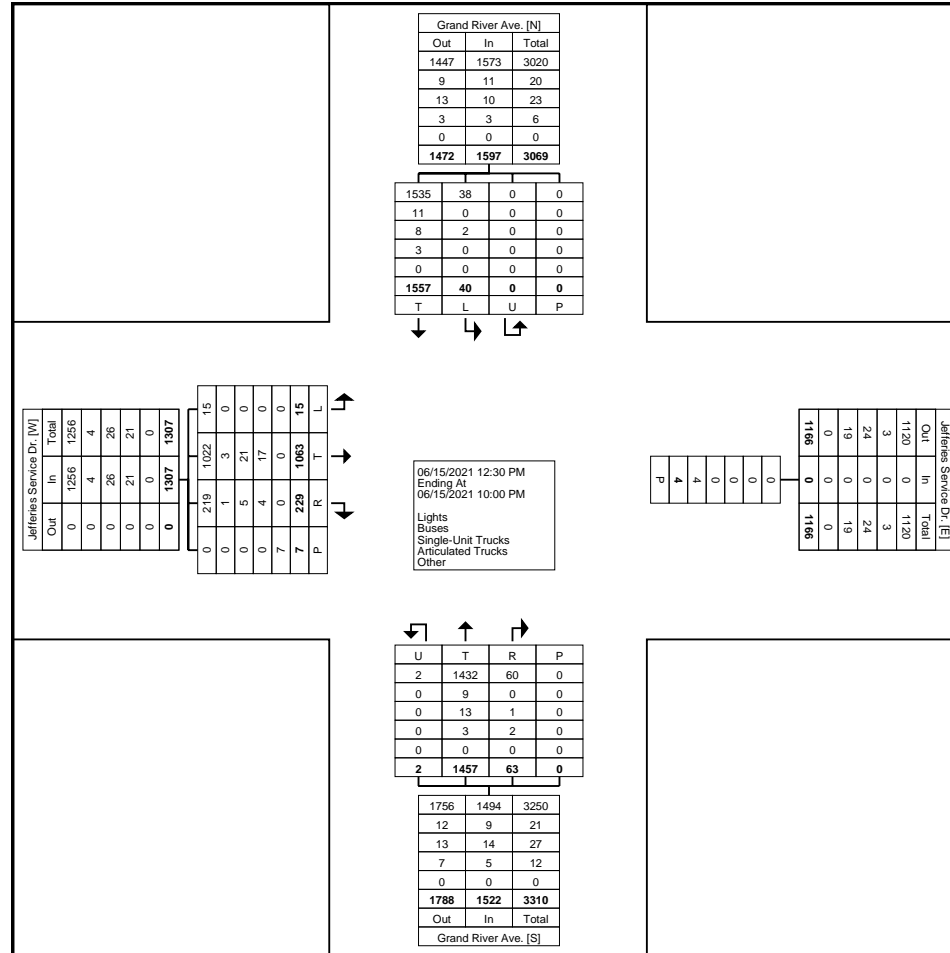
Turning Movement Data

Start Time	Grand River Ave. Southbound					Jefferies Service Dr. Westbound		Grand River Ave. Northbound					Jefferies Service Dr. Eastbound					Int. Total
	Thru	Left	U-Turn	Peds	App. Total	Peds	App. Total	Right	Thru	U-Turn	Peds	App. Total	Right	Thru	Left	Peds	App. Total	
12:30 PM	96	7	0	0	103	0	0	3	115	1	0	119	13	81	3	0	97	319
12:45 PM	118	2	0	0	120	0	0	3	78	0	0	81	16	59	1	0	76	277
Hourly Total	214	9	0	0	223	0	0	6	193	1	0	200	29	140	4	0	173	596
1:00 PM	101	8	0	0	109	0	0	6	112	0	0	118	9	72	0	0	81	308
1:15 PM	106	1	0	0	107	0	0	7	100	1	0	108	18	67	0	0	85	300
1:30 PM	117	0	0	0	117	0	0	1	109	0	0	110	14	76	0	0	90	317
1:45 PM	107	0	0	0	107	0	0	4	104	0	0	108	12	69	2	1	83	298
Hourly Total	431	9	0	0	440	0	0	18	425	1	0	444	53	284	2	1	339	1223
2:00 PM	100	4	0	0	104	0	0	3	92	0	0	95	6	68	2	0	76	275
2:15 PM	122	1	0	0	123	1	0	11	98	0	0	109	24	86	2	1	112	344
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	222	5	0	0	227	1	0	14	190	0	0	204	30	154	4	1	188	619
8:00 PM	99	3	0	0	102	0	0	2	117	0	0	119	16	68	2	1	86	307
8:15 PM	99	2	0	0	101	1	0	2	81	0	0	83	24	70	1	0	95	279
8:30 PM	105	0	0	0	105	2	0	5	66	0	0	71	12	70	1	1	83	259
8:45 PM	101	4	0	0	105	0	0	4	66	0	0	70	10	54	0	1	64	239
Hourly Total	404	9	0	0	413	3	0	13	330	0	0	343	62	262	4	3	328	1084
9:00 PM	81	0	0	0	81	0	0	1	93	0	0	94	18	53	0	0	71	246
9:15 PM	73	1	0	0	74	0	0	3	89	0	0	92	7	65	0	2	72	238
9:30 PM	63	3	0	0	66	0	0	6	71	0	0	77	21	49	1	0	71	214
9:45 PM	69	4	0	0	73	0	0	2	66	0	0	68	9	56	0	0	65	206
Hourly Total	286	8	0	0	294	0	0	12	319	0	0	331	55	223	1	2	279	904
Grand Total	1557	40	0	0	1597	4	0	63	1457	2	0	1522	229	1063	15	7	1307	4426
Approach %	97.5	2.5	0.0	-	-	-	-	4.1	95.7	0.1	-	-	17.5	81.3	1.1	-	-	-
Total %	35.2	0.9	0.0	-	36.1	-	0.0	1.4	32.9	0.0	-	34.4	5.2	24.0	0.3	-	29.5	-
Lights	1535	38	0	-	1573	-	0	60	1432	2	-	1494	219	1022	15	-	1256	4323
% Lights	98.6	95.0	-	-	98.5	-	-	95.2	98.3	100.0	-	98.2	95.6	96.1	100.0	-	96.1	97.7
Buses	11	0	0	-	11	-	0	0	9	0	-	9	1	3	0	-	4	24
% Buses	0.7	0.0	-	-	0.7	-	-	0.0	0.6	0.0	-	0.6	0.4	0.3	0.0	-	0.3	0.5
Single-Unit Trucks	8	2	0	-	10	-	0	1	13	0	-	14	5	21	0	-	26	50
% Single-Unit Trucks	0.5	5.0	-	-	0.6	-	-	1.6	0.9	0.0	-	0.9	2.2	2.0	0.0	-	2.0	1.1
Articulated Trucks	3	0	0	-	3	-	0	2	3	0	-	5	4	17	0	-	21	29
% Articulated Trucks	0.2	0.0	-	-	0.2	-	-	3.2	0.2	0.0	-	0.3	1.7	1.6	0.0	-	1.6	0.7
Bicycles on Crosswalk	-	-	-	0	-	3	-	-	-	-	0	-	-	-	-	4	-	-
% Bicycles on Crosswalk	-	-	-	-	-	75.0	-	-	-	-	-	-	-	-	-	57.1	-	-
Pedestrians	-	-	-	0	-	1	-	-	-	-	0	-	-	-	-	3	-	-
% Pedestrians	-	-	-	-	-	25.0	-	-	-	-	-	-	-	-	-	42.9	-	-

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Grand River Ave & Jefferies
 Service Dr
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 2



Turning Movement Data Plot

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Grand River Ave & Jefferies
 Service Dr
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 3

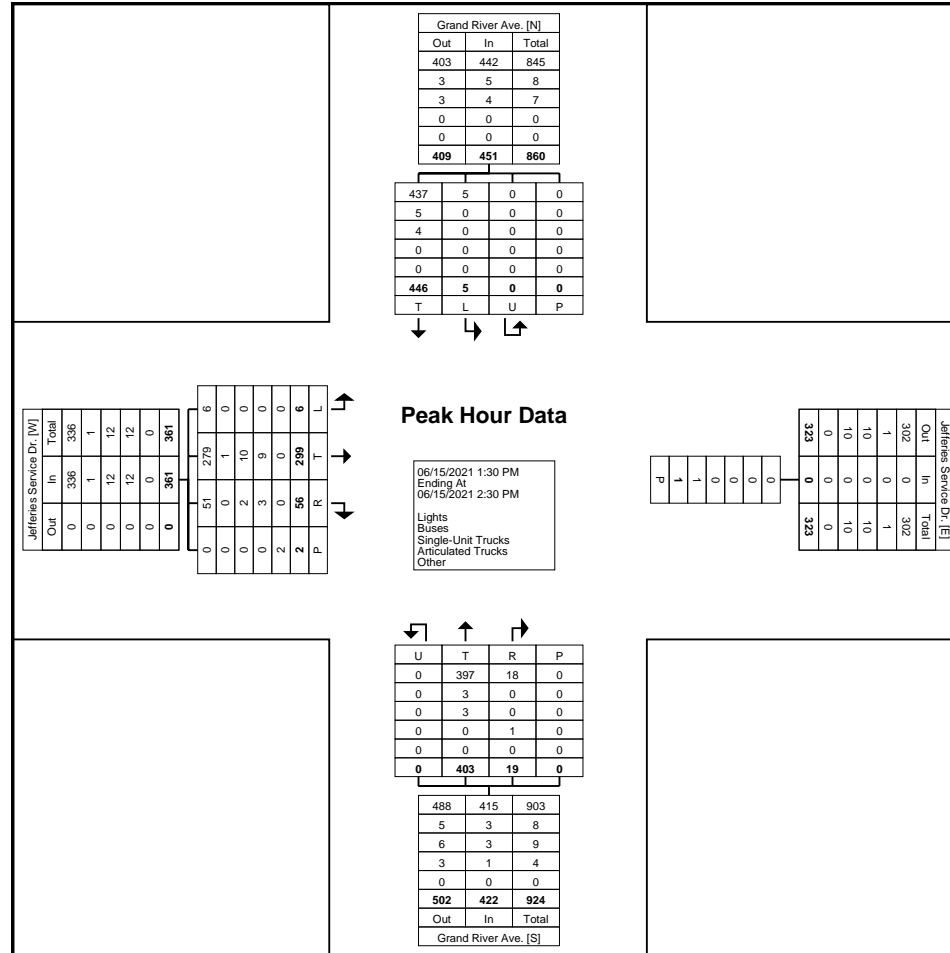
Turning Movement Peak Hour Data (1:30 PM)

Start Time	Grand River Ave. Southbound					Jefferies Service Dr. Westbound		Grand River Ave. Northbound					Jefferies Service Dr. Eastbound					Int. Total
	Thru	Left	U-Turn	Peds	App. Total	Peds	App. Total	Right	Thru	U-Turn	Peds	App. Total	Right	Thru	Left	Peds	App. Total	
1:30 PM	117	0	0	0	117	0	0	1	109	0	0	110	14	76	0	0	90	317
1:45 PM	107	0	0	0	107	0	0	4	104	0	0	108	12	69	2	1	83	298
2:00 PM	100	4	0	0	104	0	0	3	92	0	0	95	6	68	2	0	76	275
2:15 PM	122	1	0	0	123	1	0	11	98	0	0	109	24	86	2	1	112	344
Total	446	5	0	0	451	1	0	19	403	0	0	422	56	299	6	2	361	1234
Approach %	98.9	1.1	0.0	-	-	-	-	4.5	95.5	0.0	-	-	15.5	82.8	1.7	-	-	-
Total %	36.1	0.4	0.0	-	36.5	-	0.0	1.5	32.7	0.0	-	34.2	4.5	24.2	0.5	-	29.3	-
PHF	0.914	0.313	0.000	-	0.917	-	0.000	0.432	0.924	0.000	-	0.959	0.583	0.869	0.750	-	0.806	0.897
Lights	437	5	0	-	442	-	0	18	397	0	-	415	51	279	6	-	336	1193
% Lights	98.0	100.0	-	-	98.0	-	-	94.7	98.5	-	-	98.3	91.1	93.3	100.0	-	93.1	96.7
Buses	5	0	0	-	5	-	0	0	3	0	-	3	0	1	0	-	1	9
% Buses	1.1	0.0	-	-	1.1	-	-	0.0	0.7	-	-	0.7	0.0	0.3	0.0	-	0.3	0.7
Single-Unit Trucks	4	0	0	-	4	-	0	0	3	0	-	3	2	10	0	-	12	19
% Single-Unit Trucks	0.9	0.0	-	-	0.9	-	-	0.0	0.7	-	-	0.7	3.6	3.3	0.0	-	3.3	1.5
Articulated Trucks	0	0	0	-	0	-	0	1	0	0	-	1	3	9	0	-	12	13
% Articulated Trucks	0.0	0.0	-	-	0.0	-	-	5.3	0.0	-	-	0.2	5.4	3.0	0.0	-	3.3	1.1
Bicycles on Crosswalk	-	-	-	0	-	0	-	-	-	-	0	-	-	-	-	2	-	-
% Bicycles on Crosswalk	-	-	-	-	-	0.0	-	-	-	-	-	-	-	-	-	100.0	-	-
Pedestrians	-	-	-	0	-	1	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	0.0	-	-

Unit Number:

DLZ
8430 West Bryn Mawr Avenue, Suite 100
Chicago, Illinois, United States 60631
(773) 283-2600

Count Name: Grand River Ave & Jefferies
Service Dr
Site Code: 1942-6994-50/0800/2000
Start Date: 06/15/2021
Page No: 4



Turning Movement Peak Hour Data Plot (1:30 PM)

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Grand River Ave & Jefferies
 Service Dr
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 5

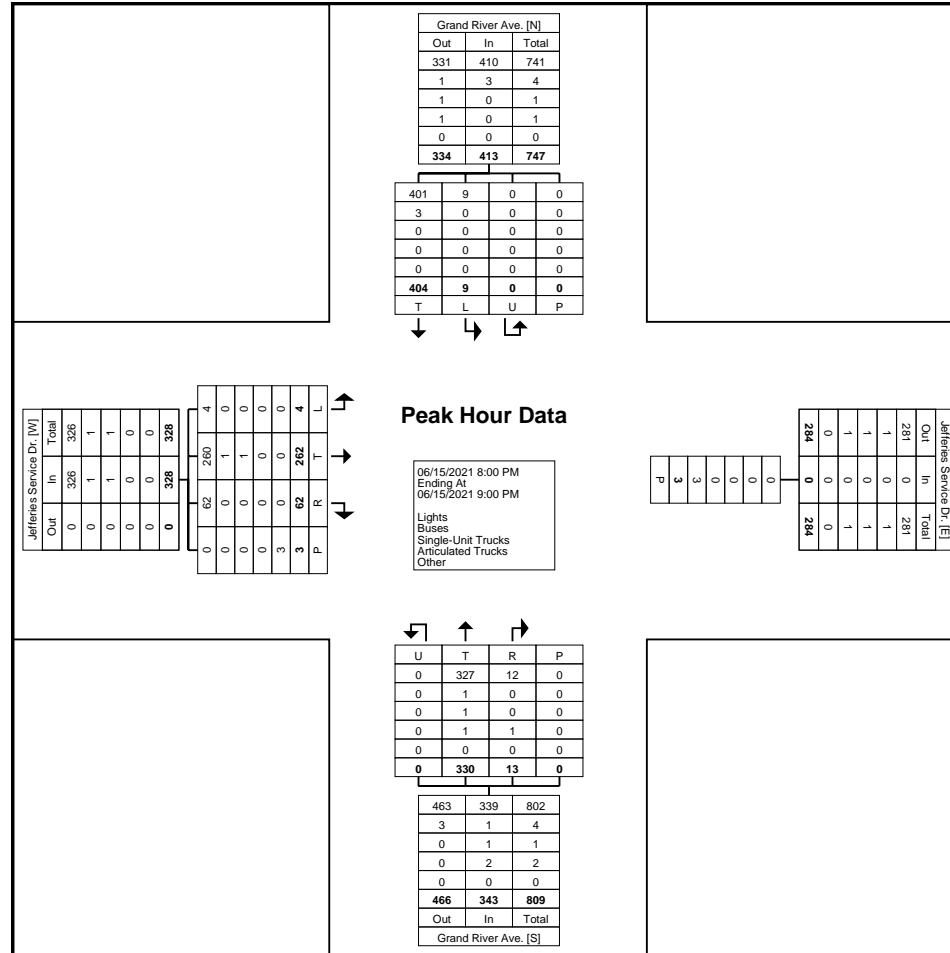
Turning Movement Peak Hour Data (8:00 PM)

Start Time	Grand River Ave. Southbound					Jefferies Service Dr. Westbound		Grand River Ave. Northbound					Jefferies Service Dr. Eastbound					Int. Total
	Thru	Left	U-Turn	Peds	App. Total	Peds	App. Total	Right	Thru	U-Turn	Peds	App. Total	Right	Thru	Left	Peds	App. Total	
8:00 PM	99	3	0	0	102	0	0	2	117	0	0	119	16	68	2	1	86	307
8:15 PM	99	2	0	0	101	1	0	2	81	0	0	83	24	70	1	0	95	279
8:30 PM	105	0	0	0	105	2	0	5	66	0	0	71	12	70	1	1	83	259
8:45 PM	101	4	0	0	105	0	0	4	66	0	0	70	10	54	0	1	64	239
Total	404	9	0	0	413	3	0	13	330	0	0	343	62	262	4	3	328	1084
Approach %	97.8	2.2	0.0	-	-	-	-	3.8	96.2	0.0	-	-	18.9	79.9	1.2	-	-	-
Total %	37.3	0.8	0.0	-	38.1	-	0.0	1.2	30.4	0.0	-	31.6	5.7	24.2	0.4	-	30.3	-
PHF	0.962	0.563	0.000	-	0.983	-	0.000	0.650	0.705	0.000	-	0.721	0.646	0.936	0.500	-	0.863	0.883
Lights	401	9	0	-	410	-	0	12	327	0	-	339	62	260	4	-	326	1075
% Lights	99.3	100.0	-	-	99.3	-	-	92.3	99.1	-	-	98.8	100.0	99.2	100.0	-	99.4	99.2
Buses	3	0	0	-	3	-	0	0	1	0	-	1	0	1	0	-	1	5
% Buses	0.7	0.0	-	-	0.7	-	-	0.0	0.3	-	-	0.3	0.0	0.4	0.0	-	0.3	0.5
Single-Unit Trucks	0	0	0	-	0	-	0	0	1	0	-	1	0	1	0	-	1	2
% Single-Unit Trucks	0.0	0.0	-	-	0.0	-	-	0.0	0.3	-	-	0.3	0.0	0.4	0.0	-	0.3	0.2
Articulated Trucks	0	0	0	-	0	-	0	1	1	0	-	2	0	0	0	-	0	2
% Articulated Trucks	0.0	0.0	-	-	0.0	-	-	7.7	0.3	-	-	0.6	0.0	0.0	0.0	-	0.0	0.2
Bicycles on Crosswalk	-	-	-	0	-	3	-	-	-	-	0	-	-	-	-	1	-	-
% Bicycles on Crosswalk	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	33.3	-	-
Pedestrians	-	-	-	0	-	0	-	-	-	-	0	-	-	-	-	2	-	-
% Pedestrians	-	-	-	-	-	0.0	-	-	-	-	-	-	-	-	-	66.7	-	-

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Grand River Ave & Jefferies
 Service Dr
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 6



Turning Movement Peak Hour Data Plot (8:00 PM)

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Kendall St & Meyers Rd
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 1

Turning Movement Data

Start Time	Meyers Rd. Southbound						Kendall St. Westbound						Meyers Rd. Northbound						Kendall St. Eastbound						Int. Total
	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	
12:30 PM	2	55	1	0	0	58	1	3	3	0	0	7	3	62	0	0	0	65	0	0	1	0	0	1	131
12:45 PM	0	60	1	0	0	61	0	2	1	0	1	3	6	67	0	0	1	73	5	0	1	0	1	6	143
Hourly Total	2	115	2	0	0	119	1	5	4	0	1	10	9	129	0	0	1	138	5	0	2	0	1	7	274
1:00 PM	1	70	3	0	0	74	3	2	2	0	1	7	1	76	0	0	1	77	1	0	1	0	0	2	160
1:15 PM	0	74	1	0	0	75	2	1	1	0	0	4	2	64	1	0	1	67	2	1	1	0	1	4	150
1:30 PM	0	67	2	0	0	69	2	1	3	0	1	6	1	83	1	0	0	85	0	0	0	0	0	0	160
1:45 PM	1	56	0	0	0	57	0	0	1	0	0	1	3	62	1	0	0	66	2	1	0	0	1	3	127
Hourly Total	2	267	6	0	0	275	7	4	7	0	2	18	7	285	3	0	2	295	5	2	2	0	2	9	597
2:00 PM	0	79	3	0	0	82	0	0	1	0	0	1	2	60	1	0	0	63	2	0	1	0	0	3	149
2:15 PM	0	73	0	0	0	73	2	0	1	0	0	3	4	76	1	0	0	81	0	0	2	0	0	2	159
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	152	3	0	0	155	2	0	2	0	0	4	6	136	2	0	0	144	2	0	3	0	0	5	308
8:00 PM	1	60	1	0	0	62	1	1	0	0	0	2	8	62	1	0	0	71	0	0	0	0	0	0	135
8:15 PM	1	58	1	0	0	60	5	0	0	0	0	5	2	44	1	0	0	47	4	1	0	0	0	5	117
8:30 PM	1	59	3	0	0	63	0	0	1	0	0	1	3	47	1	0	1	51	0	0	1	0	2	1	116
8:45 PM	0	59	0	0	0	59	1	0	2	0	0	3	3	47	1	0	0	51	2	0	1	0	0	3	116
Hourly Total	3	236	5	0	0	244	7	1	3	0	0	11	16	200	4	0	1	220	6	1	2	0	2	9	484
9:00 PM	5	42	1	0	0	48	1	0	2	0	0	3	2	49	0	0	0	51	0	0	2	0	0	2	104
9:15 PM	0	41	1	0	0	42	1	0	0	0	0	1	2	46	1	0	1	49	0	0	0	0	0	0	92
9:30 PM	0	50	0	0	0	50	2	0	1	0	0	3	2	41	0	0	0	43	0	1	0	0	1	1	97
9:45 PM	1	44	2	0	0	47	0	0	7	0	0	7	6	44	2	0	0	52	1	0	2	0	0	3	109
Hourly Total	6	177	4	0	0	187	4	0	10	0	0	14	12	180	3	0	1	195	1	1	4	0	1	6	402
Grand Total	13	947	20	0	0	980	21	10	26	0	3	57	50	930	12	0	5	992	19	4	13	0	6	36	2065
Approach %	1.3	96.6	2.0	0.0	-	-	36.8	17.5	45.6	0.0	-	-	5.0	93.8	1.2	0.0	-	-	52.8	11.1	36.1	0.0	-	-	-
Total %	0.6	45.9	1.0	0.0	-	47.5	1.0	0.5	1.3	0.0	-	2.8	2.4	45.0	0.6	0.0	-	48.0	0.9	0.2	0.6	0.0	-	1.7	-
Lights	13	930	18	0	-	961	21	10	26	0	-	57	50	901	12	0	-	963	19	4	13	0	-	36	2017
% Lights	100.0	98.2	90.0	-	-	98.1	100.0	100.0	100.0	-	-	100.0	100.0	96.9	100.0	-	-	97.1	100.0	100.0	100.0	-	-	100.0	97.7
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	3	0	0	-	3	0	0	0	0	-	0	3
% Buses	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.3	0.0	-	-	0.3	0.0	0.0	0.0	-	-	0.0	0.1
Single-Unit Trucks	0	17	2	0	-	19	0	0	0	0	-	0	0	16	0	0	-	16	0	0	0	0	-	0	35
% Single-Unit Trucks	0.0	1.8	10.0	-	-	1.9	0.0	0.0	0.0	-	-	0.0	0.0	1.7	0.0	-	-	1.6	0.0	0.0	0.0	-	-	0.0	1.7
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	10	0	0	-	10	0	0	0	0	-	0	10
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	1.1	0.0	-	-	1.0	0.0	0.0	0.0	-	-	0.0	0.5
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	1	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	16.7	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	3	-	-	-	-	-	5	-	-	-	-	-	5	-	-

% Pedestrians	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	83.3	-	-
---------------	---	---	---	---	---	---	---	---	---	-------	---	---	---	---	---	-------	---	---	---	---	---	------	---	---

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Kendall St & Meyers Rd
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 4

Turning Movement Peak Hour Data (12:45 PM)

Start Time	Meyers Rd. Southbound						Kendall St. Westbound						Meyers Rd. Northbound						Kendall St. Eastbound						Int. Total
	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	
12:45 PM	0	60	1	0	0	61	0	2	1	0	1	3	6	67	0	0	1	73	5	0	1	0	1	6	143
1:00 PM	1	70	3	0	0	74	3	2	2	0	1	7	1	76	0	0	1	77	1	0	1	0	0	2	160
1:15 PM	0	74	1	0	0	75	2	1	1	0	0	4	2	64	1	0	1	67	2	1	1	0	1	4	150
1:30 PM	0	67	2	0	0	69	2	1	3	0	1	6	1	83	1	0	0	85	0	0	0	0	0	0	160
Total	1	271	7	0	0	279	7	6	7	0	3	20	10	290	2	0	3	302	8	1	3	0	2	12	613
Approach %	0.4	97.1	2.5	0.0	-	-	35.0	30.0	35.0	0.0	-	-	3.3	96.0	0.7	0.0	-	-	66.7	8.3	25.0	0.0	-	-	-
Total %	0.2	44.2	1.1	0.0	-	45.5	1.1	1.0	1.1	0.0	-	3.3	1.6	47.3	0.3	0.0	-	49.3	1.3	0.2	0.5	0.0	-	2.0	-
PHF	0.250	0.916	0.583	0.000	-	0.930	0.583	0.750	0.583	0.000	-	0.714	0.417	0.873	0.500	0.000	-	0.888	0.400	0.250	0.750	0.000	-	0.500	0.958
Lights	1	260	5	0	-	266	7	6	7	0	-	20	10	277	2	0	-	289	8	1	3	0	-	12	587
% Lights	100.0	95.9	71.4	-	-	95.3	100.0	100.0	100.0	-	-	100.0	100.0	95.5	100.0	-	-	95.7	100.0	100.0	100.0	-	-	100.0	95.8
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Buses	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Single-Unit Trucks	0	11	2	0	-	13	0	0	0	0	-	0	0	5	0	0	-	5	0	0	0	0	-	0	18
% Single-Unit Trucks	0.0	4.1	28.6	-	-	4.7	0.0	0.0	0.0	-	-	0.0	0.0	1.7	0.0	-	-	1.7	0.0	0.0	0.0	-	-	0.0	2.9
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	8	0	0	-	8	0	0	0	0	-	0	8
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	2.8	0.0	-	-	2.6	0.0	0.0	0.0	-	-	0.0	1.3
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	3	-	-	-	-	-	3	-	-	-	-	-	2	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Kendall St & Meyers Rd
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 6

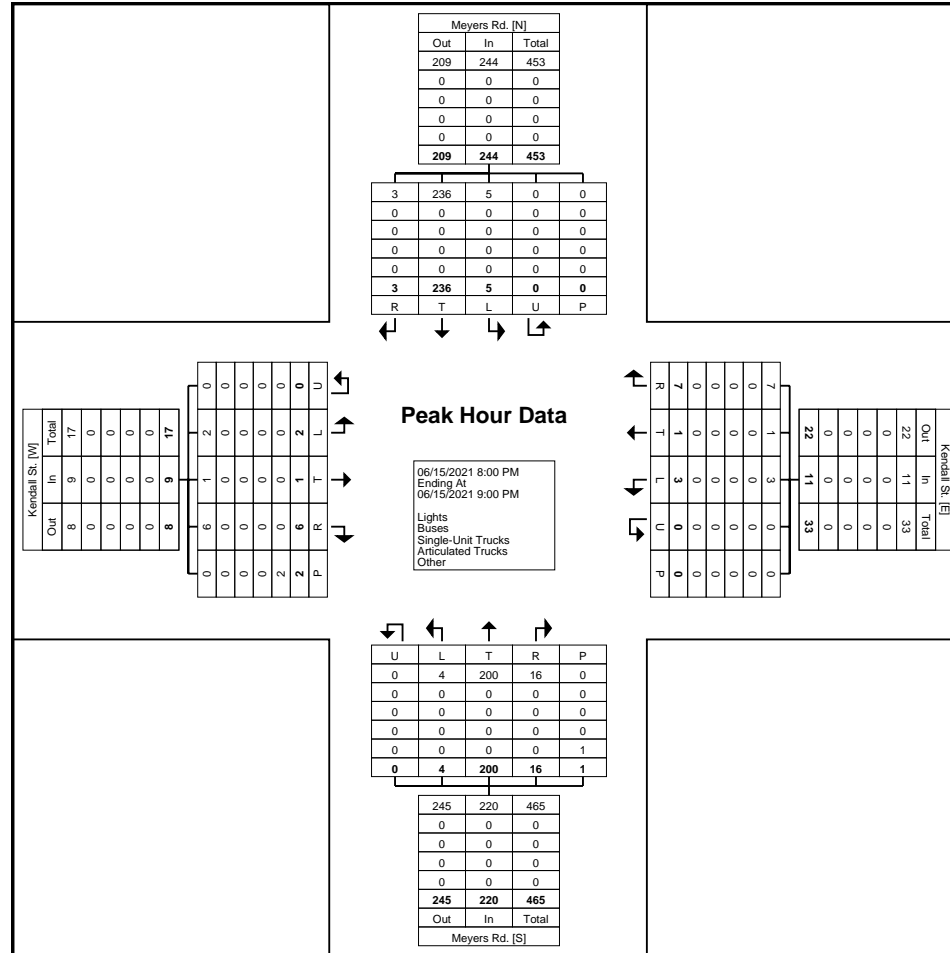
Turning Movement Peak Hour Data (8:00 PM)

Start Time	Meyers Rd. Southbound						Kendall St. Westbound						Meyers Rd. Northbound						Kendall St. Eastbound						Int. Total
	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	
8:00 PM	1	60	1	0	0	62	1	1	0	0	0	2	8	62	1	0	0	71	0	0	0	0	0	0	135
8:15 PM	1	58	1	0	0	60	5	0	0	0	0	5	2	44	1	0	0	47	4	1	0	0	0	5	117
8:30 PM	1	59	3	0	0	63	0	0	1	0	0	1	3	47	1	0	1	51	0	0	1	0	2	1	116
8:45 PM	0	59	0	0	0	59	1	0	2	0	0	3	3	47	1	0	0	51	2	0	1	0	0	3	116
Total	3	236	5	0	0	244	7	1	3	0	0	11	16	200	4	0	1	220	6	1	2	0	2	9	484
Approach %	1.2	96.7	2.0	0.0	-	-	63.6	9.1	27.3	0.0	-	-	7.3	90.9	1.8	0.0	-	-	66.7	11.1	22.2	0.0	-	-	-
Total %	0.6	48.8	1.0	0.0	-	50.4	1.4	0.2	0.6	0.0	-	2.3	3.3	41.3	0.8	0.0	-	45.5	1.2	0.2	0.4	0.0	-	1.9	-
PHF	0.750	0.983	0.417	0.000	-	0.968	0.350	0.250	0.375	0.000	-	0.550	0.500	0.806	1.000	0.000	-	0.775	0.375	0.250	0.500	0.000	-	0.450	0.896
Lights	3	236	5	0	-	244	7	1	3	0	-	11	16	200	4	0	-	220	6	1	2	0	-	9	484
% Lights	100.0	100.0	100.0	-	-	100.0	100.0	100.0	100.0	-	-	100.0	100.0	100.0	100.0	-	-	100.0	100.0	100.0	100.0	-	-	100.0	100.0
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Buses	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Single-Unit Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Single-Unit Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	-	2	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Kendall St & Meyers Rd
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 7



Turning Movement Peak Hour Data Plot (8:00 PM)

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Ward Ave & Schoolcraft
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 1

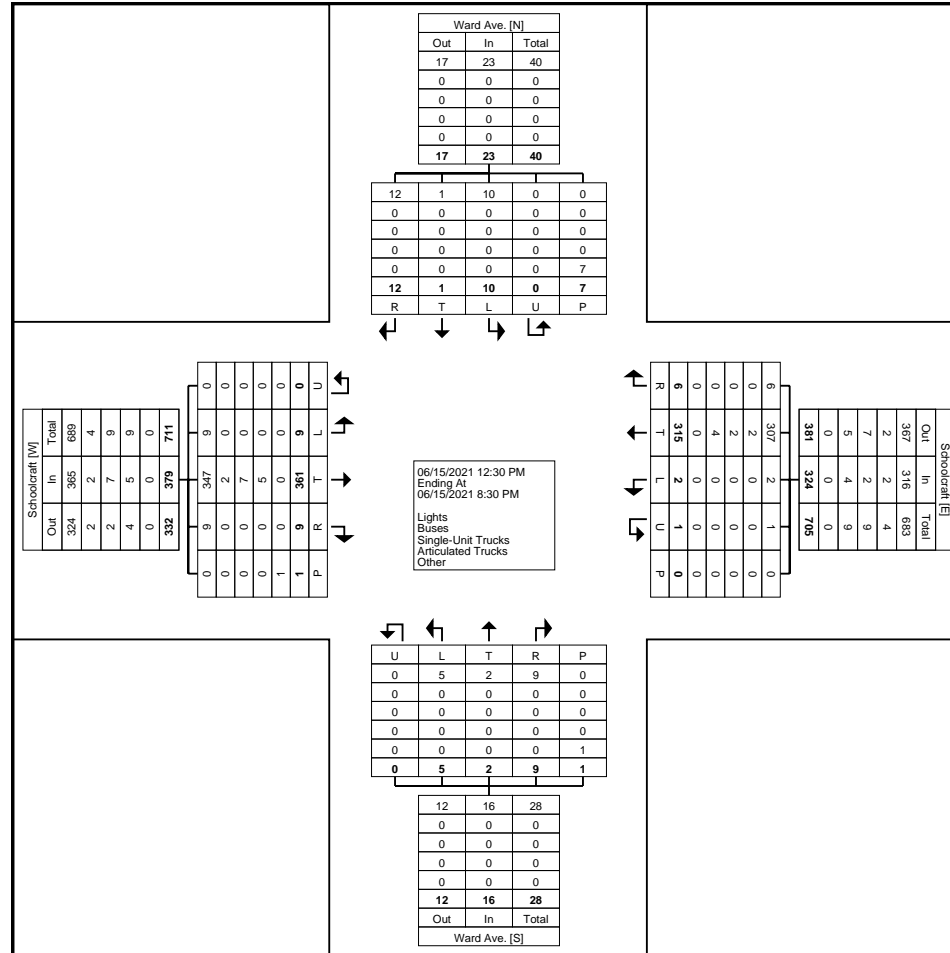
Turning Movement Data

Start Time	Ward Ave. Southbound						Schoolcraft Westbound						Ward Ave. Northbound						Schoolcraft Eastbound						Int. Total
	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	
12:30 PM	1	0	0	0	2	1	1	25	2	0	0	28	1	0	1	0	0	2	1	32	0	0	0	33	64
12:45 PM	2	1	1	0	0	4	0	44	0	0	0	44	1	0	1	0	0	2	2	42	1	0	1	45	95
Hourly Total	3	1	1	0	2	5	1	69	2	0	0	72	2	0	2	0	0	4	3	74	1	0	1	78	159
1:00 PM	1	0	2	0	1	3	1	26	0	0	0	27	1	2	2	0	0	5	1	35	0	0	0	36	71
1:15 PM	0	0	2	0	1	2	0	28	0	0	0	28	0	0	0	0	0	0	2	40	1	0	0	43	73
1:30 PM	1	0	0	0	1	1	0	39	0	0	0	39	2	0	0	0	0	2	1	41	1	0	0	43	85
1:45 PM	3	0	2	0	0	5	1	46	0	0	0	47	3	0	1	0	1	4	0	41	2	0	0	43	99
Hourly Total	5	0	6	0	3	11	2	139	0	0	0	141	6	2	3	0	1	11	4	157	4	0	0	165	328
2:00 PM	1	0	2	0	2	3	1	45	0	1	0	47	0	0	0	0	0	0	1	40	2	0	0	43	93
2:15 PM	1	0	1	0	0	2	2	28	0	0	0	30	1	0	0	0	0	1	0	44	0	0	0	44	77
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	2	0	3	0	2	5	3	73	0	1	0	77	1	0	0	0	0	1	1	84	2	0	0	87	170
8:00 PM	2	0	0	0	0	2	0	34	0	0	0	34	0	0	0	0	0	0	1	46	2	0	0	49	85
8:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	12	1	10	0	7	23	6	315	2	1	0	324	9	2	5	0	1	16	9	361	9	0	1	379	742
Approach %	52.2	4.3	43.5	0.0	-	-	1.9	97.2	0.6	0.3	-	-	56.3	12.5	31.3	0.0	-	-	2.4	95.3	2.4	0.0	-	-	-
Total %	1.6	0.1	1.3	0.0	-	3.1	0.8	42.5	0.3	0.1	-	43.7	1.2	0.3	0.7	0.0	-	2.2	1.2	48.7	1.2	0.0	-	51.1	-
Lights	12	1	10	0	-	23	6	307	2	1	-	316	9	2	5	0	-	16	9	347	9	0	-	365	720
% Lights	100.0	100.0	100.0	-	-	100.0	100.0	97.5	100.0	100.0	-	97.5	100.0	100.0	100.0	-	-	100.0	100.0	96.1	100.0	-	-	96.3	97.0
Buses	0	0	0	0	-	0	0	2	0	0	-	2	0	0	0	0	-	0	0	2	0	0	-	2	4
% Buses	0.0	0.0	0.0	-	-	0.0	0.0	0.6	0.0	0.0	-	0.6	0.0	0.0	0.0	-	-	0.0	0.0	0.6	0.0	-	-	0.5	0.5
Single-Unit Trucks	0	0	0	0	-	0	0	2	0	0	-	2	0	0	0	0	-	0	0	7	0	0	-	7	9
% Single-Unit Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.6	0.0	0.0	-	0.6	0.0	0.0	0.0	-	-	0.0	0.0	1.9	0.0	-	-	1.8	1.2
Articulated Trucks	0	0	0	0	-	0	0	4	0	0	-	4	0	0	0	0	-	0	0	5	0	0	-	5	9
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	1.3	0.0	0.0	-	1.2	0.0	0.0	0.0	-	-	0.0	0.0	1.4	0.0	-	-	1.3	1.2
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	7	-	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	-	1	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Ward Ave & Schoolcraft
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 2



Turning Movement Data Plot

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Ward Ave & Schoolcraft
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 3

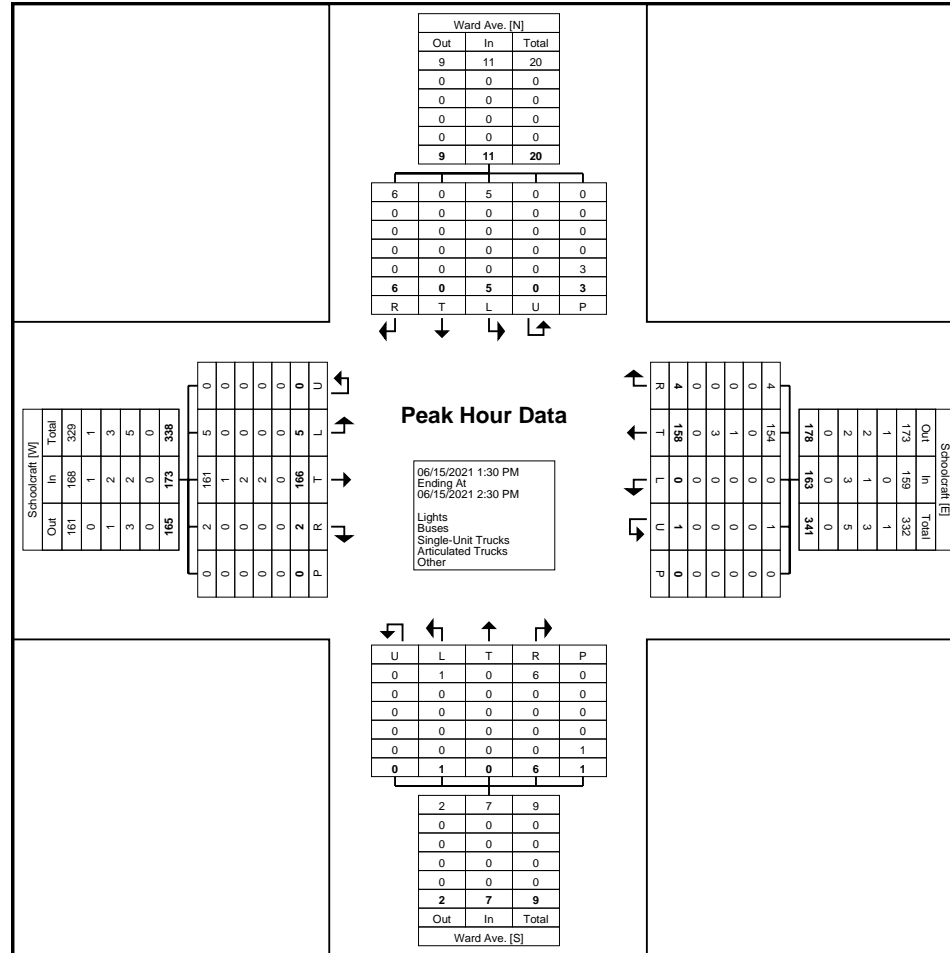
Turning Movement Peak Hour Data (1:30 PM)

Start Time	Ward Ave. Southbound						Schoolcraft Westbound						Ward Ave. Northbound						Schoolcraft Eastbound						Int. Total
	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	
1:30 PM	1	0	0	0	1	1	0	39	0	0	0	39	2	0	0	0	0	2	1	41	1	0	0	43	85
1:45 PM	3	0	2	0	0	5	1	46	0	0	0	47	3	0	1	0	1	4	0	41	2	0	0	43	99
2:00 PM	1	0	2	0	2	3	1	45	0	1	0	47	0	0	0	0	0	0	1	40	2	0	0	43	93
2:15 PM	1	0	1	0	0	2	2	28	0	0	0	30	1	0	0	0	0	1	0	44	0	0	0	44	77
Total	6	0	5	0	3	11	4	158	0	1	0	163	6	0	1	0	1	7	2	166	5	0	0	173	354
Approach %	54.5	0.0	45.5	0.0	-	-	2.5	96.9	0.0	0.6	-	-	85.7	0.0	14.3	0.0	-	-	1.2	96.0	2.9	0.0	-	-	-
Total %	1.7	0.0	1.4	0.0	-	3.1	1.1	44.6	0.0	0.3	-	46.0	1.7	0.0	0.3	0.0	-	2.0	0.6	46.9	1.4	0.0	-	48.9	-
PHF	0.500	0.000	0.625	0.000	-	0.550	0.500	0.859	0.000	0.250	-	0.867	0.500	0.000	0.250	0.000	-	0.438	0.500	0.943	0.625	0.000	-	0.983	0.894
Lights	6	0	5	0	-	11	4	154	0	1	-	159	6	0	1	0	-	7	2	161	5	0	-	168	345
% Lights	100.0	-	100.0	-	-	100.0	100.0	97.5	-	100.0	-	97.5	100.0	-	100.0	-	-	100.0	100.0	97.0	100.0	-	-	97.1	97.5
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	1	0	0	-	1	1
% Buses	0.0	-	0.0	-	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	-	0.0	0.0	0.6	0.0	-	-	0.6	0.3
Single-Unit Trucks	0	0	0	0	-	0	0	1	0	0	-	1	0	0	0	0	-	0	0	2	0	0	-	2	3
% Single-Unit Trucks	0.0	-	0.0	-	-	0.0	0.0	0.6	-	0.0	-	0.6	0.0	-	0.0	-	-	0.0	0.0	1.2	0.0	-	-	1.2	0.8
Articulated Trucks	0	0	0	0	-	0	0	3	0	0	-	3	0	0	0	0	-	0	0	2	0	0	-	2	5
% Articulated Trucks	0.0	-	0.0	-	-	0.0	0.0	1.9	-	0.0	-	1.8	0.0	-	0.0	-	-	0.0	0.0	1.2	0.0	-	-	1.2	1.4
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	-	3	-	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-

Unit Number:

DLZ
 8430 West Bryn Mawr Avenue, Suite 100
 Chicago, Illinois, United States 60631
 (773) 283-2600

Count Name: Ward Ave & Schoolcraft
 Site Code: 1942-6994-50/0800/2000
 Start Date: 06/15/2021
 Page No: 4



Turning Movement Peak Hour Data Plot (1:30 PM)

Study Name Grand River Ave & I-96 Service Rd
Start Date Tuesday, June 15, 2021 12:30 PM
End Date Tuesday, June 15, 2021 10:00 PM
Site Code 1942-6994-50/0800/2000

Report Summary

Time Period	Class.	Southwestbound						Northwestbound				Northbound				Northeastbound				Eastbound				Southeastbound				Crosswalk		
		R	T	BL	L	I	O	T	L	HL	U	I	O	I	O	I	O	I	O	R	BR	T	U	I	O	Total	NE	SE	S	SW
Peak 1	Lights	292	175	5	40	512	0	344	59	0	0	403	443	0	102	0	240	0	0	6	97	403	0	506	636	1421	NE	0	0	0
Specified Period	%	96%	95%	100%	100%	96%	0%	98%	100%	0%	0%	98%	98%	0%	94%	0%	96%	0%	0%	100%	94%	98%	0%	97%	97%	97%		0%	0%	
12:30 PM - 2:30 PM	Buses	1	0	0	0	1	0	3	0	0	0	3	5	0	0	0	0	0	0	0	0	5	0	5	4	9	SE	0	0	0
One Hour Peak	%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	1%	1%	1%		0%	0%	
1:30 PM - 2:30 PM	Single-Unit Truc	8	5	0	0	13	0	4	0	0	0	4	4	0	5	0	5	0	0	0	5	4	0	9	12	26	S	0	0	0
	%	3%	3%	0%	0%	2%	0%	1%	0%	0%	0%	1%	1%	0%	5%	0%	2%	0%	0%	0%	5%	1%	0%	2%	2%	2%		0%	0%	
	articulated Truc	2	5	0	0	7	0	0	0	0	0	0	0	0	1	0	5	0	0	0	1	0	0	1	2	8	SW	0	0	0
	%	1%	3%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	2%	0%	0%	0%	1%	0%	0%	0%	0%	1%		0%	0%	
	Total	303	185	5	40	533	0	351	59	0	0	410	452	0	108	0	250	0	0	6	103	412	0	521	654	1464	W	1	0	1
	PHF	0.8	0.84	0.62	0.62	0.79	0	0.89	0.78	0	0	0.93	0.91	0	0.75	0	0.82	0	0	0.5	0.74	0.93	0	0.93	0.9	0.89		100%	0%	
	Approach %					36%	0%					28%	31%	0%	7%	0%	17%	0%	0%					36%	45%		NW	0	0	0
																												1	0	1
Peak 2	Lights	253	185	6	23	467	0	285	46	0	0	331	402	0	101	0	234	0	0	3	95	379	0	477	538	1275	NE	2	1	3
Specified Period	%	98%	99%	100%	100%	99%	0%	99%	100%	0%	0%	99%	99%	0%	99%	0%	99%	0%	0%	100%	99%	99%	0%	99%	99%	99%		67%	33%	
8:00 PM - 10:00 PM	Buses	0	0	0	0	0	0	1	0	0	0	1	3	0	1	0	0	0	0	0	1	3	0	4	1	5	SE	0	0	0
One Hour Peak	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	1%	0%	0%	0%	0%	0%	1%	1%	0%	1%	0%	0%		0%	0%	
8:00 PM - 9:00 PM	Single-Unit Truc	2	1	0	0	3	0	1	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	3	4	S	0	0	0
	%	1%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%		0%	0%	
	articulated Truc	3	1	0	0	4	0	1	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	4	5	SW	0	0	0
	%	1%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%		0%	0%	
	Total	258	187	6	23	474	0	288	46	0	0	334	405	0	102	0	236	0	0	3	96	382	0	481	546	1289	W	1	1	2
	PHF	0.85	0.85	0.5	0.64	0.86	0	0.73	0.57	0	0	0.7	0.96	0	0.85	0	0.95	0	0	0.75	0.83	0.94	0	0.95	0.89	0.97		50%	50%	
	Approach %					37%	0%					26%	31%	0%	8%	0%	18%	0%	0%					37%	42%		NW	0	1	1
																												0%	100%	
																												3	3	6

Lights

Study Name Grand River Ave & I-96 Service Rd

Start Date 06/15/2021

Start Time 12:30 PM

Site Code 1942-6994-50/0800/2000

Unit Number

Start Time	I-96 Service Rd Southwestbound				Grand River Ave Northwestbound				Grand River Ave Southeastbound			
	Right	Thru	Bear Left	Left	Thru	Left	Hard Left	U-Turn	Right	Bear Right	Thru	U-Turn
12:30 PM	50	47	6	10	89	25	1	0	1	20	91	2
12:45 PM	67	38	1	12	55	17	0	0	3	14	103	2
1:00 PM	65	36	2	14	91	18	0	0	2	27	91	0
1:15 PM	62	42	4	3	87	12	0	0	0	21	101	1
1:30 PM	66	35	1	8	89	19	0	0	3	19	110	0
1:45 PM	63	45	1	6	97	9	0	0	0	33	102	0
2:00 PM	72	43	1	10	73	13	0	0	0	20	86	0
2:15 PM	91	52	2	16	85	18	0	0	3	25	105	0
8:00 PM	51	41	3	6	96	20	0	0	1	20	88	0
8:15 PM	68	44	1	9	69	13	0	0	1	29	89	0
8:30 PM	74	53	2	4	61	6	0	0	1	24	101	0
8:45 PM	60	47	0	4	59	7	0	0	0	22	101	0
9:00 PM	72	41	5	8	75	15	0	0	2	18	69	0
9:15 PM	66	50	5	5	79	11	0	0	0	20	62	0
9:30 PM	45	45	2	8	53	13	0	0	1	18	53	0
9:45 PM	69	25	3	3	56	8	0	0	1	20	67	0

Totals

Study Name Grand River Ave & I-96 Service Rd

Start Date 06/15/2021

Start Time 12:30 PM

Site Code 1942-6994-50/0800/2000

Unit Number

Start Time	I-96 Service Rd Southwestbound				Grand River Ave Northwestbound				Grand River Ave Southeastbound			
	Right	Thru	Bear Left	Left	Thru	Left	Hard Left	U-Turn	Right	Bear Right	Thru	U-Turn
12:30 PM	53	49	6	10	90	27	1	0	1	20	93	2
12:45 PM	69	45	1	13	59	18	0	0	3	18	105	2
1:00 PM	70	36	2	14	97	18	0	0	2	28	95	0
1:15 PM	65	47	4	3	87	13	0	0	0	21	103	1
1:30 PM	67	39	1	8	91	19	0	0	3	21	111	0
1:45 PM	66	46	1	6	99	9	0	0	0	35	105	0
2:00 PM	75	45	1	10	74	13	0	0	0	21	88	0
2:15 PM	95	55	2	16	87	18	0	0	3	26	108	0
8:00 PM	54	41	3	6	99	20	0	0	1	20	88	0
8:15 PM	68	44	1	9	69	13	0	0	1	29	91	0
8:30 PM	76	55	2	4	61	6	0	0	1	24	102	0
8:45 PM	60	47	0	4	59	7	0	0	0	23	101	0
9:00 PM	73	41	5	8	75	15	0	0	2	18	71	0
9:15 PM	66	50	5	5	79	11	0	0	0	20	62	0
9:30 PM	46	45	2	8	55	13	0	0	1	18	53	0
9:45 PM	69	25	3	3	56	8	0	0	1	20	67	0



INNOVATIVE IDEAS
EXCEPTIONAL DESIGN
UNMATCHED CLIENT SERVICE

APPENDIX B
SYNCHRO REPORTS



INNOVATIVE IDEAS
EXCEPTIONAL DESIGN
UNMATCHED CLIENT SERVICE

EXISTING CONDITIONS



Lanes, Volumes, Timings
1011: Schaefer Hwy & Grand River Ave

11/01/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↕↕↕		↖	↕↕↕		↖	↕	↗	↖	↕↕	
Traffic Volume (vph)	57	442	96	26	495	131	76	320	10	65	339	29
Future Volume (vph)	57	442	96	26	495	131	76	320	10	65	339	29
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	115		0	95		0	155		0	80		240
Storage Lanes	1		0	0		0	1		1	1		1
Taper Length (ft)	65			65			25			65		
Lane Util. Factor	1.00	0.91	0.91	1.00	0.91	0.91	1.00	1.00	1.00	1.00	0.95	0.95
Ped Bike Factor	1.00	1.00		1.00	0.99		1.00		0.99	1.00	1.00	
Frt		0.971			0.964				0.850		0.988	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	4943	0	1805	4835	0	1805	1845	1615	1583	3369	0
Flt Permitted	0.376			0.416			0.464			0.296		
Satd. Flow (perm)	712	4943	0	789	4835	0	881	1845	1593	493	3369	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		100			141				34		11	
Link Speed (mph)		35			35			30			30	
Link Distance (ft)		932			427			332			1209	
Travel Time (s)		18.2			8.3			7.5			27.5	
Confl. Peds. (#/hr)	5		3	3		5	3		2	2		3
Peak Hour Factor	0.71	0.92	0.83	0.78	0.94	0.78	0.83	0.89	0.63	0.81	0.93	0.91
Heavy Vehicles (%)	0%	2%	0%	0%	2%	5%	0%	3%	0%	14%	6%	3%
Adj. Flow (vph)	80	480	116	33	527	168	92	360	16	80	365	32
Shared Lane Traffic (%)												
Lane Group Flow (vph)	80	596	0	33	695	0	92	360	16	80	397	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Perm	NA		Perm	NA		Perm	NA	Perm	Perm	NA	
Protected Phases		6			2			8				4
Permitted Phases	6			2			8		8	4		
Detector Phase	6	6		2	2		8	8	8	4	4	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		7.0	7.0	7.0	7.0	7.0	
Minimum Split (s)	35.8	35.8		35.8	35.8		46.5	46.5	46.5	46.5	46.5	
Total Split (s)	55.0	55.0		55.0	55.0		35.0	35.0	35.0	35.0	35.0	
Total Split (%)	61.1%	61.1%		61.1%	61.1%		38.9%	38.9%	38.9%	38.9%	38.9%	
Maximum Green (s)	49.2	49.2		49.2	49.2		28.5	28.5	28.5	28.5	28.5	
Yellow Time (s)	3.6	3.6		3.6	3.6		3.2	3.2	3.2	3.2	3.2	
All-Red Time (s)	2.2	2.2		2.2	2.2		3.3	3.3	3.3	3.3	3.3	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.8	5.8		5.8	5.8		6.5	6.5	6.5	6.5	6.5	
Lead/Lag												
Lead-Lag Optimize?												

Lanes, Volumes, Timings
 1011: Schaefer Hwy & Grand River Ave

11/01/2021

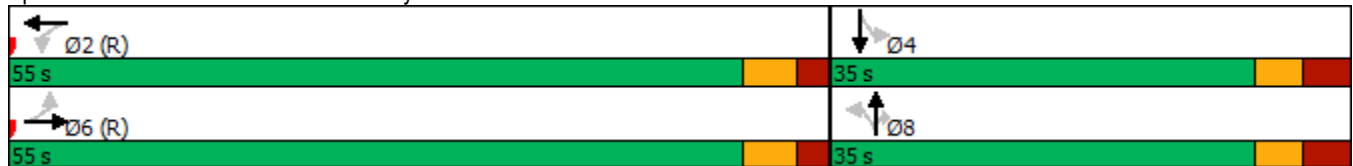


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None	None	None	None	
Walk Time (s)	7.0	7.0		7.0	7.0		10.0	10.0	10.0	10.0	10.0	
Flash Dont Walk (s)	23.0	23.0		23.0	23.0		30.0	30.0	30.0	30.0	30.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0	0	0	0	
Act Effct Green (s)	55.1	55.1		55.1	55.1		22.6	22.6	22.6	22.6	22.6	
Actuated g/C Ratio	0.61	0.61		0.61	0.61		0.25	0.25	0.25	0.25	0.25	
v/c Ratio	0.18	0.19		0.07	0.23		0.42	0.78	0.04	0.65	0.47	
Control Delay	10.5	7.1		8.7	6.5		26.2	36.8	6.9	53.5	28.8	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.5	0.0	0.0	0.0	
Total Delay	10.5	7.1		8.7	6.5		26.2	37.4	6.9	53.5	28.8	
LOS	B	A		A	A		C	D	A	D	C	
Approach Delay		7.5			6.6			34.1				32.9
Approach LOS		A			A			C				C
Queue Length 50th (ft)	18	41		6	40		47	199	0	41	97	
Queue Length 95th (ft)	36	67		19	68		81	275	6	75	128	
Internal Link Dist (ft)		852			347			252				1129
Turn Bay Length (ft)	115			95			155			80		
Base Capacity (vph)	436	3066		483	3015		278	584	527	156	1074	
Starvation Cap Reductn	0	0		0	0		0	46	0	0	0	
Spillback Cap Reductn	0	0		0	0		0	0	0	0	0	
Storage Cap Reductn	0	0		0	0		0	0	0	0	0	
Reduced v/c Ratio	0.18	0.19		0.07	0.23		0.33	0.67	0.03	0.51	0.37	

Intersection Summary

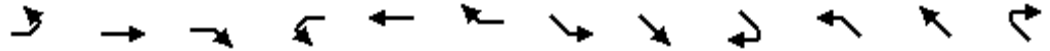
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 59 (66%), Referenced to phase 2:WBTL and 6:EBTL, Start of Green
 Natural Cycle: 85
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.78
 Intersection Signal Delay: 17.7
 Intersection LOS: B
 Intersection Capacity Utilization 77.6%
 ICU Level of Service D
 Analysis Period (min) 15

Splits and Phases: 1011: Schaefer Hwy & Grand River Ave



Lanes, Volumes, Timings
 1093: Grand River Ave & Jeffries Service Dr

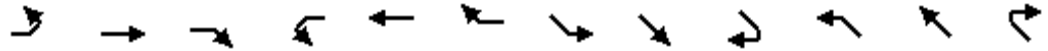
11/01/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↑↑↑						↑↑↑			↑↑↑	
Traffic Volume (vph)	6	299	56	0	0	0	5	446	0	0	403	19
Future Volume (vph)	6	299	56	0	0	0	5	446	0	0	403	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.86	0.86	0.86	1.00	1.00	1.00	0.91	0.91	1.00	1.00	0.91	0.91
Ped Bike Factor								1.00			1.00	
Frt		0.977									0.990	
Flt Protected		0.999						0.999				
Satd. Flow (prot)	0	6254	0	0	0	0	0	5082	0	0	5066	0
Flt Permitted		0.999						0.933				
Satd. Flow (perm)	0	6254	0	0	0	0	0	4746	0	0	5066	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		55									19	
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		329			379			582			812	
Travel Time (s)		6.4			7.4			11.3			15.8	
Confl. Peds. (#/hr)							1					1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.60	0.91	0.92	0.92	0.92	0.60
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	0%	2%	2%	2%	1%	5%
Adj. Flow (vph)	7	325	61	0	0	0	8	490	0	0	438	32
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	393	0	0	0	0	0	498	0	0	470	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Perm	NA					Perm	NA			NA	
Protected Phases		4						6			2	
Permitted Phases	4						6					
Detector Phase	4	4					6	6			2	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0			10.0	
Minimum Split (s)	57.4	57.4					37.0	37.0			37.0	
Total Split (s)	37.0	37.0					53.0	53.0			53.0	
Total Split (%)	41.1%	41.1%					58.9%	58.9%			58.9%	
Maximum Green (s)	29.6	29.6					47.0	47.0			47.0	
Yellow Time (s)	3.0	3.0					3.6	3.6			3.6	
All-Red Time (s)	4.4	4.4					2.4	2.4			2.4	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		7.4						6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0			3.0	
Recall Mode	None	None					C-Max	C-Max			C-Max	
Walk Time (s)	9.0	9.0					7.0	7.0			7.0	

Lanes, Volumes, Timings
 1093: Grand River Ave & Jeffries Service Dr

11/01/2021

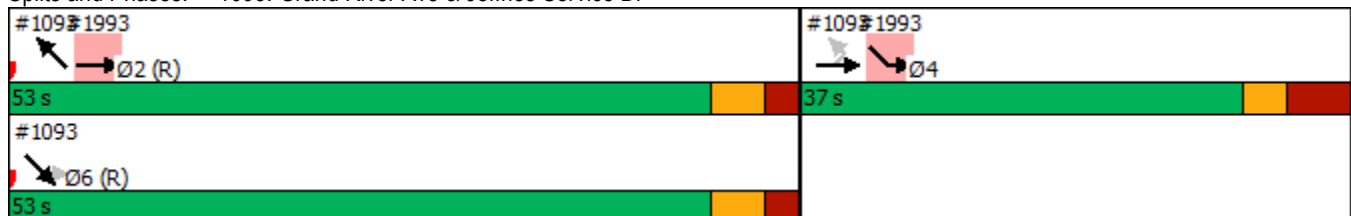


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Flash Dont Walk (s)	33.0	33.0					24.0	24.0			24.0	
Pedestrian Calls (#/hr)	0	0					0	0			0	
Act Effct Green (s)		11.5						65.1			65.1	
Actuated g/C Ratio		0.13						0.72			0.72	
v/c Ratio		0.46						0.15			0.13	
Control Delay		31.4						2.3			3.9	
Queue Delay		0.0						0.0			0.0	
Total Delay		31.4						2.3			3.9	
LOS		C						A			A	
Approach Delay		31.4						2.3			3.9	
Approach LOS		C						A			A	
Queue Length 50th (ft)		59						11			23	
Queue Length 95th (ft)		86						15			38	
Internal Link Dist (ft)		249			299			502			732	
Turn Bay Length (ft)												
Base Capacity (vph)		2093						3431			3667	
Starvation Cap Reductn		0						0			0	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.19						0.15			0.13	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 54 (60%), Referenced to phase 2:NWT and 6:SETL, Start of Green
 Natural Cycle: 95
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.46
 Intersection Signal Delay: 11.2
 Intersection Capacity Utilization 45.3%
 Analysis Period (min) 15
 Intersection LOS: B
 ICU Level of Service A

Splits and Phases: 1093: Grand River Ave & Jeffries Service Dr



Lanes, Volumes, Timings

1193: Left Turn Bridge & Grand River Ave & I-96 WB Service Rd/I-96 WB Service Dr 11/01/2021



Lane Group	WBL2	WBL	WBT	WBR	SET	SER	SER2	NWL	NWT
Lane Configurations			↑↑↑	↑	↑↑↑	↑		↑	↑↑↑
Traffic Volume (vph)	40	5	185	303	412	103	6	59	351
Future Volume (vph)	40	5	185	303	412	103	6	59	351
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0		0		140		260	
Storage Lanes		0		1		1		1	
Taper Length (ft)		65						65	
Lane Util. Factor	0.91	0.91	0.86	0.86	0.86	0.86	0.91	1.00	0.91
Frt			0.940	0.850	0.994	0.850			
Flt Protected			0.994					0.950	
Satd. Flow (prot)	0	0	4490	1362	4770	1316	0	1805	5085
Flt Permitted			0.994					0.950	
Satd. Flow (perm)	0	0	4490	1362	4770	1316	0	1805	5085
Right Turn on Red				Yes			Yes		
Satd. Flow (RTOR)			165	164		126			
Link Speed (mph)			35		35				35
Link Distance (ft)			675		427				582
Travel Time (s)			13.1		8.3				11.3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.93	0.74	0.60	0.78	0.89
Heavy Vehicles (%)	2%	2%	2%	2%	2%	6%	0%	0%	2%
Adj. Flow (vph)	43	5	201	329	443	139	10	76	394
Shared Lane Traffic (%)				50%		13%			
Lane Group Flow (vph)	0	0	414	164	461	131	0	76	394
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right	Right	Left	Left
Median Width(ft)			0		12				12
Link Offset(ft)			0		0				0
Crosswalk Width(ft)			16		16				16
Two way Left Turn Lane									
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	15		9		9	9	15	
Turn Type	Perm	Perm	NA	Perm	NA	Perm		Prot	NA
Protected Phases			4		6			5	2
Permitted Phases	4	4		4		6			
Detector Phase	4	4	4	4	6	6		5	2
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0		7.0	10.0
Minimum Split (s)	55.5	55.5	55.5	55.5	37.9	37.9		12.9	37.9
Total Split (s)	37.0	37.0	37.0	37.0	40.0	40.0		13.0	53.0
Total Split (%)	41.1%	41.1%	41.1%	41.1%	44.4%	44.4%		14.4%	58.9%
Maximum Green (s)	29.5	29.5	29.5	29.5	34.1	34.1		7.1	47.1
Yellow Time (s)	3.0	3.0	3.0	3.0	3.6	3.6		3.6	3.6
All-Red Time (s)	4.5	4.5	4.5	4.5	2.3	2.3		2.3	2.3
Lost Time Adjust (s)			0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)			7.5	7.5	5.9	5.9		5.9	5.9
Lead/Lag					Lead	Lead		Lag	
Lead-Lag Optimize?					Yes	Yes		Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max		None	C-Max

Lanes, Volumes, Timings

1193: Left Turn Bridge & Grand River Ave & I-96 WB Service Rd/I-96 WB Service Dr 11/01/2021

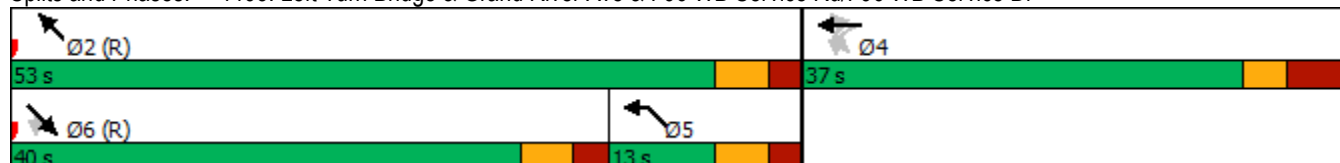


Lane Group	WBL2	WBL	WBT	WBR	SET	SER	SER2	NWL	NWT
Walk Time (s)	9.0	9.0	9.0	9.0	7.0	7.0			7.0
Flash Dont Walk (s)	39.0	39.0	39.0	39.0	25.0	25.0			25.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0			0
Act Effct Green (s)			12.4	12.4	53.8	53.8		7.1	64.2
Actuated g/C Ratio			0.14	0.14	0.60	0.60		0.08	0.71
v/c Ratio			0.55	0.50	0.16	0.16		0.54	0.11
Control Delay			23.8	11.2	7.0	1.1		52.2	1.2
Queue Delay			0.0	0.0	0.0	0.0		0.0	0.0
Total Delay			23.8	11.2	7.0	1.1		52.2	1.2
LOS			C	B	A	A		D	A
Approach Delay			20.3		5.7				9.5
Approach LOS			C		A				A
Queue Length 50th (ft)			51	0	35	1		43	2
Queue Length 95th (ft)			79	60	44	3		76	4
Internal Link Dist (ft)			595		347				502
Turn Bay Length (ft)						140		260	
Base Capacity (vph)			1582	556	2852	837		142	3628
Starvation Cap Reductn			0	0	0	0		0	0
Spillback Cap Reductn			0	0	0	0		0	0
Storage Cap Reductn			0	0	0	0		0	0
Reduced v/c Ratio			0.26	0.29	0.16	0.16		0.54	0.11

Intersection Summary

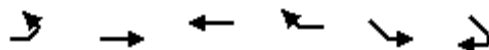
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 66 (73%), Referenced to phase 2:NWT and 6:SET, Start of Green
 Natural Cycle: 110
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.55
 Intersection Signal Delay: 11.9
 Intersection Capacity Utilization 39.0%
 Analysis Period (min) 15
 Intersection LOS: B
 ICU Level of Service A

Splits and Phases: 1193: Left Turn Bridge & Grand River Ave & I-96 WB Service Rd/I-96 WB Service Dr



Lanes, Volumes, Timings
 1993: Jeffries Service Dr & Left Turn Bridge

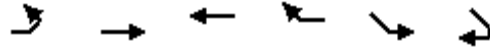
11/01/2021



Lane Group	EBL	EBT	WBT	WBR	SEL	SER	Ø6
Lane Configurations		↑↑↑			↑↑		
Traffic Volume (vph)	0	253	0	0	108	0	
Future Volume (vph)	0	253	0	0	108	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (ft)	0			0	50	0	
Storage Lanes	0			0	0	0	
Taper Length (ft)	65				65		
Lane Util. Factor	1.00	0.91	1.00	1.00	0.97	1.00	
Frt							
Flt Protected					0.950		
Satd. Flow (prot)	0	5085	0	0	3433	0	
Flt Permitted					0.950		
Satd. Flow (perm)	0	5085	0	0	3433	0	
Right Turn on Red				Yes	Yes	Yes	
Satd. Flow (RTOR)					1077		
Link Speed (mph)		35	35		30		
Link Distance (ft)		184	329		419		
Travel Time (s)		3.6	6.4		9.5		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	275	0	0	117	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	275	0	0	117	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		0	0		24		
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		16	16		16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15			9	15	9	
Turn Type		NA			Prot		
Protected Phases		2			4		6
Permitted Phases							
Detector Phase		2			4		
Switch Phase							
Minimum Initial (s)		10.0			10.0		10.0
Minimum Split (s)		37.0			57.4		37.0
Total Split (s)		53.0			37.0		53.0
Total Split (%)		58.9%			41.1%		59%
Maximum Green (s)		47.0			29.6		47.0
Yellow Time (s)		3.6			3.0		3.6
All-Red Time (s)		2.4			4.4		2.4
Lost Time Adjust (s)		0.0			0.0		
Total Lost Time (s)		6.0			7.4		
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)		3.0			3.0		3.0
Recall Mode		C-Max			None		C-Max
Walk Time (s)		7.0			9.0		7.0

Lanes, Volumes, Timings
 1993: Jeffries Service Dr & Left Turn Bridge

11/01/2021

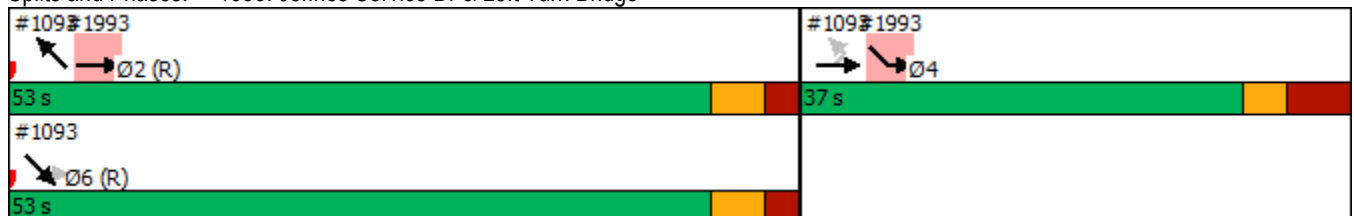


Lane Group	EBL	EBT	WBT	WBR	SEL	SER	Ø6
Flash Dont Walk (s)		24.0			33.0		24.0
Pedestrian Calls (#/hr)		0			0		0
Act Effct Green (s)		65.1			11.5		
Actuated g/C Ratio		0.72			0.13		
v/c Ratio		0.07			0.08		
Control Delay		3.2			0.1		
Queue Delay		0.0			0.0		
Total Delay		3.2			0.1		
LOS		A			A		
Approach Delay		3.2			0.1		
Approach LOS		A			A		
Queue Length 50th (ft)		8			0		
Queue Length 95th (ft)		23			0		
Internal Link Dist (ft)		104	249		339		
Turn Bay Length (ft)					50		
Base Capacity (vph)		3675			1851		
Starvation Cap Reductn		0			0		
Spillback Cap Reductn		0			0		
Storage Cap Reductn		0			0		
Reduced v/c Ratio		0.07			0.06		

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 54 (60%), Referenced to phase 2:NWT and 6:SETL, Start of Green
 Natural Cycle: 95
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.46
 Intersection Signal Delay: 2.3
 Intersection LOS: A
 Intersection Capacity Utilization 27.8%
 ICU Level of Service A
 Analysis Period (min) 15

Splits and Phases: 1993: Jeffries Service Dr & Left Turn Bridge



Lanes, Volumes, Timings
2609: Schaefer Hwy & Schoolcraft Rd

11/01/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	38	81	13	47	77	41	16	437	55	29	373	30
Future Volume (vph)	38	81	13	47	77	41	16	437	55	29	373	30
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	60		0	60		0	80		0	80		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor								1.00		1.00		
Fr _t		0.971			0.951			0.981				0.985
Fl _t Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1830	0	1656	1795	0	1805	1799	0	1752	1760	0
Fl _t Permitted	0.656			0.684			0.373			0.295		
Satd. Flow (perm)	1246	1830	0	1192	1795	0	709	1799	0	543	1760	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		14			29			12				8
Link Speed (mph)		30			30			30				30
Link Distance (ft)		699			1470			1209				234
Travel Time (s)		15.9			33.4			27.5				5.3
Confl. Peds. (#/hr)									4	4		
Peak Hour Factor	0.63	0.88	0.60	0.84	0.71	0.79	0.60	0.94	0.81	0.66	0.91	0.68
Heavy Vehicles (%)	0%	1%	0%	9%	1%	0%	0%	3%	5%	3%	7%	0%
Adj. Flow (vph)	60	92	22	56	108	52	27	465	68	44	410	44
Shared Lane Traffic (%)												
Lane Group Flow (vph)	60	114	0	56	160	0	27	533	0	44	454	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt		NA
Protected Phases		8			4		1	6		5		2
Permitted Phases	8			4			6			2		
Detector Phase	8	8		4	4		1	6		5		2
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0		5.0	9.0		5.0		9.0
Minimum Split (s)	28.8	28.8		28.8	28.8		10.6	26.6		10.6		26.6
Total Split (s)	27.0	27.0		27.0	27.0		12.0	41.0		12.0		41.0
Total Split (%)	33.8%	33.8%		33.8%	33.8%		15.0%	51.3%		15.0%		51.3%
Maximum Green (s)	20.2	20.2		20.2	20.2		6.4	34.4		6.4		34.4
Yellow Time (s)	3.8	3.8		3.8	3.8		3.5	3.6		3.5		3.6
All-Red Time (s)	3.0	3.0		3.0	3.0		2.1	3.0		2.1		3.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0		0.0
Total Lost Time (s)	6.8	6.8		6.8	6.8		5.6	6.6		5.6		6.6
Lead/Lag							Lead	Lag		Lead		Lag
Lead-Lag Optimize?							Yes	Yes		Yes		Yes

Lanes, Volumes, Timings
 2609: Schaefer Hwy & Schoolcraft Rd

11/01/2021

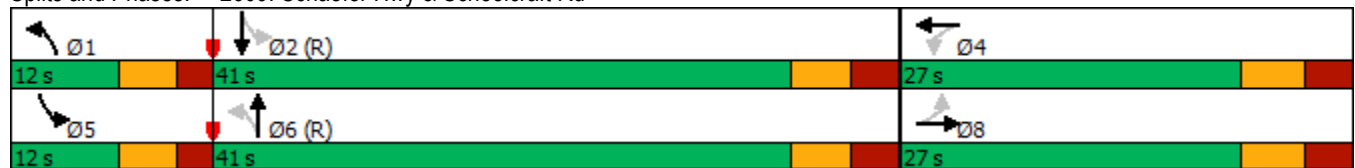


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	Max	Max		Max	Max		Max	C-Max		Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0			13.0			13.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	20.2	20.2		20.2	20.2		41.8	34.4		41.8	34.4	
Actuated g/C Ratio	0.25	0.25		0.25	0.25		0.52	0.43		0.52	0.43	
v/c Ratio	0.19	0.24		0.19	0.34		0.06	0.68		0.12	0.60	
Control Delay	25.5	22.4		25.5	22.2		7.3	23.5		6.4	22.6	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	25.5	22.4		25.5	22.2		7.3	23.5		6.4	22.6	
LOS	C	C		C	C		A	C		A	C	
Approach Delay		23.5			23.1			22.7			21.2	
Approach LOS		C			C			C			C	
Queue Length 50th (ft)	24	39		22	53		5	203		3	191	
Queue Length 95th (ft)	37	79		49	77		10	316		7	303	
Internal Link Dist (ft)		619			1390			1129			154	
Turn Bay Length (ft)	60			60			80			80		
Base Capacity (vph)	314	472		300	474		458	780		380	761	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.19	0.24		0.19	0.34		0.06	0.68		0.12	0.60	

Intersection Summary

Area Type: Other
 Cycle Length: 80
 Actuated Cycle Length: 80
 Offset: 0 (0%), Referenced to phase 2:SBTL and 6:NBTL, Start of Green
 Natural Cycle: 70
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.68
 Intersection Signal Delay: 22.3
 Intersection LOS: C
 Intersection Capacity Utilization 46.8%
 ICU Level of Service A
 Analysis Period (min) 15

Splits and Phases: 2609: Schaefer Hwy & Schoolcraft Rd



Lanes, Volumes, Timings
3011: Schaefer Hwy & I-96 WB Service Rd

11/01/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	0	0	123	124	3	184	403	0	0	261	200
Future Volume (vph)	0	0	0	123	124	3	184	403	0	0	261	200
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Lane Width (ft)	12	12	12	12	11	12	11	11	12	12	11	12
Storage Length (ft)	0		0	0		0	250		0	0		0
Storage Lanes	0		0	1		0	1		0	0		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95
Ped Bike Factor							1.00				0.99	
Frt					0.997						0.934	
Flt Protected				0.950			0.950					
Satd. Flow (prot)	0	0	0	1863	3591	0	1766	3637	0	0	3255	0
Flt Permitted				0.950			0.462					
Satd. Flow (perm)	0	0	0	1863	3591	0	856	3637	0	0	3255	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					2						215	
Link Speed (mph)		25			25			25			35	
Link Distance (ft)		298			364			384			332	
Travel Time (s)		8.1			9.9			10.5			6.5	
Confl. Peds. (#/hr)							4					4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.83	0.91	0.92	0.92	0.95	0.93
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	4%	1%	2%	2%	5%	4%
Adj. Flow (vph)	0	0	0	134	135	3	222	443	0	0	275	215
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	134	138	0	222	443	0	0	490	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	0.94	0.94	0.94	0.94	0.98	0.94	0.98	0.98	0.94	0.94	0.98	0.94
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type				Perm	NA		pm+pt	NA			NA	
Protected Phases					8		5	2			6	
Permitted Phases				8			2					
Detector Phase				8	8		5	2			6	
Switch Phase												
Minimum Initial (s)				10.0	10.0		7.0	10.0			10.0	
Minimum Split (s)				28.7	28.7		12.4	23.4			23.4	
Total Split (s)				31.0	31.0		14.0	59.0			45.0	
Total Split (%)				34.4%	34.4%		15.6%	65.6%			50.0%	
Maximum Green (s)				25.3	25.3		8.6	53.6			39.6	
Yellow Time (s)				3.0	3.0		3.2	3.2			3.2	
All-Red Time (s)				2.7	2.7		2.2	2.2			2.2	
Lost Time Adjust (s)				0.0	0.0		0.0	0.0			0.0	
Total Lost Time (s)				5.7	5.7		5.4	5.4			5.4	
Lead/Lag							Lag				Lead	

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
Minimum Split (s)	30.5
Total Split (s)	31.0
Total Split (%)	34%
Maximum Green (s)	24.5
Yellow Time (s)	5.0
All-Red Time (s)	1.5
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	

Lanes, Volumes, Timings
 3011: Schaefer Hwy & I-96 WB Service Rd

11/01/2021

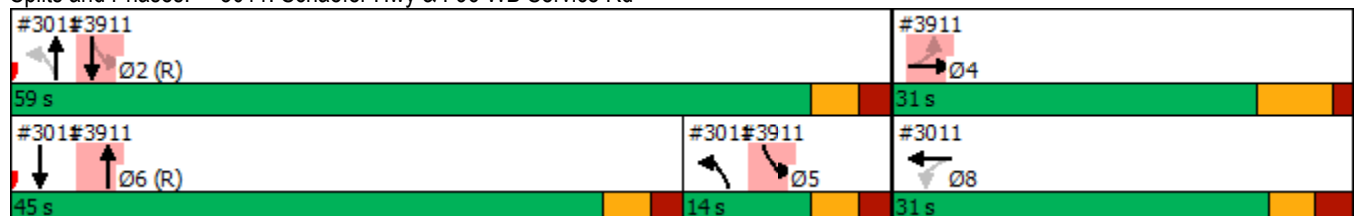


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lead-Lag Optimize?							Yes					Yes
Vehicle Extension (s)				3.0	3.0		3.0	3.0				3.0
Recall Mode				None	None		None	C-Max				C-Max
Walk Time (s)				7.0	7.0			7.0				7.0
Flash Dont Walk (s)				16.0	16.0			11.0				11.0
Pedestrian Calls (#/hr)				0	0			0				0
Act Effct Green (s)				14.9	14.9		64.0	64.0				50.0
Actuated g/C Ratio				0.17	0.17		0.71	0.71				0.56
v/c Ratio				0.44	0.23		0.32	0.17				0.26
Control Delay				30.9	25.3		4.6	2.6				10.1
Queue Delay				0.0	0.0		0.0	0.0				0.0
Total Delay				30.9	25.3		4.6	2.6				10.1
LOS				C	C		A	A				B
Approach Delay					28.1			3.3				10.1
Approach LOS					C			A				B
Queue Length 50th (ft)				80	42		30	31				40
Queue Length 95th (ft)				138	72		52	52				94
Internal Link Dist (ft)		218			284			304				252
Turn Bay Length (ft)							250					
Base Capacity (vph)				523	1010		695	2587				1905
Starvation Cap Reductn				0	0		0	0				0
Spillback Cap Reductn				0	0		0	113				0
Storage Cap Reductn				0	0		0	0				0
Reduced v/c Ratio				0.26	0.14		0.32	0.18				0.26

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	87 (97%), Referenced to phase 2:NBTL and 6:SBT, Start of Green
Natural Cycle:	70
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.65
Intersection Signal Delay:	10.3
Intersection LOS:	B
Intersection Capacity Utilization:	46.8%
ICU Level of Service:	A
Analysis Period (min):	15

Splits and Phases: 3011: Schaefer Hwy & I-96 WB Service Rd



Lane Group	Ø4
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	17.0
Pedestrian Calls (#/hr)	0
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

Lanes, Volumes, Timings
3911: Schaefer Hwy & Jeffries Service Dr

11/01/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔↑↔						↔↑↔		↔	↔↑↔	
Traffic Volume (vph)	204	106	235	0	0	0	0	383	96	51	333	0
Future Volume (vph)	204	106	235	0	0	0	0	383	96	51	333	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	0		0	90		0
Storage Lanes	0		0	0		0	0		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	0.91	0.91	0.91	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00
Ped Bike Factor								1.00		1.00		
Frt		0.935						0.969				
Flt Protected		0.982								0.950		
Satd. Flow (prot)	0	4669	0	0	0	0	0	3334	0	1671	3471	0
Flt Permitted		0.982								0.414		
Satd. Flow (perm)	0	4669	0	0	0	0	0	3334	0	726	3471	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		209						47				
Link Speed (mph)		35			35			30				30
Link Distance (ft)		417			372			291				384
Travel Time (s)		8.1			7.2			6.6				8.7
Confl. Peds. (#/hr)									6	6		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.84	0.80	0.80	0.92	0.92
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	10%	8%	4%	2%
Adj. Flow (vph)	222	115	255	0	0	0	0	456	120	64	362	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	592	0	0	0	0	0	576	0	64	362	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Perm	NA						NA		pm+pt	NA	
Protected Phases		4						6		5	2	
Permitted Phases	4									2		
Detector Phase	4	4						6		5	2	
Switch Phase												
Minimum Initial (s)	10.0	10.0						10.0		7.0	10.0	
Minimum Split (s)	30.5	30.5						23.4		12.4	23.4	
Total Split (s)	31.0	31.0						45.0		14.0	59.0	
Total Split (%)	34.4%	34.4%						50.0%		15.6%	65.6%	
Maximum Green (s)	24.5	24.5						39.6		8.6	53.6	
Yellow Time (s)	5.0	5.0						3.2		3.2	3.2	
All-Red Time (s)	1.5	1.5						2.2		2.2	2.2	
Lost Time Adjust (s)		0.0						0.0		0.0	0.0	
Total Lost Time (s)		6.5						5.4		5.4	5.4	
Lead/Lag								Lead		Lag		
Lead-Lag Optimize?								Yes		Yes		

Lane Group	Ø8
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Turn Type	
Protected Phases	8
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
Minimum Split (s)	28.7
Total Split (s)	31.0
Total Split (%)	34%
Maximum Green (s)	25.3
Yellow Time (s)	3.0
All-Red Time (s)	2.7
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	

Lanes, Volumes, Timings
3911: Schaefer Hwy & Jeffries Service Dr

11/01/2021

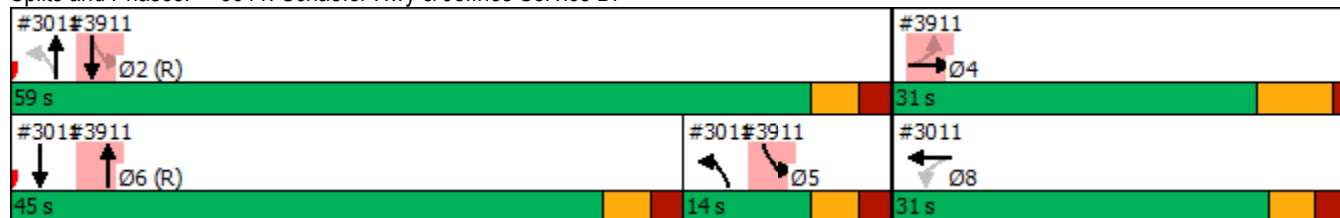


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vehicle Extension (s)	3.0	3.0						3.0		3.0	3.0	
Recall Mode	None	None						C-Max		None	C-Max	
Walk Time (s)	7.0	7.0						7.0			7.0	
Flash Dont Walk (s)	17.0	17.0						11.0			11.0	
Pedestrian Calls (#/hr)	0	0						0			0	
Act Effct Green (s)		14.1						50.0		64.0	64.0	
Actuated g/C Ratio		0.16						0.56		0.71	0.71	
v/c Ratio		0.65						0.31		0.11	0.15	
Control Delay		26.0						10.7		5.6	5.0	
Queue Delay		0.0						0.0		0.0	0.0	
Total Delay		26.0						10.7		5.6	5.0	
LOS		C						B		A	A	
Approach Delay		26.0						10.7			5.1	
Approach LOS		C						B			A	
Queue Length 50th (ft)		74						77		15	46	
Queue Length 95th (ft)		105						113		29	65	
Internal Link Dist (ft)		337			292			211			304	
Turn Bay Length (ft)										90		
Base Capacity (vph)		1423						1874		607	2469	
Starvation Cap Reductn		0						0		0	0	
Spillback Cap Reductn		0						0		0	0	
Storage Cap Reductn		0						0		0	0	
Reduced v/c Ratio		0.42						0.31		0.11	0.15	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	87 (97%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Natural Cycle:	70
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.65
Intersection Signal Delay:	14.9
Intersection LOS:	B
Intersection Capacity Utilization:	46.8%
ICU Level of Service:	A
Analysis Period (min):	15

Splits and Phases: 3911: Schaefer Hwy & Jeffries Service Dr



Lane Group	Ø8
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	16.0
Pedestrian Calls (#/hr)	0
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

Lanes, Volumes, Timings
4001: Schaefer Hwy & Lyndon St

11/01/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	22	115	27	54	78	26	32	437	47	45	351	34
Future Volume (vph)	22	115	27	54	78	26	32	437	47	45	351	34
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	80		0	80		0	80		0	80		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor		1.00		1.00			0.99	1.00		1.00	1.00	
Frt		0.971			0.963			0.983				0.983
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1783	0	1656	1812	0	1752	3506	0	1770	3433	0
Flt Permitted	0.647			0.538			0.491			0.429		
Satd. Flow (perm)	1229	1783	0	936	1812	0	900	3506	0	796	3433	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		14			19			23			23	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		648			545			2255			685	
Travel Time (s)		14.7			12.4			51.3			15.6	
Confl. Peds. (#/hr)			2	2			6		5	5		6
Peak Hour Factor	0.69	0.76	0.75	0.75	0.70	0.72	0.67	0.88	0.73	0.75	0.88	0.65
Heavy Vehicles (%)	0%	3%	4%	9%	0%	4%	3%	1%	0%	2%	3%	3%
Adj. Flow (vph)	32	151	36	72	111	36	48	497	64	60	399	52
Shared Lane Traffic (%)												
Lane Group Flow (vph)	32	187	0	72	147	0	48	561	0	60	451	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		4.0	15.0		4.0	15.0	
Minimum Split (s)	24.6	24.6		24.6	24.6		9.6	26.6		9.6	26.6	
Total Split (s)	24.0	24.0		24.0	24.0		14.0	42.0		14.0	42.0	
Total Split (%)	30.0%	30.0%		30.0%	30.0%		17.5%	52.5%		17.5%	52.5%	
Maximum Green (s)	18.4	18.4		18.4	18.4		8.4	36.4		8.4	36.4	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	2.1	2.1		2.1	2.1		2.1	2.1		2.1	2.1	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.6	5.6		5.6	5.6		5.6	5.6		5.6	5.6	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	

Lanes, Volumes, Timings
4001: Schaefer Hwy & Lyndon St

11/01/2021

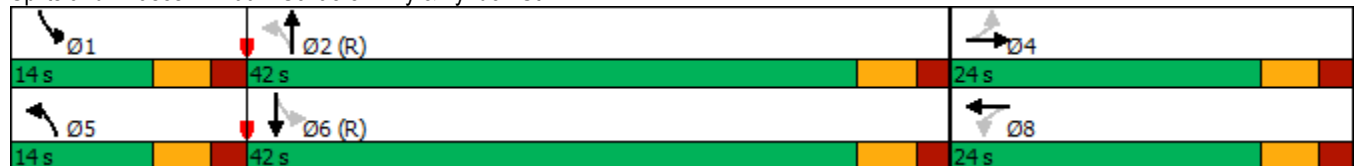


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	Min	Min		Min	Min		None	C-Max		None	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	12.0	12.0		12.0	12.0			14.0			14.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	13.4	13.4		13.4	13.4		51.9	47.9		52.2	48.0	
Actuated g/C Ratio	0.17	0.17		0.17	0.17		0.65	0.60		0.65	0.60	
v/c Ratio	0.16	0.60		0.46	0.46		0.07	0.27		0.10	0.22	
Control Delay	28.6	36.3		38.8	30.0		7.1	16.1		5.3	9.0	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	28.6	36.3		38.8	30.0		7.1	16.1		5.3	9.0	
LOS	C	D		D	C		A	B		A	A	
Approach Delay		35.2			32.9			15.4			8.5	
Approach LOS		D			C			B			A	
Queue Length 50th (ft)	14	81		33	58		12	125		8	53	
Queue Length 95th (ft)	27	110		56	77		m19	178		19	92	
Internal Link Dist (ft)		568			465			2175			605	
Turn Bay Length (ft)	80			80			80			80		
Base Capacity (vph)	282	420		215	431		684	2106		629	2068	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.11	0.45		0.33	0.34		0.07	0.27		0.10	0.22	

Intersection Summary

Area Type: Other
 Cycle Length: 80
 Actuated Cycle Length: 80
 Offset: 14 (18%), Referenced to phase 2:NBT and 6:SBTL, Start of Green
 Natural Cycle: 65
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.60
 Intersection Signal Delay: 18.4
 Intersection LOS: B
 Intersection Capacity Utilization 56.7%
 ICU Level of Service B
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4001: Schaefer Hwy & Lyndon St



Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	5	166	2	1	158	4	1	0	6	5	0	6
Future Vol, veh/h	5	166	2	1	158	4	1	0	6	5	0	6
Conflicting Peds, #/hr	3	0	1	1	0	3	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	63	94	60	60	86	60	60	60	60	63	60	60
Heavy Vehicles, %	0	3	0	0	3	0	0	0	0	0	0	0
Mvmt Flow	8	177	3	2	184	7	2	0	10	8	0	10

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	194	0	0	181	0	0	393	394	180	395	392	191
Stage 1	-	-	-	-	-	-	196	196	-	195	195	-
Stage 2	-	-	-	-	-	-	197	198	-	200	197	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1391	-	-	1407	-	-	570	546	868	568	547	856
Stage 1	-	-	-	-	-	-	810	742	-	811	743	-
Stage 2	-	-	-	-	-	-	809	741	-	806	742	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1387	-	-	1406	-	-	559	539	867	557	540	854
Mov Cap-2 Maneuver	-	-	-	-	-	-	559	539	-	557	540	-
Stage 1	-	-	-	-	-	-	804	737	-	804	739	-
Stage 2	-	-	-	-	-	-	798	737	-	792	737	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0.1			9.5			10.3		
HCM LOS							A			B		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	804	1387	-	-	1406	-	-	691
HCM Lane V/C Ratio	0.015	0.006	-	-	0.001	-	-	0.026
HCM Control Delay (s)	9.5	7.6	0	-	7.6	0	-	10.3
HCM Lane LOS	A	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0.1

Intersection												
Int Delay, s/veh	1.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	3	1	8	7	6	7	2	290	10	7	271	1
Future Vol, veh/h	3	1	8	7	6	7	2	290	10	7	271	1
Conflicting Peds, #/hr	0	0	0	0	0	0	1	0	1	1	0	1
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	50	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	60	60	60	60	60	60	100	85	63	60	87	60
Heavy Vehicles, %	0	0	0	0	0	0	0	5	0	0	3	0
Mvmt Flow	5	2	13	12	10	12	2	341	16	12	311	2

Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	701	699	313	698	692	350	314	0	0	358	0	0
Stage 1	337	337	-	354	354	-	-	-	-	-	-	-
Stage 2	364	362	-	344	338	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	356	366	732	358	370	698	1258	-	-	1212	-	-
Stage 1	681	645	-	667	634	-	-	-	-	-	-	-
Stage 2	659	629	-	676	644	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	339	361	731	347	365	697	1257	-	-	1211	-	-
Mov Cap-2 Maneuver	339	361	-	347	365	-	-	-	-	-	-	-
Stage 1	679	638	-	665	632	-	-	-	-	-	-	-
Stage 2	637	627	-	655	637	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	12	14.1	0	0.3
HCM LOS	B	B		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1257	-	-	532	429	1211	-
HCM Lane V/C Ratio	0.002	-	-	0.038	0.078	0.01	-
HCM Control Delay (s)	7.9	-	-	12	14.1	8	-
HCM Lane LOS	A	-	-	B	B	A	-
HCM 95th %tile Q(veh)	0	-	-	0.1	0.3	0	-

Lanes, Volumes, Timings
1011: Schaefer Hwy & Grand River Ave

11/01/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↕↕↕		↖	↕↕↕		↖	↕	↗	↖	↕↕	
Traffic Volume (vph)	35	416	63	19	429	98	53	239	18	55	261	16
Future Volume (vph)	35	416	63	19	429	98	53	239	18	55	261	16
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	115		0	95		0	155		0	80		240
Storage Lanes	1		0	0		0	1		1	1		1
Taper Length (ft)	65			65			25			65		
Lane Util. Factor	1.00	0.91	0.91	1.00	0.91	0.91	1.00	1.00	1.00	1.00	0.95	0.95
Ped Bike Factor		1.00		1.00					0.99	1.00		
Frt		0.976			0.969				0.850		0.991	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	5009	0	1805	4955	0	1805	1900	1615	1770	3544	0
Flt Permitted	0.420			0.431			0.537			0.332		
Satd. Flow (perm)	798	5009	0	818	4955	0	1020	1900	1593	618	3544	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		66			111				34			8
Link Speed (mph)		35			35			30				30
Link Distance (ft)		932			422			332				1209
Travel Time (s)		18.2			8.2			7.5				27.5
Confl. Peds. (#/hr)			2	2					2	2		
Peak Hour Factor	0.80	0.88	0.72	0.59	0.92	0.82	0.74	0.74	0.60	0.76	0.86	0.80
Heavy Vehicles (%)	0%	1%	0%	0%	0%	7%	0%	0%	0%	2%	1%	0%
Adj. Flow (vph)	44	473	88	32	466	120	72	323	30	72	303	20
Shared Lane Traffic (%)												
Lane Group Flow (vph)	44	561	0	32	586	0	72	323	30	72	323	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Perm	NA		Perm	NA		Perm	NA	Perm	Perm	NA	
Protected Phases		6			2			8				4
Permitted Phases	6			2			8		8	4		
Detector Phase	6	6		2	2		8	8	8	4	4	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		7.0	7.0	7.0	7.0	7.0	7.0
Minimum Split (s)	35.8	35.8		35.8	35.8		46.5	46.5	46.5	46.5	46.5	46.5
Total Split (s)	55.0	55.0		55.0	55.0		35.0	35.0	35.0	35.0	35.0	35.0
Total Split (%)	61.1%	61.1%		61.1%	61.1%		38.9%	38.9%	38.9%	38.9%	38.9%	38.9%
Maximum Green (s)	49.2	49.2		49.2	49.2		28.5	28.5	28.5	28.5	28.5	28.5
Yellow Time (s)	3.6	3.6		3.6	3.6		3.2	3.2	3.2	3.2	3.2	3.2
All-Red Time (s)	2.2	2.2		2.2	2.2		3.3	3.3	3.3	3.3	3.3	3.3
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.8	5.8		5.8	5.8		6.5	6.5	6.5	6.5	6.5	6.5
Lead/Lag												
Lead-Lag Optimize?												

Lanes, Volumes, Timings
 1011: Schaefer Hwy & Grand River Ave

11/01/2021

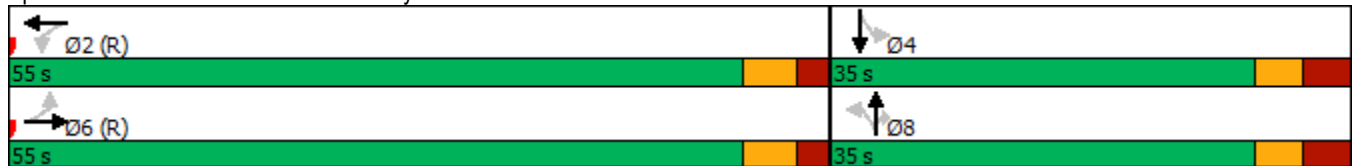


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None	None	None	None	
Walk Time (s)	7.0	7.0		7.0	7.0		10.0	10.0	10.0	10.0	10.0	
Flash Dont Walk (s)	23.0	23.0		23.0	23.0		30.0	30.0	30.0	30.0	30.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0	0	0	0	
Act Effct Green (s)	56.8	56.8		56.8	56.8		20.9	20.9	20.9	20.9	20.9	
Actuated g/C Ratio	0.63	0.63		0.63	0.63		0.23	0.23	0.23	0.23	0.23	
v/c Ratio	0.09	0.18		0.06	0.19		0.30	0.73	0.08	0.50	0.39	
Control Delay	8.9	6.9		8.1	5.8		24.6	36.1	10.4	40.8	28.7	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.3	0.0	0.0	0.0	
Total Delay	8.9	6.9		8.1	5.8		24.6	36.4	10.4	40.8	28.7	
LOS	A	A		A	A		C	D	B	D	C	
Approach Delay		7.0			5.9			32.6				30.9
Approach LOS		A			A			C				C
Queue Length 50th (ft)	9	37		6	32		36	177	0	36	80	
Queue Length 95th (ft)	25	65		13	57		56	193	16	59	99	
Internal Link Dist (ft)		852			342			252				1129
Turn Bay Length (ft)	115			95			155			80		
Base Capacity (vph)	503	3183		515	3165		323	601	527	195	1127	
Starvation Cap Reductn	0	0		0	0		0	48	0	0	0	
Spillback Cap Reductn	0	0		0	0		0	0	0	0	0	
Storage Cap Reductn	0	0		0	0		0	0	0	0	0	
Reduced v/c Ratio	0.09	0.18		0.06	0.19		0.22	0.58	0.06	0.37	0.29	

Intersection Summary

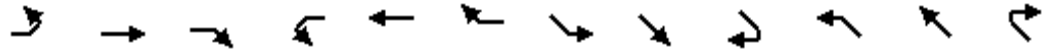
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 59 (66%), Referenced to phase 2:WBTL and 6:EBTL, Start of Green
 Natural Cycle: 85
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.73
 Intersection Signal Delay: 16.6
 Intersection LOS: B
 Intersection Capacity Utilization 64.5%
 ICU Level of Service C
 Analysis Period (min) 15

Splits and Phases: 1011: Schaefer Hwy & Grand River Ave



Lanes, Volumes, Timings
 1093: Grand River Ave & Jeffries Service Dr

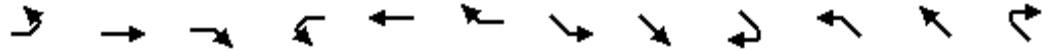
11/01/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↑↑↑↑						↑↑↑↑			↑↑↑↑	
Traffic Volume (vph)	4	262	62	0	0	0	9	404	0	0	330	13
Future Volume (vph)	4	262	62	0	0	0	9	404	0	0	330	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.86	0.86	0.86	1.00	1.00	1.00	0.91	0.91	1.00	1.00	0.91	0.91
Ped Bike Factor								1.00			1.00	
Frt		0.972									0.994	
Flt Protected		0.999						0.998				
Satd. Flow (prot)	0	6222	0	0	0	0	0	5127	0	0	5087	0
Flt Permitted		0.999						0.919				
Satd. Flow (perm)	0	6222	0	0	0	0	0	4721	0	0	5087	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		67									11	
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		342			423			587			812	
Travel Time (s)		6.7			8.2			11.4			15.8	
Confl. Peds. (#/hr)							3					3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.60	0.96	0.92	0.92	0.71	0.65
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	0%	1%	2%	2%	1%	8%
Adj. Flow (vph)	4	285	67	0	0	0	15	421	0	0	465	20
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	356	0	0	0	0	0	436	0	0	485	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Perm	NA					Perm	NA			NA	
Protected Phases		4						6			2	
Permitted Phases	4						6					
Detector Phase	4	4					6	6			2	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0			10.0	
Minimum Split (s)	49.4	49.4					37.0	37.0			37.0	
Total Split (s)	37.0	37.0					53.0	53.0			53.0	
Total Split (%)	41.1%	41.1%					58.9%	58.9%			58.9%	
Maximum Green (s)	29.6	29.6					47.0	47.0			47.0	
Yellow Time (s)	3.0	3.0					3.6	3.6			3.6	
All-Red Time (s)	4.4	4.4					2.4	2.4			2.4	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		7.4						6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0			3.0	
Recall Mode	None	None					C-Max	C-Max			C-Max	
Walk Time (s)	9.0	9.0					7.0	7.0			7.0	

Lanes, Volumes, Timings
 1093: Grand River Ave & Jeffries Service Dr

11/01/2021

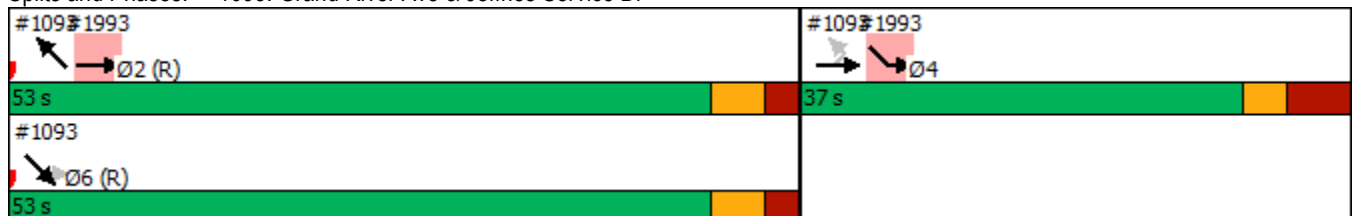


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Flash Dont Walk (s)	33.0	33.0					24.0	24.0			24.0	
Pedestrian Calls (#/hr)	0	0					0	0			0	
Act Effct Green (s)		11.0						65.6			65.6	
Actuated g/C Ratio		0.12						0.73			0.73	
v/c Ratio		0.43						0.13			0.13	
Control Delay		31.5						2.2			3.8	
Queue Delay		0.0						0.0			0.0	
Total Delay		31.5						2.2			3.8	
LOS		C						A			A	
Approach Delay		31.5						2.2			3.8	
Approach LOS		C						A			A	
Queue Length 50th (ft)		51						10			23	
Queue Length 95th (ft)		76						13			29	
Internal Link Dist (ft)		262			343			507			732	
Turn Bay Length (ft)												
Base Capacity (vph)		2091						3440			3709	
Starvation Cap Reductn		0						0			0	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.17						0.13			0.13	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 54 (60%), Referenced to phase 2:NWT and 6:SETL, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.43
 Intersection Signal Delay: 11.0
 Intersection Capacity Utilization 45.3%
 Analysis Period (min) 15
 Intersection LOS: B
 ICU Level of Service A

Splits and Phases: 1093: Grand River Ave & Jeffries Service Dr



Lanes, Volumes, Timings

1193: Left Turn Bridge & Grand River Ave & I-96 Service Dr

11/01/2021



Lane Group	WBL2	WBL	WBT	WBR	SET	SER	SER2	NWL	NWT
Lane Configurations			↑↑↑	↑	↑↑↑	↑		↑	↑↑↑
Traffic Volume (vph)	23	6	187	258	390	96	3	46	288
Future Volume (vph)	23	6	187	258	390	96	3	46	288
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0		0		140		260	
Storage Lanes		0		1		1		1	
Taper Length (ft)		65						65	
Lane Util. Factor	0.91	0.91	0.86	0.86	0.86	0.86	0.91	1.00	0.91
Frt			0.944	0.850	0.996	0.850			
Flt Protected			0.996					0.950	
Satd. Flow (prot)	0	0	4519	1362	4834	1376	0	1805	5136
Flt Permitted			0.996					0.950	
Satd. Flow (perm)	0	0	4519	1362	4834	1376	0	1805	5136
Right Turn on Red				Yes			Yes		
Satd. Flow (RTOR)			140	140		126			
Link Speed (mph)			35		35				35
Link Distance (ft)			480		422				587
Travel Time (s)			9.4		8.2				11.4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.94	0.83	0.75	0.60	0.73
Heavy Vehicles (%)	2%	2%	2%	2%	1%	1%	0%	0%	1%
Adj. Flow (vph)	25	7	203	280	415	116	4	77	395
Shared Lane Traffic (%)				50%		10%			
Lane Group Flow (vph)	0	0	375	140	427	108	0	77	395
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right	Right	Left	Left
Median Width(ft)			0		12				12
Link Offset(ft)			0		0				0
Crosswalk Width(ft)			16		16				16
Two way Left Turn Lane									
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	15		9		9	9	15	
Turn Type	Perm	Perm	NA	Perm	NA	Perm		Prot	NA
Protected Phases			4		6			5	2
Permitted Phases	4	4		4		6			
Detector Phase	4	4	4	4	6	6		5	2
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0		7.0	10.0
Minimum Split (s)	55.5	55.5	55.5	55.5	37.9	37.9		12.9	37.9
Total Split (s)	37.0	37.0	37.0	37.0	40.0	40.0		13.0	53.0
Total Split (%)	41.1%	41.1%	41.1%	41.1%	44.4%	44.4%		14.4%	58.9%
Maximum Green (s)	29.5	29.5	29.5	29.5	34.1	34.1		7.1	47.1
Yellow Time (s)	3.0	3.0	3.0	3.0	3.6	3.6		3.6	3.6
All-Red Time (s)	4.5	4.5	4.5	4.5	2.3	2.3		2.3	2.3
Lost Time Adjust (s)			0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)			7.5	7.5	5.9	5.9		5.9	5.9
Lead/Lag					Lead	Lead		Lag	
Lead-Lag Optimize?					Yes	Yes		Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max		None	C-Max

Lanes, Volumes, Timings
 1193: Left Turn Bridge & Grand River Ave & I-96 Service Dr

11/01/2021

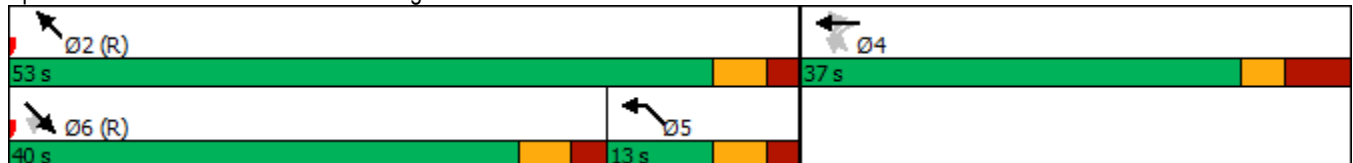


Lane Group	WBL2	WBL	WBT	WBR	SET	SER	SER2	NWL	NWT
Walk Time (s)	9.0	9.0	9.0	9.0	7.0	7.0			7.0
Flash Dont Walk (s)	39.0	39.0	39.0	39.0	25.0	25.0			25.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0			0
Act Effct Green (s)			11.7	11.7	54.5	54.5		7.1	64.9
Actuated g/C Ratio			0.13	0.13	0.61	0.61		0.08	0.72
v/c Ratio			0.53	0.47	0.15	0.12		0.54	0.11
Control Delay			25.1	11.7	6.9	0.8		52.1	0.6
Queue Delay			0.0	0.0	0.0	0.0		0.0	0.0
Total Delay			25.1	11.7	6.9	0.8		52.1	0.6
LOS			C	B	A	A		D	A
Approach Delay			21.5		5.6				9.0
Approach LOS			C		A				A
Queue Length 50th (ft)			48	0	32	1		43	0
Queue Length 95th (ft)			76	58	44	2		58	0
Internal Link Dist (ft)			400		342				507
Turn Bay Length (ft)						140		260	
Base Capacity (vph)			1575	540	2925	882		142	3701
Starvation Cap Reductn			0	0	0	0		0	0
Spillback Cap Reductn			0	0	0	0		0	0
Storage Cap Reductn			0	0	0	0		0	0
Reduced v/c Ratio			0.24	0.26	0.15	0.12		0.54	0.11

Intersection Summary

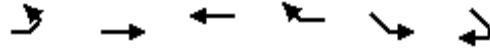
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 66 (73%), Referenced to phase 2:NWT and 6:SET, Start of Green
 Natural Cycle: 110
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.54
 Intersection Signal Delay: 12.0
 Intersection LOS: B
 Intersection Capacity Utilization 38.6%
 ICU Level of Service A
 Analysis Period (min) 15

Splits and Phases: 1193: Left Turn Bridge & Grand River Ave & I-96 Service Dr



Lanes, Volumes, Timings
 1993: Jeffries Service Dr & Left Turn Bridge

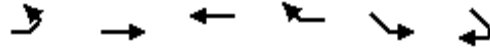
11/01/2021



Lane Group	EBL	EBT	WBT	WBR	SEL	SER	Ø6
Lane Configurations		↑↑↑			↑↑		
Traffic Volume (vph)	0	223	0	0	102	0	
Future Volume (vph)	0	223	0	0	102	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	0.91	1.00	1.00	0.97	1.00	
Fr							
Flt Protected					0.950		
Satd. Flow (prot)	0	5085	0	0	3433	0	
Flt Permitted					0.950		
Satd. Flow (perm)	0	5085	0	0	3433	0	
Right Turn on Red				Yes	Yes	Yes	
Satd. Flow (RTOR)					1170		
Link Speed (mph)		35	35		30		
Link Distance (ft)		191	342		422		
Travel Time (s)		3.7	6.7		9.6		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	242	0	0	111	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	242	0	0	111	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		0	0		24		
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		16	16		16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15			9	15	9	
Turn Type		NA			Prot		
Protected Phases		2			4		6
Permitted Phases							
Detector Phase		2			4		
Switch Phase							
Minimum Initial (s)		10.0			10.0		10.0
Minimum Split (s)		37.0			49.4		37.0
Total Split (s)		53.0			37.0		53.0
Total Split (%)		58.9%			41.1%		59%
Maximum Green (s)		47.0			29.6		47.0
Yellow Time (s)		3.6			3.0		3.6
All-Red Time (s)		2.4			4.4		2.4
Lost Time Adjust (s)		0.0			0.0		
Total Lost Time (s)		6.0			7.4		
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)		3.0			3.0		3.0
Recall Mode		C-Max			None		C-Max
Walk Time (s)		7.0			9.0		7.0
Flash Dont Walk (s)		24.0			33.0		24.0
Pedestrian Calls (#/hr)		0			0		0
Act Effct Green (s)		65.6			11.0		

Lanes, Volumes, Timings
 1993: Jeffries Service Dr & Left Turn Bridge

11/01/2021

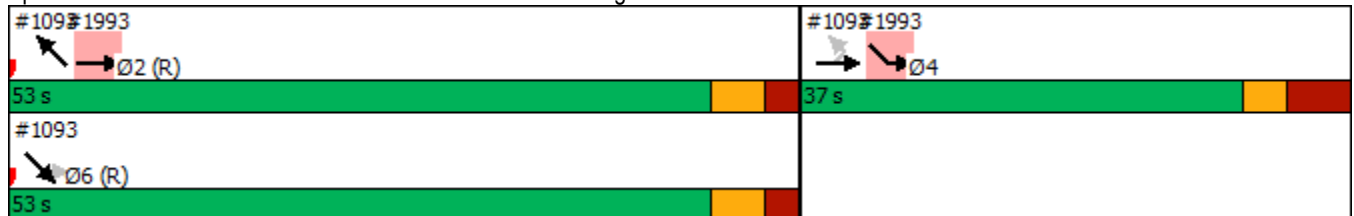


Lane Group	EBL	EBT	WBT	WBR	SEL	SER	Ø6
Actuated g/C Ratio		0.73			0.12		
v/c Ratio		0.07			0.08		
Control Delay		2.0			0.1		
Queue Delay		0.0			0.0		
Total Delay		2.0			0.1		
LOS		A			A		
Approach Delay		2.0			0.1		
Approach LOS		A			A		
Queue Length 50th (ft)		6			0		
Queue Length 95th (ft)		10			0		
Internal Link Dist (ft)		111	262		342		
Turn Bay Length (ft)							
Base Capacity (vph)		3704			1914		
Starvation Cap Reductn		0			0		
Spillback Cap Reductn		0			0		
Storage Cap Reductn		0			0		
Reduced v/c Ratio		0.07			0.06		

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	54 (60%), Referenced to phase 2:NWT and 6:SETL, Start of Green
Natural Cycle:	90
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.43
Intersection Signal Delay:	1.4
Intersection LOS:	A
Intersection Capacity Utilization:	27.8%
ICU Level of Service:	A
Analysis Period (min):	15

Splits and Phases: 1993: Jeffries Service Dr & Left Turn Bridge



Lanes, Volumes, Timings
2609: Schaefer Hwy & Schoolcraft Rd

11/01/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	19	86	4	23	90	32	12	317	43	35	305	25
Future Volume (vph)	19	86	4	23	90	32	12	317	43	35	305	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	60		0	60		0	80		0	80		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00	1.00		0.99	0.99		1.00	1.00		0.99	1.00	
Fr _t		0.993			0.964			0.978			0.984	
Fl _t Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1849	0	1805	1808	0	1805	1789	0	1805	1849	0
Fl _t Permitted	0.662			0.665			0.450			0.420		
Satd. Flow (perm)	1256	1849	0	1252	1808	0	854	1789	0	793	1849	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		3			19			14			9	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		699			1470			1209			215	
Travel Time (s)		15.9			33.4			27.5			4.9	
Confl. Peds. (#/hr)	1		6	6		1	1		6	6		1
Peak Hour Factor	0.60	0.63	0.60	0.72	0.80	0.89	0.75	0.91	0.72	0.88	0.90	0.63
Heavy Vehicles (%)	0%	2%	0%	0%	1%	0%	0%	4%	0%	0%	1%	0%
Adj. Flow (vph)	32	137	7	32	113	36	16	348	60	40	339	40
Shared Lane Traffic (%)												
Lane Group Flow (vph)	32	144	0	32	149	0	16	408	0	40	379	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		
Detector Phase	8	8		4	4		1	6		5	2	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0		5.0	9.0		5.0	9.0	
Minimum Split (s)	28.8	28.8		28.8	28.8		10.6	26.6		10.6	26.6	
Total Split (s)	27.0	27.0		27.0	27.0		12.0	41.0		12.0	41.0	
Total Split (%)	33.8%	33.8%		33.8%	33.8%		15.0%	51.3%		15.0%	51.3%	
Maximum Green (s)	20.2	20.2		20.2	20.2		6.4	34.4		6.4	34.4	
Yellow Time (s)	3.8	3.8		3.8	3.8		3.5	3.6		3.5	3.6	
All-Red Time (s)	3.0	3.0		3.0	3.0		2.1	3.0		2.1	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.8	6.8		6.8	6.8		5.6	6.6		5.6	6.6	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	

Lanes, Volumes, Timings
 2609: Schaefer Hwy & Schoolcraft Rd

11/01/2021

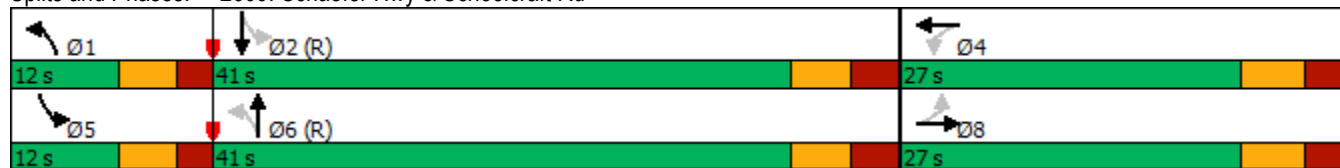


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	Max	Max		Max	Max		Max	C-Max		Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0			13.0			13.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	20.2	20.2		20.2	20.2		41.8	34.4		41.8	34.4	
Actuated g/C Ratio	0.25	0.25		0.25	0.25		0.52	0.43		0.52	0.43	
v/c Ratio	0.10	0.31		0.10	0.32		0.03	0.53		0.08	0.47	
Control Delay	24.1	25.9		24.1	23.3		7.1	19.2		7.4	18.4	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	24.1	25.9		24.1	23.3		7.1	19.2		7.4	18.4	
LOS	C	C		C	C		A	B		A	B	
Approach Delay		25.6			23.5			18.7			17.3	
Approach LOS		C			C			B			B	
Queue Length 50th (ft)	12	57		12	52		3	140		8	128	
Queue Length 95th (ft)	22	71		27	88		9	222		19	203	
Internal Link Dist (ft)		619			1390			1129			135	
Turn Bay Length (ft)	60			60			80			80		
Base Capacity (vph)	317	469		316	470		522	777		495	800	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.10	0.31		0.10	0.32		0.03	0.53		0.08	0.47	

Intersection Summary

Area Type: Other
 Cycle Length: 80
 Actuated Cycle Length: 80
 Offset: 0 (0%), Referenced to phase 2:SBTL and 6:NBTL, Start of Green
 Natural Cycle: 70
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.53
 Intersection Signal Delay: 20.0
 Intersection LOS: B
 Intersection Capacity Utilization 57.7%
 ICU Level of Service B
 Analysis Period (min) 15

Splits and Phases: 2609: Schaefer Hwy & Schoolcraft Rd



Lanes, Volumes, Timings
3011: Schaefer Hwy & I-96 Service Dr

11/01/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				↙	↕		↘	↕			↕	
Traffic Volume (vph)	0	0	0	94	122	3	142	307	0	0	190	153
Future Volume (vph)	0	0	0	94	122	3	142	307	0	0	190	153
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	90		0	0		0
Storage Lanes	0		0	1		0	1		0	0		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95
Ped Bike Factor							1.00					0.99
Fr t					0.997							0.932
Flt Protected				0.950			0.950					
Satd. Flow (prot)	0	0	0	1770	3529	0	1752	3574	0	0	3319	0
Flt Permitted				0.950			0.509					
Satd. Flow (perm)	0	0	0	1770	3529	0	936	3574	0	0	3319	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					2							189
Link Speed (mph)		35			35			30				30
Link Distance (ft)		298			375			384				332
Travel Time (s)		5.8			7.3			8.7				7.5
Confl. Peds. (#/hr)							4					4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.87	0.76	0.92	0.92	0.84	0.81
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	3%	1%	2%	2%	1%	0%
Adj. Flow (vph)	0	0	0	102	133	3	163	404	0	0	226	189
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	102	136	0	163	404	0	0	415	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type				Perm	NA		pm+pt	NA				NA
Protected Phases					8			5	2			6
Permitted Phases				8				2				
Detector Phase				8	8			5	2			6
Switch Phase												
Minimum Initial (s)				10.0	10.0		7.0	10.0				10.0
Minimum Split (s)				28.7	28.7		12.4	23.4				23.4
Total Split (s)				31.0	31.0		14.0	59.0				45.0
Total Split (%)				34.4%	34.4%		15.6%	65.6%				50.0%
Maximum Green (s)				25.3	25.3		8.6	53.6				39.6
Yellow Time (s)				3.0	3.0		3.2	3.2				3.2
All-Red Time (s)				2.7	2.7		2.2	2.2				2.2
Lost Time Adjust (s)				0.0	0.0		0.0	0.0				0.0
Total Lost Time (s)				5.7	5.7		5.4	5.4				5.4
Lead/Lag							Lag					Lead
Lead-Lag Optimize?							Yes					Yes

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
Minimum Split (s)	30.5
Total Split (s)	31.0
Total Split (%)	34%
Maximum Green (s)	24.5
Yellow Time (s)	5.0
All-Red Time (s)	1.5
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	

Lanes, Volumes, Timings
 3011: Schaefer Hwy & I-96 Service Dr

11/01/2021

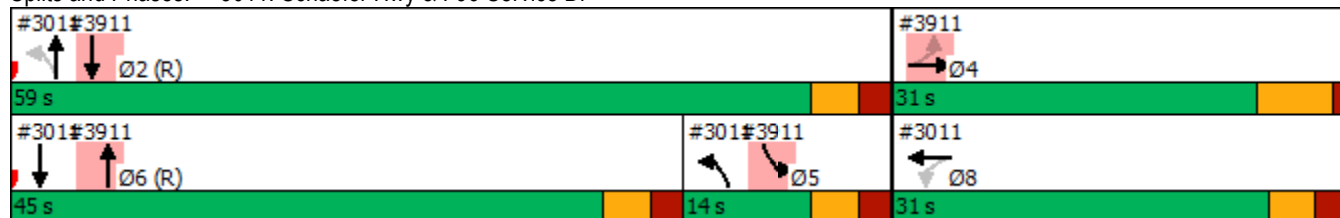


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vehicle Extension (s)				3.0	3.0		3.0	3.0				3.0
Recall Mode				None	None		None	C-Max				C-Max
Walk Time (s)				7.0	7.0			7.0				7.0
Flash Dont Walk (s)				16.0	16.0			11.0				11.0
Pedestrian Calls (#/hr)				0	0			0				0
Act Effct Green (s)				12.2	12.2		66.7	66.7				52.7
Actuated g/C Ratio				0.14	0.14		0.74	0.74				0.59
v/c Ratio				0.42	0.28		0.21	0.15				0.21
Control Delay				33.1	27.8		2.8	2.0				8.3
Queue Delay				0.0	0.0		0.0	0.0				0.0
Total Delay				33.1	27.8		2.8	2.0				8.3
LOS				C	C		A	A				A
Approach Delay					30.1			2.2				8.3
Approach LOS					C			A				A
Queue Length 50th (ft)				62	42		15	21				22
Queue Length 95th (ft)				113	73		35	33				71
Internal Link Dist (ft)		218			295			304				252
Turn Bay Length (ft)							90					
Base Capacity (vph)				497	993		771	2647				2021
Starvation Cap Reductn				0	0		0	0				0
Spillback Cap Reductn				0	0		0	43				0
Storage Cap Reductn				0	0		0	0				0
Reduced v/c Ratio				0.21	0.14		0.21	0.16				0.21

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	87 (97%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Natural Cycle:	70
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.58
Intersection Signal Delay:	9.7
Intersection LOS:	A
Intersection Capacity Utilization:	45.0%
ICU Level of Service:	A
Analysis Period (min):	15

Splits and Phases: 3011: Schaefer Hwy & I-96 Service Dr



Lane Group	Ø4
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	17.0
Pedestrian Calls (#/hr)	0
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

Lanes, Volumes, Timings
3911: Schaefer Hwy & Jeffries Service Dr

11/01/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕↕↕						↕↕		↘	↕↕	
Traffic Volume (vph)	139	103	149	0	0	0	0	308	78	42	242	0
Future Volume (vph)	139	103	149	0	0	0	0	308	78	42	242	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	0		0	90		0
Storage Lanes	0		0	0		0	0		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	0.91	0.91	0.91	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00
Ped Bike Factor								1.00		1.00		
Fr _t		0.943						0.969				
Fl _t Protected		0.983								0.950		
Satd. Flow (prot)	0	4714	0	0	0	0	0	3424	0	1770	3574	0
Fl _t Permitted		0.983								0.950		
Satd. Flow (perm)	0	4714	0	0	0	0	0	3424	0	1764	3574	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		162						47				
Link Speed (mph)		35			35			30				30
Link Distance (ft)		417			354			291				384
Travel Time (s)		8.1			6.9			6.6				8.7
Confl. Peds. (#/hr)									4	4		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.75	0.72	0.81	0.79	0.92
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	1%	2%	1%	2%
Adj. Flow (vph)	151	112	162	0	0	0	0	411	108	52	306	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	425	0	0	0	0	0	519	0	52	306	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane								Yes				Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Perm	NA						NA		Prot	NA	
Protected Phases		4						6		5	2	
Permitted Phases	4											
Detector Phase	4	4						6		5	2	
Switch Phase												
Minimum Initial (s)	10.0	10.0						10.0		7.0	10.0	
Minimum Split (s)	30.5	30.5						23.4		12.4	23.4	
Total Split (s)	31.0	31.0						45.0		14.0	59.0	
Total Split (%)	34.4%	34.4%						50.0%		15.6%	65.6%	
Maximum Green (s)	24.5	24.5						39.6		8.6	53.6	
Yellow Time (s)	5.0	5.0						3.2		3.2	3.2	
All-Red Time (s)	1.5	1.5						2.2		2.2	2.2	
Lost Time Adjust (s)		0.0						0.0		0.0	0.0	
Total Lost Time (s)		6.5						5.4		5.4	5.4	
Lead/Lag								Lead		Lag		
Lead-Lag Optimize?								Yes		Yes		

Lane Group	Ø8
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Turn Type	
Protected Phases	8
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
Minimum Split (s)	28.7
Total Split (s)	31.0
Total Split (%)	34%
Maximum Green (s)	25.3
Yellow Time (s)	3.0
All-Red Time (s)	2.7
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	

Lanes, Volumes, Timings
 3911: Schaefer Hwy & Jeffries Service Dr

11/01/2021

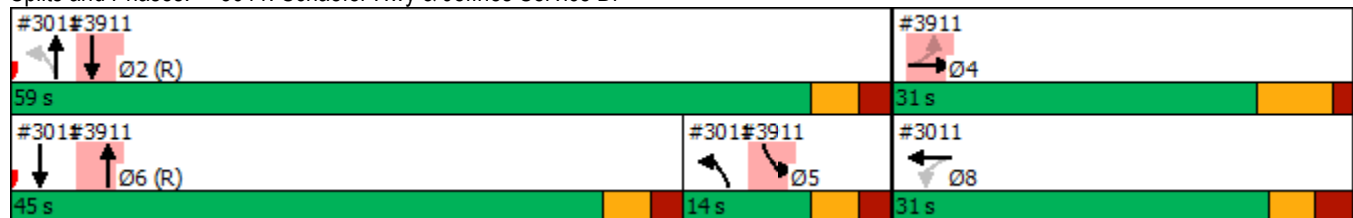


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vehicle Extension (s)	3.0	3.0						3.0		3.0	3.0	
Recall Mode	None	None						C-Max		None	C-Max	
Walk Time (s)	7.0	7.0						7.0			7.0	
Flash Dont Walk (s)	17.0	17.0						11.0			11.0	
Pedestrian Calls (#/hr)	0	0						0			0	
Act Effct Green (s)		11.4						52.7		8.6	66.7	
Actuated g/C Ratio		0.13						0.59		0.10	0.74	
v/c Ratio		0.58						0.26		0.31	0.12	
Control Delay		25.6						8.8		45.5	3.7	
Queue Delay		0.0						0.0		0.0	0.0	
Total Delay		25.6						8.8		45.5	3.7	
LOS		C						A		D	A	
Approach Delay		25.6						8.8			9.8	
Approach LOS		C						A			A	
Queue Length 50th (ft)		52						61		22	35	
Queue Length 95th (ft)		80						77		51	45	
Internal Link Dist (ft)		337			274			211			304	
Turn Bay Length (ft)										90		
Base Capacity (vph)		1401						2023		169	2647	
Starvation Cap Reductn		0						0		0	0	
Spillback Cap Reductn		0						0		0	0	
Storage Cap Reductn		0						0		0	0	
Reduced v/c Ratio		0.30						0.26		0.31	0.12	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	87 (97%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Natural Cycle:	70
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.58
Intersection Signal Delay:	14.6
Intersection LOS:	B
Intersection Capacity Utilization:	45.0%
ICU Level of Service:	A
Analysis Period (min):	15

Splits and Phases: 3911: Schaefer Hwy & Jeffries Service Dr



Lane Group	Ø8
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	16.0
Pedestrian Calls (#/hr)	0
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

Lanes, Volumes, Timings
4001: Schaefer Hwy & Lyndon St

11/01/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	29	81	26	44	102	23	28	298	40	31	295	25
Future Volume (vph)	29	81	26	44	102	23	28	298	40	31	295	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	80		0	80		0	80		0	80		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	1.00	0.99		1.00	1.00		0.99	1.00		1.00	1.00	
Fr _t		0.956			0.975			0.979			0.983	
Fl _t Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1807	0	1805	1848	0	1805	3479	0	1752	3537	0
Fl _t Permitted	0.665			0.677			0.534			0.504		
Satd. Flow (perm)	1262	1807	0	1280	1848	0	1009	3479	0	929	3537	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		35			17			31			24	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		648			545			2274			685	
Travel Time (s)		14.7			12.4			51.7			15.6	
Confl. Peds. (#/hr)	2		6	6		2	6		1	1		6
Peak Hour Factor	0.73	0.92	0.72	0.73	0.85	0.95	0.68	0.91	0.77	0.60	0.91	0.60
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	1%	3%	3%	0%	0%
Adj. Flow (vph)	40	88	36	60	120	24	41	327	52	52	324	42
Shared Lane Traffic (%)												
Lane Group Flow (vph)	40	124	0	60	144	0	41	379	0	52	366	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		4.0	15.0		4.0	15.0	
Minimum Split (s)	24.6	24.6		24.6	24.6		9.6	26.6		9.6	26.6	
Total Split (s)	24.0	24.0		24.0	24.0		12.0	24.0		12.0	24.0	
Total Split (%)	40.0%	40.0%		40.0%	40.0%		20.0%	40.0%		20.0%	40.0%	
Maximum Green (s)	18.4	18.4		18.4	18.4		6.4	18.4		6.4	18.4	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	2.1	2.1		2.1	2.1		2.1	2.1		2.1	2.1	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.6	5.6		5.6	5.6		5.6	5.6		5.6	5.6	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	

Lanes, Volumes, Timings
4001: Schaefer Hwy & Lyndon St

11/01/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	Min	Min		Min	Min		None	C-Max		None	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	12.0	12.0		12.0	12.0			14.0			14.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	10.8	10.8		10.8	10.8		34.5	30.6		35.8	33.1	
Actuated g/C Ratio	0.18	0.18		0.18	0.18		0.58	0.51		0.60	0.55	
v/c Ratio	0.18	0.35		0.26	0.41		0.06	0.21		0.08	0.19	
Control Delay	22.3	18.7		23.8	22.8		5.1	9.4		5.1	8.3	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	22.3	18.7		23.8	22.8		5.1	9.4		5.1	8.3	
LOS	C	B		C	C		A	A		A	A	
Approach Delay		19.6			23.1			9.0			7.9	
Approach LOS		B			C			A			A	
Queue Length 50th (ft)	13	29		19	42		4	37		6	21	
Queue Length 95th (ft)	27	65		36	76		11	72		12	70	
Internal Link Dist (ft)		568			465			2194			605	
Turn Bay Length (ft)	80			80			80			80		
Base Capacity (vph)	387	578		392	578		668	1790		645	1959	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.10	0.21		0.15	0.25		0.06	0.21		0.08	0.19	

Intersection Summary

Area Type: Other
 Cycle Length: 60
 Actuated Cycle Length: 60
 Offset: 14 (23%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 65
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.41
 Intersection Signal Delay: 12.4
 Intersection LOS: B
 Intersection Capacity Utilization 57.5%
 ICU Level of Service B
 Analysis Period (min) 15

Splits and Phases: 4001: Schaefer Hwy & Lyndon St



Intersection												
Int Delay, s/veh	0.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	8	152	4	0	136	0	0	0	0	0	0	8
Future Vol, veh/h	8	152	4	0	136	0	0	0	0	0	0	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	60	60	60	60	60	60	25	25	25	25	25	60
Heavy Vehicles, %	0	2	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	13	253	7	0	227	0	0	0	0	0	0	13

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	227	0	0	260	0	0	517	510	257	510	513	227
Stage 1	-	-	-	-	-	-	283	283	-	227	227	-
Stage 2	-	-	-	-	-	-	234	227	-	283	286	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1353	-	-	1316	-	-	472	469	787	477	468	817
Stage 1	-	-	-	-	-	-	728	681	-	780	720	-
Stage 2	-	-	-	-	-	-	774	720	-	728	679	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1353	-	-	1316	-	-	460	464	787	473	463	817
Mov Cap-2 Maneuver	-	-	-	-	-	-	460	464	-	473	463	-
Stage 1	-	-	-	-	-	-	720	674	-	771	720	-
Stage 2	-	-	-	-	-	-	761	720	-	720	672	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.4	0	0	9.5
HCM LOS			A	A

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	1353	-	-	1316	-	-	817
HCM Lane V/C Ratio	-	0.01	-	-	-	-	-	0.016
HCM Control Delay (s)	0	7.7	0	-	0	-	-	9.5
HCM Lane LOS	A	A	A	-	A	-	-	A
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	0.1

Intersection												
Int Delay, s/veh	0.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	2	1	6	3	1	7	4	200	16	5	236	3
Future Vol, veh/h	2	1	6	3	1	7	4	200	16	5	236	3
Conflicting Peds, #/hr	0	0	1	1	0	0	2	0	0	0	0	2
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	50	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	60	60	60	60	60	60	100	81	60	60	95	75
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	3	2	10	5	2	12	4	247	27	8	248	4

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	544	550	253	542	539	261	254	0	0	274	0	0
Stage 1	268	268	-	269	269	-	-	-	-	-	-	-
Stage 2	276	282	-	273	270	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	453	446	791	454	452	783	1323	-	-	1301	-	-
Stage 1	742	691	-	741	690	-	-	-	-	-	-	-
Stage 2	735	681	-	737	690	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	441	441	789	444	447	783	1320	-	-	1301	-	-
Mov Cap-2 Maneuver	441	441	-	444	447	-	-	-	-	-	-	-
Stage 1	738	685	-	739	688	-	-	-	-	-	-	-
Stage 2	720	679	-	721	684	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	10.9		11.1		0.1		0.2	
HCM LOS	B		B					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1320	-	-	625	613	1301	-	-
HCM Lane V/C Ratio	0.003	-	-	0.024	0.03	0.006	-	-
HCM Control Delay (s)	7.7	-	-	10.9	11.1	7.8	-	-
HCM Lane LOS	A	-	-	B	B	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0.1	0.1	0	-	-



INNOVATIVE IDEAS
EXCEPTIONAL DESIGN
UNMATCHED CLIENT SERVICE

NO BUILD CONDITIONS



Lanes, Volumes, Timings
1011: Schaefer Hwy & Grand River Ave

07/07/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↕↕↕		↘	↕↕↕		↘	↕	↗	↘	↕↕	
Traffic Volume (vph)	59	460	100	27	516	137	79	333	10	68	353	30
Future Volume (vph)	59	460	100	27	516	137	79	333	10	68	353	30
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	115		0	95		0	155		0	80		240
Storage Lanes	1		0	0		0	1		1	1		1
Taper Length (ft)	65			65			25			65		
Lane Util. Factor	1.00	0.91	0.91	1.00	0.91	0.91	1.00	1.00	1.00	1.00	0.95	0.95
Ped Bike Factor	1.00	1.00		1.00	0.99		1.00		0.99	1.00	1.00	
Fr _t		0.971			0.964				0.850		0.988	
Fl _t Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	4943	0	1805	4835	0	1805	1845	1615	1583	3369	0
Fl _t Permitted	0.362			0.406			0.451			0.283		
Satd. Flow (perm)	686	4943	0	771	4835	0	856	1845	1593	471	3369	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		99			142				34			10
Link Speed (mph)		35			35			30				30
Link Distance (ft)		932			427			332				1209
Travel Time (s)		18.2			8.3			7.5				27.5
Confl. Peds. (#/hr)	5		3	3		5	3		2	2		3
Peak Hour Factor	0.71	0.92	0.83	0.78	0.94	0.78	0.83	0.89	0.63	0.81	0.93	0.91
Heavy Vehicles (%)	0%	2%	0%	0%	2%	5%	0%	3%	0%	14%	6%	3%
Adj. Flow (vph)	83	500	120	35	549	176	95	374	16	84	380	33
Shared Lane Traffic (%)												
Lane Group Flow (vph)	83	620	0	35	725	0	95	374	16	84	413	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2	1	1		2
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right	Left		Thru
Leading Detector (ft)	20	100		20	100		20	100	20	20		100
Trailing Detector (ft)	0	0		0	0		0	0	0	0		0
Detector 1 Position(ft)	0	0		0	0		0	0	0	0		0
Detector 1 Size(ft)	20	6		20	6		20	6	20	20		6
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0		0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0		0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0		0.0
Detector 2 Position(ft)		94			94			94				94
Detector 2 Size(ft)		6			6			6				6
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				0.0

Lanes, Volumes, Timings
 1011: Schaefer Hwy & Grand River Ave

07/07/2022

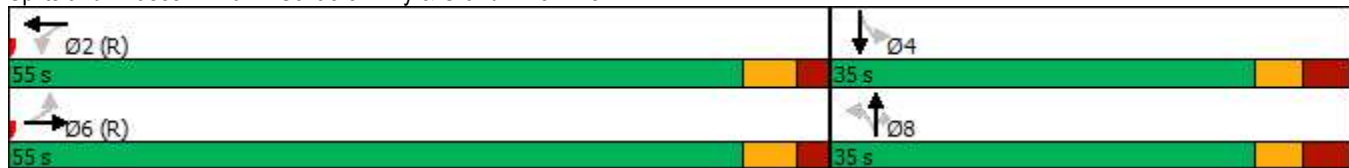


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA		Perm	NA		Perm	NA	Perm	Perm	NA	
Protected Phases		6			2			8				4
Permitted Phases	6			2			8		8	4		
Detector Phase	6	6		2	2		8	8	8	4		4
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		7.0	7.0	7.0	7.0		7.0
Minimum Split (s)	35.8	35.8		35.8	35.8		46.5	46.5	46.5	46.5		46.5
Total Split (s)	55.0	55.0		55.0	55.0		35.0	35.0	35.0	35.0		35.0
Total Split (%)	61.1%	61.1%		61.1%	61.1%		38.9%	38.9%	38.9%	38.9%		38.9%
Maximum Green (s)	49.2	49.2		49.2	49.2		28.5	28.5	28.5	28.5		28.5
Yellow Time (s)	3.6	3.6		3.6	3.6		3.2	3.2	3.2	3.2		3.2
All-Red Time (s)	2.2	2.2		2.2	2.2		3.3	3.3	3.3	3.3		3.3
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0		0.0
Total Lost Time (s)	5.8	5.8		5.8	5.8		6.5	6.5	6.5	6.5		6.5
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0		3.0
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None	None	None		None
Walk Time (s)	7.0	7.0		7.0	7.0		10.0	10.0	10.0	10.0		10.0
Flash Dont Walk (s)	23.0	23.0		23.0	23.0		30.0	30.0	30.0	30.0		30.0
Pedestrian Calls (#/hr)	0	0		0	0		0	0	0	0		0
Act Effct Green (s)	54.5	54.5		54.5	54.5		23.2	23.2	23.2	23.2		23.2
Actuated g/C Ratio	0.61	0.61		0.61	0.61		0.26	0.26	0.26	0.26		0.26
v/c Ratio	0.20	0.20		0.08	0.24		0.43	0.79	0.04	0.69		0.47
Control Delay	11.0	7.4		8.9	6.8		26.0	36.4	6.6	58.6		28.6
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.7	0.0	0.0		0.0
Total Delay	11.0	7.4		8.9	6.8		26.0	37.1	6.6	58.6		28.6
LOS	B	A		A	A		C	D	A	E		C
Approach Delay		7.8			6.9			33.9				33.7
Approach LOS		A			A			C				C

Intersection Summary

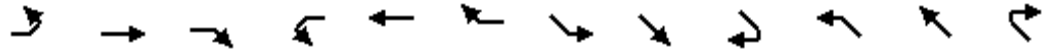
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 59 (66%), Referenced to phase 2:WBTL and 6:EBTL, Start of Green
 Natural Cycle: 85
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.79
 Intersection Signal Delay: 18.0
 Intersection LOS: B
 Intersection Capacity Utilization 78.2%
 ICU Level of Service D
 Analysis Period (min) 15

Splits and Phases: 1011: Schaefer Hwy & Grand River Ave



Lanes, Volumes, Timings
 1093: Grand River Ave & Jeffries Service Dr

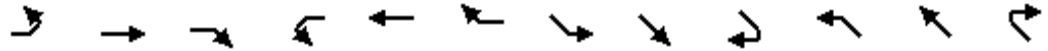
07/07/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↑↑↑↑						↑↑↑↑			↑↑↑↑	
Traffic Volume (vph)	6	311	58	0	0	0	5	462	0	0	419	20
Future Volume (vph)	6	311	58	0	0	0	5	462	0	0	419	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.86	0.86	0.86	1.00	1.00	1.00	0.91	0.91	1.00	1.00	0.91	0.91
Ped Bike Factor								1.00			1.00	
Frt		0.977									0.990	
Flt Protected		0.999						0.999				
Satd. Flow (prot)	0	6254	0	0	0	0	0	5082	0	0	5066	0
Flt Permitted		0.999						0.933				
Satd. Flow (perm)	0	6254	0	0	0	0	0	4746	0	0	5066	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		54									19	
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		329			379			582			812	
Travel Time (s)		6.4			7.4			11.3			15.8	
Confl. Peds. (#/hr)							1					1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.60	0.91	0.92	0.92	0.92	0.60
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	0%	2%	2%	2%	1%	5%
Adj. Flow (vph)	7	338	63	0	0	0	8	508	0	0	455	33
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	408	0	0	0	0	0	516	0	0	488	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2					1	2			2	
Detector Template	Left	Thru					Left	Thru			Thru	
Leading Detector (ft)	20	100					20	100			100	
Trailing Detector (ft)	0	0					0	0			0	
Detector 1 Position(ft)	0	0					0	0			0	
Detector 1 Size(ft)	20	6					20	6			6	
Detector 1 Type	Cl+Ex	Cl+Ex					Cl+Ex	Cl+Ex			Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0			0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0			0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0			0.0	
Detector 2 Position(ft)		94						94			94	
Detector 2 Size(ft)		6						6			6	
Detector 2 Type		Cl+Ex						Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA			NA	
Protected Phases		4						6			2	
Permitted Phases	4						6					

Lanes, Volumes, Timings
 1093: Grand River Ave & Jeffries Service Dr

07/07/2022

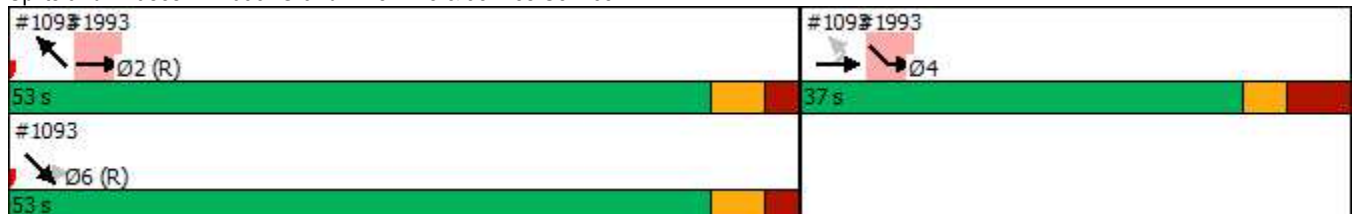


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Detector Phase	4	4					6	6				2
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0				10.0
Minimum Split (s)	57.4	57.4					37.0	37.0				37.0
Total Split (s)	37.0	37.0					53.0	53.0				53.0
Total Split (%)	41.1%	41.1%					58.9%	58.9%				58.9%
Maximum Green (s)	29.6	29.6					47.0	47.0				47.0
Yellow Time (s)	3.0	3.0					3.6	3.6				3.6
All-Red Time (s)	4.4	4.4					2.4	2.4				2.4
Lost Time Adjust (s)		0.0						0.0				0.0
Total Lost Time (s)		7.4						6.0				6.0
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0				3.0
Recall Mode	None	None					C-Max	C-Max				C-Max
Walk Time (s)	9.0	9.0					7.0	7.0				7.0
Flash Dont Walk (s)	33.0	33.0					24.0	24.0				24.0
Pedestrian Calls (#/hr)	0	0					0	0				0
Act Effct Green (s)		11.7						64.9				64.9
Actuated g/C Ratio		0.13						0.72				0.72
v/c Ratio		0.47						0.15				0.13
Control Delay		31.9						2.3				4.0
Queue Delay		0.0						0.0				0.0
Total Delay		31.9						2.3				4.0
LOS		C						A				A
Approach Delay		31.9						2.3				4.0
Approach LOS		C						A				A

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 54 (60%), Referenced to phase 2:NWT and 6:SETL, Start of Green
 Natural Cycle: 95
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.47
 Intersection Signal Delay: 11.4
 Intersection LOS: B
 Intersection Capacity Utilization 45.3%
 ICU Level of Service A
 Analysis Period (min) 15

Splits and Phases: 1093: Grand River Ave & Jeffries Service Dr



Lanes, Volumes, Timings

1193: Left Turn Bridge & Grand River Ave & I-96 WB Service Rd/I-96 WB Service Dr 07/07/2022



Lane Group	WBL2	WBL	WBT	WBR	SET	SER	SER2	NWL	NWT
Lane Configurations			↑↑↑	↑	↑↑↑	↑		↑	↑↑↑
Traffic Volume (vph)	42	5	193	315	425	107	6	61	365
Future Volume (vph)	42	5	193	315	425	107	6	61	365
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0		0		140		260	
Storage Lanes		0		1		1		1	
Taper Length (ft)		65						65	
Lane Util. Factor	0.91	0.91	0.86	0.86	0.86	0.86	0.91	1.00	0.91
Frt			0.941	0.850	0.994	0.850			
Flt Protected			0.994					0.950	
Satd. Flow (prot)	0	0	4495	1362	4770	1316	0	1805	5085
Flt Permitted			0.994					0.950	
Satd. Flow (perm)	0	0	4495	1362	4770	1316	0	1805	5085
Right Turn on Red				Yes			Yes		
Satd. Flow (RTOR)			171	171		126			
Link Speed (mph)			35		35				35
Link Distance (ft)			675		427				582
Travel Time (s)			13.1		8.3				11.3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.93	0.74	0.60	0.78	0.89
Heavy Vehicles (%)	2%	2%	2%	2%	2%	6%	0%	0%	2%
Adj. Flow (vph)	46	5	210	342	457	145	10	78	410
Shared Lane Traffic (%)				50%		13%			
Lane Group Flow (vph)	0	0	432	171	476	136	0	78	410
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right	Right	Left	Left
Median Width(ft)			0		12				12
Link Offset(ft)			0		0				0
Crosswalk Width(ft)			16		16				16
Two way Left Turn Lane									
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	15		9		9	9	15	
Number of Detectors	1	1	2	1	2	1		1	2
Detector Template	Left	Left	Thru	Right	Thru	Right		Left	Thru
Leading Detector (ft)	20	20	100	20	100	20		20	100
Trailing Detector (ft)	0	0	0	0	0	0		0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0
Detector 1 Size(ft)	20	20	6	20	6	20		20	6
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex
Detector 1 Channel									
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
Detector 2 Position(ft)			94		94				94
Detector 2 Size(ft)			6		6				6
Detector 2 Type			Cl+Ex		Cl+Ex				Cl+Ex
Detector 2 Channel									
Detector 2 Extend (s)			0.0		0.0				0.0
Turn Type	Perm	Perm	NA	Perm	NA	Perm		Prot	NA
Protected Phases			4		6			5	2

Lanes, Volumes, Timings

1193: Left Turn Bridge & Grand River Ave & I-96 WB Service Rd/I-96 WB Service Dr 07/07/2022

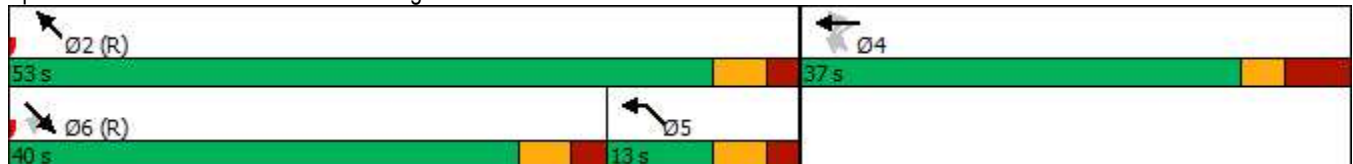


Lane Group	WBL2	WBL	WBT	WBR	SET	SER	SER2	NWL	NWT
Permitted Phases	4	4		4		6			
Detector Phase	4	4	4	4	6	6		5	2
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0		7.0	10.0
Minimum Split (s)	55.5	55.5	55.5	55.5	37.9	37.9		12.9	37.9
Total Split (s)	37.0	37.0	37.0	37.0	40.0	40.0		13.0	53.0
Total Split (%)	41.1%	41.1%	41.1%	41.1%	44.4%	44.4%		14.4%	58.9%
Maximum Green (s)	29.5	29.5	29.5	29.5	34.1	34.1		7.1	47.1
Yellow Time (s)	3.0	3.0	3.0	3.0	3.6	3.6		3.6	3.6
All-Red Time (s)	4.5	4.5	4.5	4.5	2.3	2.3		2.3	2.3
Lost Time Adjust (s)			0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)			7.5	7.5	5.9	5.9		5.9	5.9
Lead/Lag					Lead	Lead		Lag	
Lead-Lag Optimize?					Yes	Yes		Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max		None	C-Max
Walk Time (s)	9.0	9.0	9.0	9.0	7.0	7.0			7.0
Flash Dont Walk (s)	39.0	39.0	39.0	39.0	25.0	25.0			25.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0			0
Act Effct Green (s)			12.5	12.5	53.7	53.7		7.1	64.1
Actuated g/C Ratio			0.14	0.14	0.60	0.60		0.08	0.71
v/c Ratio			0.56	0.51	0.17	0.16		0.55	0.11
Control Delay			24.0	11.1	7.0	1.2		52.9	1.2
Queue Delay			0.0	0.0	0.0	0.0		0.0	0.0
Total Delay			24.0	11.1	7.0	1.2		52.9	1.2
LOS			C	B	A	A		D	A
Approach Delay			20.3		5.7				9.5
Approach LOS			C		A				A

Intersection Summary

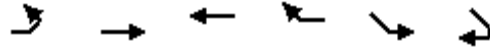
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 66 (73%), Referenced to phase 2:NWT and 6:SET, Start of Green
 Natural Cycle: 110
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.56
 Intersection Signal Delay: 12.0 Intersection LOS: B
 Intersection Capacity Utilization 39.3% ICU Level of Service A
 Analysis Period (min) 15

Splits and Phases: 1193: Left Turn Bridge & Grand River Ave & I-96 WB Service Rd/I-96 WB Service Dr



Lanes, Volumes, Timings
 1993: Jeffries Service Dr & Left Turn Bridge

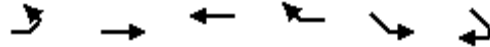
07/07/2022



Lane Group	EBL	EBT	WBT	WBR	SEL	SER	Ø6
Lane Configurations		↑↑↑			↑↑		
Traffic Volume (vph)	0	263	0	0	112	0	
Future Volume (vph)	0	263	0	0	112	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (ft)	0			0	50	0	
Storage Lanes	0			0	0	0	
Taper Length (ft)	65				65		
Lane Util. Factor	1.00	0.91	1.00	1.00	0.97	1.00	
Frt							
Flt Protected					0.950		
Satd. Flow (prot)	0	5085	0	0	3433	0	
Flt Permitted					0.950		
Satd. Flow (perm)	0	5085	0	0	3433	0	
Right Turn on Red				Yes	Yes	Yes	
Satd. Flow (RTOR)					1048		
Link Speed (mph)		35	35		30		
Link Distance (ft)		184	329		419		
Travel Time (s)		3.6	6.4		9.5		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	286	0	0	122	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	286	0	0	122	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		0	0		24		
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		16	16		16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15			9	15	9	
Number of Detectors		2			1		
Detector Template		Thru			Left		
Leading Detector (ft)		100			20		
Trailing Detector (ft)		0			0		
Detector 1 Position(ft)		0			0		
Detector 1 Size(ft)		6			20		
Detector 1 Type		Cl+Ex			Cl+Ex		
Detector 1 Channel							
Detector 1 Extend (s)		0.0			0.0		
Detector 1 Queue (s)		0.0			0.0		
Detector 1 Delay (s)		0.0			0.0		
Detector 2 Position(ft)		94					
Detector 2 Size(ft)		6					
Detector 2 Type		Cl+Ex					
Detector 2 Channel							
Detector 2 Extend (s)		0.0					
Turn Type		NA			Prot		
Protected Phases		2			4		6
Permitted Phases							

Lanes, Volumes, Timings
 1993: Jeffries Service Dr & Left Turn Bridge

07/07/2022

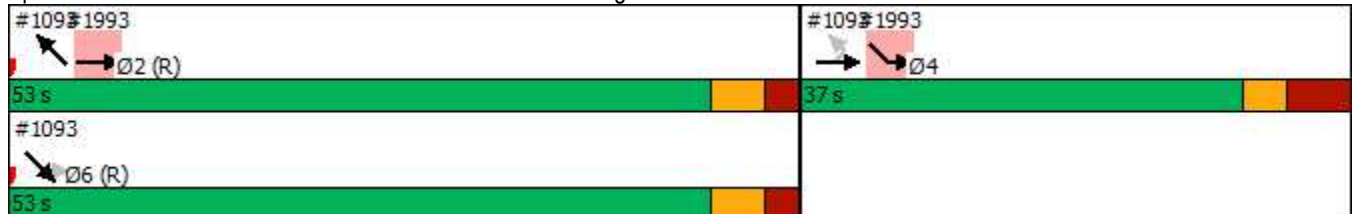


Lane Group	EBL	EBT	WBT	WBR	SEL	SER	Ø6
Detector Phase		2			4		
Switch Phase							
Minimum Initial (s)		10.0			10.0		10.0
Minimum Split (s)		37.0			57.4		37.0
Total Split (s)		53.0			37.0		53.0
Total Split (%)		58.9%			41.1%		59%
Maximum Green (s)		47.0			29.6		47.0
Yellow Time (s)		3.6			3.0		3.6
All-Red Time (s)		2.4			4.4		2.4
Lost Time Adjust (s)		0.0			0.0		
Total Lost Time (s)		6.0			7.4		
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)		3.0			3.0		3.0
Recall Mode		C-Max			None		C-Max
Walk Time (s)		7.0			9.0		7.0
Flash Dont Walk (s)		24.0			33.0		24.0
Pedestrian Calls (#/hr)		0			0		0
Act Effct Green (s)		64.9			11.7		
Actuated g/C Ratio		0.72			0.13		
v/c Ratio		0.08			0.09		
Control Delay		3.5			0.1		
Queue Delay		0.0			0.0		
Total Delay		3.5			0.1		
LOS		A			A		
Approach Delay		3.5			0.1		
Approach LOS		A			A		

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	54 (60%), Referenced to phase 2:NWT and 6:SETL, Start of Green
Natural Cycle:	95
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.47
Intersection Signal Delay:	2.5
Intersection LOS:	A
Intersection Capacity Utilization:	27.8%
ICU Level of Service:	A
Analysis Period (min):	15

Splits and Phases: 1993: Jeffries Service Dr & Left Turn Bridge



Lanes, Volumes, Timings
2609: Schaefer Hwy & Schoolcraft Rd

07/07/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	40	93	14	49	80	43	17	455	57	30	388	32
Future Volume (vph)	40	93	14	49	80	43	17	455	57	30	388	32
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	60		0	60		0	80		0	80		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor								1.00		1.00		
Fr _t		0.973			0.951			0.981				0.985
Fl _t Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1834	0	1656	1795	0	1805	1799	0	1752	1761	0
Fl _t Permitted	0.651			0.674			0.354			0.275		
Satd. Flow (perm)	1237	1834	0	1175	1795	0	673	1799	0	506	1761	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		13			29			11				9
Link Speed (mph)		30			30			30				30
Link Distance (ft)		699			1470			1209				234
Travel Time (s)		15.9			33.4			27.5				5.3
Confl. Peds. (#/hr)									4	4		
Peak Hour Factor	0.63	0.88	0.60	0.84	0.71	0.79	0.60	0.94	0.81	0.66	0.91	0.68
Heavy Vehicles (%)	0%	1%	0%	9%	1%	0%	0%	3%	5%	3%	7%	0%
Adj. Flow (vph)	63	106	23	58	113	54	28	484	70	45	426	47
Shared Lane Traffic (%)												
Lane Group Flow (vph)	63	129	0	58	167	0	28	554	0	45	473	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	

Lanes, Volumes, Timings
2609: Schaefer Hwy & Schoolcraft Rd

07/07/2022

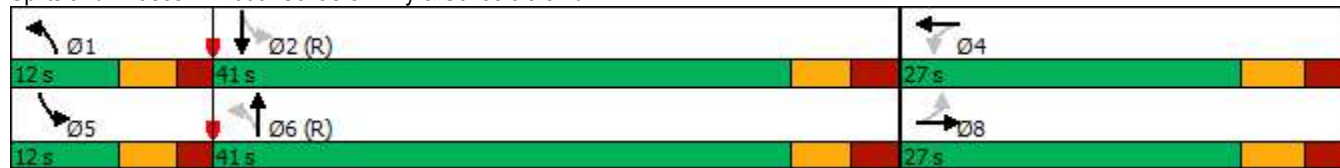


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		
Detector Phase	8	8		4	4		1	6		5	2	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0		5.0	9.0		5.0	9.0	
Minimum Split (s)	28.8	28.8		28.8	28.8		10.6	26.6		10.6	26.6	
Total Split (s)	27.0	27.0		27.0	27.0		12.0	41.0		12.0	41.0	
Total Split (%)	33.8%	33.8%		33.8%	33.8%		15.0%	51.3%		15.0%	51.3%	
Maximum Green (s)	20.2	20.2		20.2	20.2		6.4	34.4		6.4	34.4	
Yellow Time (s)	3.8	3.8		3.8	3.8		3.5	3.6		3.5	3.6	
All-Red Time (s)	3.0	3.0		3.0	3.0		2.1	3.0		2.1	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.8	6.8		6.8	6.8		5.6	6.6		5.6	6.6	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	Max	Max		Max	Max		Max	C-Max		Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0			13.0			13.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	20.2	20.2		20.2	20.2		41.8	34.4		41.8	34.4	
Actuated g/C Ratio	0.25	0.25		0.25	0.25		0.52	0.43		0.52	0.43	
v/c Ratio	0.20	0.27		0.20	0.35		0.06	0.71		0.12	0.62	
Control Delay	25.7	23.4		25.7	22.6		7.4	24.5		6.5	23.2	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	25.7	23.4		25.7	22.6		7.4	24.5		6.5	23.2	
LOS	C	C		C	C		A	C		A	C	
Approach Delay		24.1			23.4			23.7			21.7	
Approach LOS		C			C			C			C	

Intersection Summary

Area Type: Other
 Cycle Length: 80
 Actuated Cycle Length: 80
 Offset: 0 (0%), Referenced to phase 2:SBTL and 6:NBTL, Start of Green
 Natural Cycle: 70
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.71
 Intersection Signal Delay: 23.0
 Intersection LOS: C
 Intersection Capacity Utilization 56.1%
 ICU Level of Service B
 Analysis Period (min) 15

Splits and Phases: 2609: Schaefer Hwy & Schoolcraft Rd



Lanes, Volumes, Timings
3011: Schaefer Hwy & I-96 WB Service Rd

07/07/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	0	0	128	129	3	191	419	0	0	272	208
Future Volume (vph)	0	0	0	128	129	3	191	419	0	0	272	208
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Lane Width (ft)	12	12	12	12	11	12	11	11	12	12	11	12
Storage Length (ft)	0		0	0		0	250		0	0		0
Storage Lanes	0		0	1		0	1		0	0		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95
Ped Bike Factor							1.00				0.99	
Frt					0.997						0.934	
Flt Protected				0.950			0.950					
Satd. Flow (prot)	0	0	0	1863	3591	0	1766	3637	0	0	3255	0
Flt Permitted				0.950			0.450					
Satd. Flow (perm)	0	0	0	1863	3591	0	834	3637	0	0	3255	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					2						224	
Link Speed (mph)		25			25			25			35	
Link Distance (ft)		298			364			384			332	
Travel Time (s)		8.1			9.9			10.5			6.5	
Confl. Peds. (#/hr)							4					4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.83	0.91	0.92	0.92	0.95	0.93
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	4%	1%	2%	2%	5%	4%
Adj. Flow (vph)	0	0	0	139	140	3	230	460	0	0	286	224
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	139	143	0	230	460	0	0	510	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	0.94	0.94	0.94	0.94	0.98	0.94	0.98	0.98	0.94	0.94	0.98	0.94
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors				1	2		1	2				2
Detector Template				Left	Thru		Left	Thru				Thru
Leading Detector (ft)				20	100		20	100				100
Trailing Detector (ft)				0	0		0	0				0
Detector 1 Position(ft)				0	0		0	0				0
Detector 1 Size(ft)				20	6		20	6				6
Detector 1 Type				Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex				Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)				0.0	0.0		0.0	0.0				0.0
Detector 1 Queue (s)				0.0	0.0		0.0	0.0				0.0
Detector 1 Delay (s)				0.0	0.0		0.0	0.0				0.0
Detector 2 Position(ft)					94			94				94
Detector 2 Size(ft)					6			6				6
Detector 2 Type					Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	

Lanes, Volumes, Timings
 3011: Schaefer Hwy & I-96 WB Service Rd

07/07/2022

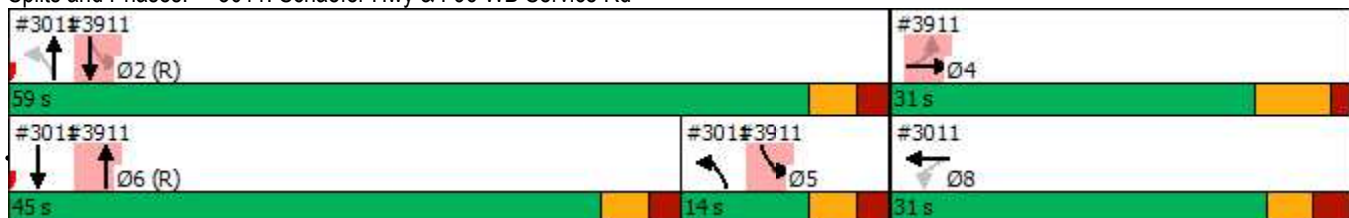


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)					0.0			0.0			0.0	
Turn Type				Perm	NA		pm+pt	NA			NA	
Protected Phases					8		5	2				6
Permitted Phases				8			2					
Detector Phase				8	8		5	2				6
Switch Phase												
Minimum Initial (s)				10.0	10.0		7.0	10.0				10.0
Minimum Split (s)				28.7	28.7		12.4	23.4				23.4
Total Split (s)				31.0	31.0		14.0	59.0				45.0
Total Split (%)				34.4%	34.4%		15.6%	65.6%				50.0%
Maximum Green (s)				25.3	25.3		8.6	53.6				39.6
Yellow Time (s)				3.0	3.0		3.2	3.2				3.2
All-Red Time (s)				2.7	2.7		2.2	2.2				2.2
Lost Time Adjust (s)				0.0	0.0		0.0	0.0				0.0
Total Lost Time (s)				5.7	5.7		5.4	5.4				5.4
Lead/Lag							Lag					Lead
Lead-Lag Optimize?							Yes					Yes
Vehicle Extension (s)				3.0	3.0		3.0	3.0				3.0
Recall Mode				None	None		None	C-Max				C-Max
Walk Time (s)				7.0	7.0			7.0				7.0
Flash Dont Walk (s)				16.0	16.0			11.0				11.0
Pedestrian Calls (#/hr)				0	0			0				0
Act Effct Green (s)				15.5	15.5		63.4	63.4				49.4
Actuated g/C Ratio				0.17	0.17		0.70	0.70				0.55
v/c Ratio				0.43	0.23		0.34	0.18				0.27
Control Delay				30.4	24.9		5.1	2.8				10.9
Queue Delay				0.0	0.0		0.0	0.0				0.0
Total Delay				30.4	24.9		5.1	2.8				10.9
LOS				C	C		A	A				B
Approach Delay					27.6			3.5				10.9
Approach LOS					C			A				B

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 87 (97%), Referenced to phase 2:NBTL and 6:SBT, Start of Green
 Natural Cycle: 70
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.66
 Intersection Signal Delay: 10.6
 Intersection LOS: B
 Intersection Capacity Utilization 47.2%
 ICU Level of Service A
 Analysis Period (min) 15

Splits and Phases: 3011: Schaefer Hwy & I-96 WB Service Rd



Lane Group	Ø4
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
Minimum Split (s)	30.5
Total Split (s)	31.0
Total Split (%)	34%
Maximum Green (s)	24.5
Yellow Time (s)	5.0
All-Red Time (s)	1.5
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	17.0
Pedestrian Calls (#/hr)	0
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Lanes, Volumes, Timings
3911: Schaefer Hwy & Jeffries Service Dr

07/07/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕↕↕						↕↕		↕	↕↕	
Traffic Volume (vph)	212	110	245	0	0	0	0	399	100	53	347	0
Future Volume (vph)	212	110	245	0	0	0	0	399	100	53	347	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	0		0	90		0
Storage Lanes	0		0	0		0	0		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	0.91	0.91	0.91	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00
Ped Bike Factor								1.00		1.00		
Fr _t		0.935						0.969				
Fl _t Protected		0.982								0.950		
Satd. Flow (prot)	0	4669	0	0	0	0	0	3334	0	1671	3471	0
Fl _t Permitted		0.982								0.400		
Satd. Flow (perm)	0	4669	0	0	0	0	0	3334	0	701	3471	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		209						47				
Link Speed (mph)		35			35			30				30
Link Distance (ft)		417			372			291				384
Travel Time (s)		8.1			7.2			6.6				8.7
Confl. Peds. (#/hr)									6	6		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.84	0.80	0.80	0.92	0.92
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	3%	10%	8%	4%	2%
Adj. Flow (vph)	230	120	266	0	0	0	0	475	125	66	377	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	616	0	0	0	0	0	600	0	66	377	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2						2		1	2	
Detector Template	Left	Thru						Thru		Left	Thru	
Leading Detector (ft)	20	100						100		20	100	
Trailing Detector (ft)	0	0						0		0	0	
Detector 1 Position(ft)	0	0						0		0	0	
Detector 1 Size(ft)	20	6						6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex						Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0						0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0						0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0						0.0		0.0	0.0	
Detector 2 Position(ft)		94						94			94	
Detector 2 Size(ft)		6						6			6	
Detector 2 Type		Cl+Ex						Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	

Lane Group	Ø8
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	

Lanes, Volumes, Timings
 3911: Schaefer Hwy & Jeffries Service Dr

07/07/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA						NA		pm+pt	NA	
Protected Phases		4						6		5	2	
Permitted Phases	4									2		
Detector Phase	4	4						6		5	2	
Switch Phase												
Minimum Initial (s)	10.0	10.0						10.0		7.0	10.0	
Minimum Split (s)	30.5	30.5						23.4		12.4	23.4	
Total Split (s)	31.0	31.0						45.0		14.0	59.0	
Total Split (%)	34.4%	34.4%						50.0%		15.6%	65.6%	
Maximum Green (s)	24.5	24.5						39.6		8.6	53.6	
Yellow Time (s)	5.0	5.0						3.2		3.2	3.2	
All-Red Time (s)	1.5	1.5						2.2		2.2	2.2	
Lost Time Adjust (s)		0.0						0.0		0.0	0.0	
Total Lost Time (s)		6.5						5.4		5.4	5.4	
Lead/Lag								Lead		Lag		
Lead-Lag Optimize?								Yes		Yes		
Vehicle Extension (s)	3.0	3.0						3.0		3.0	3.0	
Recall Mode	None	None						C-Max		None	C-Max	
Walk Time (s)	7.0	7.0						7.0			7.0	
Flash Dont Walk (s)	17.0	17.0						11.0			11.0	
Pedestrian Calls (#/hr)	0	0						0			0	
Act Effct Green (s)		14.7						49.4		63.4	63.4	
Actuated g/C Ratio		0.16						0.55		0.70	0.70	
v/c Ratio		0.66						0.32		0.11	0.15	
Control Delay		26.2						11.3		5.9	5.3	
Queue Delay		0.0						0.0		0.0	0.0	
Total Delay		26.2						11.3		5.9	5.3	
LOS		C						B		A	A	
Approach Delay		26.2						11.3			5.4	
Approach LOS		C						B			A	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	87 (97%), Referenced to phase 2:NBTL and 6:SBT, Start of Green
Natural Cycle:	70
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.66
Intersection Signal Delay:	15.2
Intersection LOS:	B
Intersection Capacity Utilization:	47.2%
ICU Level of Service:	A
Analysis Period (min):	15

Splits and Phases: 3911: Schaefer Hwy & Jeffries Service Dr



Lane Group	Ø8
Turn Type	
Protected Phases	8
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
Minimum Split (s)	28.7
Total Split (s)	31.0
Total Split (%)	34%
Maximum Green (s)	25.3
Yellow Time (s)	3.0
All-Red Time (s)	2.7
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	16.0
Pedestrian Calls (#/hr)	0
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Lanes, Volumes, Timings
4001: Schaefer Hwy & Lyndon St

07/07/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	23	120	28	56	81	27	33	455	49	47	365	35
Future Volume (vph)	23	120	28	56	81	27	33	455	49	47	365	35
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	80		0	80		0	80		0	80		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor		1.00		1.00			0.99	1.00		1.00	1.00	
Fr _t		0.972			0.963			0.983			0.983	
Fl _t Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1785	0	1656	1812	0	1752	3506	0	1770	3433	0
Fl _t Permitted	0.630			0.521			0.483			0.402		
Satd. Flow (perm)	1197	1785	0	907	1812	0	886	3506	0	746	3433	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		14			19			23			23	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		648			545			2255			685	
Travel Time (s)		14.7			12.4			51.3			15.6	
Confl. Peds. (#/hr)			2	2			6		5	5		6
Peak Hour Factor	0.69	0.76	0.75	0.75	0.70	0.72	0.67	0.88	0.73	0.75	0.88	0.65
Heavy Vehicles (%)	0%	3%	4%	9%	0%	4%	3%	1%	0%	2%	3%	3%
Adj. Flow (vph)	33	158	37	75	116	38	49	517	67	63	415	54
Shared Lane Traffic (%)												
Lane Group Flow (vph)	33	195	0	75	154	0	49	584	0	63	469	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	

Lanes, Volumes, Timings
4001: Schaefer Hwy & Lyndon St

07/07/2022

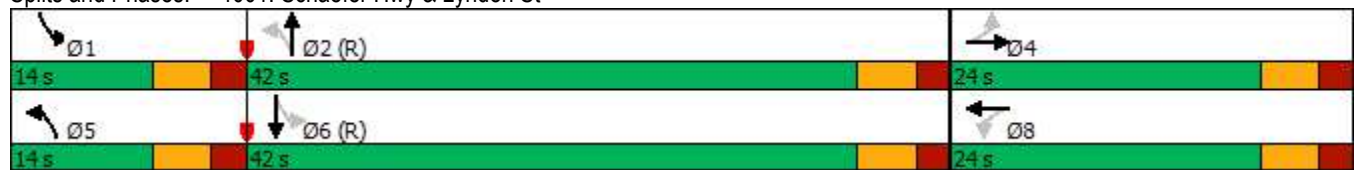


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		4.0	15.0		4.0	15.0	
Minimum Split (s)	24.6	24.6		24.6	24.6		9.6	26.6		9.6	26.6	
Total Split (s)	24.0	24.0		24.0	24.0		14.0	42.0		14.0	42.0	
Total Split (%)	30.0%	30.0%		30.0%	30.0%		17.5%	52.5%		17.5%	52.5%	
Maximum Green (s)	18.4	18.4		18.4	18.4		8.4	36.4		8.4	36.4	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	2.1	2.1		2.1	2.1		2.1	2.1		2.1	2.1	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.6	5.6		5.6	5.6		5.6	5.6		5.6	5.6	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	Min	Min		Min	Min		None	C-Max		None	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	12.0	12.0		12.0	12.0			14.0			14.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	13.7	13.7		13.7	13.7		50.4	45.2		51.9	47.7	
Actuated g/C Ratio	0.17	0.17		0.17	0.17		0.63	0.56		0.65	0.60	
v/c Ratio	0.16	0.62		0.48	0.47		0.08	0.29		0.11	0.23	
Control Delay	28.5	36.6		39.9	30.2		7.2	17.8		5.4	9.2	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	28.5	36.6		39.9	30.2		7.2	17.8		5.4	9.2	
LOS	C	D		D	C		A	B		A	A	
Approach Delay		35.4			33.4			17.0			8.7	
Approach LOS		D			C			B			A	

Intersection Summary

Area Type: Other
 Cycle Length: 80
 Actuated Cycle Length: 80
 Offset: 14 (18%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 65
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.62
 Intersection Signal Delay: 19.2
 Intersection LOS: B
 Intersection Capacity Utilization 56.7%
 ICU Level of Service B
 Analysis Period (min) 15

Splits and Phases: 4001: Schaefer Hwy & Lyndon St



Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	5	173	2	1	164	4	1	0	6	5	0	6
Future Vol, veh/h	5	173	2	1	164	4	1	0	6	5	0	6
Conflicting Peds, #/hr	3	0	1	1	0	3	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	63	94	60	60	86	60	60	60	60	63	60	60
Heavy Vehicles, %	0	3	0	0	3	0	0	0	0	0	0	0
Mvmt Flow	8	184	3	2	191	7	2	0	10	8	0	10

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	201	0	0	188	0	0	407	408	187	409	406	198
Stage 1	-	-	-	-	-	-	203	203	-	202	202	-
Stage 2	-	-	-	-	-	-	204	205	-	207	204	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1383	-	-	1398	-	-	558	536	860	556	537	848
Stage 1	-	-	-	-	-	-	804	737	-	805	738	-
Stage 2	-	-	-	-	-	-	803	736	-	800	737	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1379	-	-	1397	-	-	547	530	859	545	531	846
Mov Cap-2 Maneuver	-	-	-	-	-	-	547	530	-	545	531	-
Stage 1	-	-	-	-	-	-	798	732	-	798	734	-
Stage 2	-	-	-	-	-	-	792	732	-	786	732	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0.1			9.6			10.4		
HCM LOS							A			B		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	794	1379	-	-	1397	-	-	680
HCM Lane V/C Ratio	0.015	0.006	-	-	0.001	-	-	0.026
HCM Control Delay (s)	9.6	7.6	0	-	7.6	0	-	10.4
HCM Lane LOS	A	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0.1

Intersection												
Int Delay, s/veh	1.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	3	1	8	7	6	7	2	302	10	7	282	1
Future Vol, veh/h	3	1	8	7	6	7	2	302	10	7	282	1
Conflicting Peds, #/hr	0	0	0	0	0	0	1	0	1	1	0	1
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	50	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	60	60	60	60	60	60	100	85	63	60	87	60
Heavy Vehicles, %	0	0	0	0	0	0	0	5	0	0	3	0
Mvmt Flow	5	2	13	12	10	12	2	355	16	12	324	2

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	728	726	326	725	719	364	327	0	0	372	0	0
Stage 1	350	350	-	368	368	-	-	-	-	-	-	-
Stage 2	378	376	-	357	351	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	341	354	720	343	357	685	1244	-	-	1198	-	-
Stage 1	671	636	-	656	625	-	-	-	-	-	-	-
Stage 2	648	620	-	665	636	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	325	349	719	332	352	684	1243	-	-	1197	-	-
Mov Cap-2 Maneuver	325	349	-	332	352	-	-	-	-	-	-	-
Stage 1	669	629	-	654	623	-	-	-	-	-	-	-
Stage 2	626	618	-	644	629	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	12.2		14.5		0		0.3	
HCM LOS	B		B					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1243	-	-	517	414	1197	-
HCM Lane V/C Ratio	0.002	-	-	0.039	0.081	0.01	-
HCM Control Delay (s)	7.9	-	-	12.2	14.5	8	-
HCM Lane LOS	A	-	-	B	B	A	-
HCM 95th %tile Q(veh)	0	-	-	0.1	0.3	0	-

Lanes, Volumes, Timings
 1011: Schaefer Hwy & Grand River Ave

07/07/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↗	↗↗↗		↗	↗↗↗		↗	↗	↗	↗	↗↗	
Traffic Volume (vph)	36	433	66	20	446	102	55	249	19	57	272	17
Future Volume (vph)	36	433	66	20	446	102	55	249	19	57	272	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	115		0	95		0	155		0	80		240
Storage Lanes	1		0	0		0	1		1	1		1
Taper Length (ft)	65			65			25			65		
Lane Util. Factor	1.00	0.91	0.91	1.00	0.91	0.91	1.00	1.00	1.00	1.00	0.95	0.95
Ped Bike Factor		1.00		1.00					0.99	1.00		
Frt		0.976			0.969				0.850		0.991	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	5009	0	1805	4956	0	1805	1900	1615	1770	3544	0
Flt Permitted	0.411			0.421			0.523			0.317		
Satd. Flow (perm)	781	5009	0	799	4956	0	994	1900	1593	590	3544	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		67			111				34			8
Link Speed (mph)		35			35			30				30
Link Distance (ft)		932			422			332				1209
Travel Time (s)		18.2			8.2			7.5				27.5
Confl. Peds. (#/hr)			2	2					2	2		
Peak Hour Factor	0.80	0.88	0.72	0.59	0.92	0.82	0.74	0.74	0.60	0.76	0.86	0.80
Heavy Vehicles (%)	0%	1%	0%	0%	0%	7%	0%	0%	0%	2%	1%	0%
Adj. Flow (vph)	45	492	92	34	485	124	74	336	32	75	316	21
Shared Lane Traffic (%)												
Lane Group Flow (vph)	45	584	0	34	609	0	74	336	32	75	337	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2	1	1		2
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right	Left		Thru
Leading Detector (ft)	20	100		20	100		20	100	20	20		100
Trailing Detector (ft)	0	0		0	0		0	0	0	0		0
Detector 1 Position(ft)	0	0		0	0		0	0	0	0		0
Detector 1 Size(ft)	20	6		20	6		20	6	20	20		6
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0		0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0		0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0		0.0
Detector 2 Position(ft)		94			94			94				94
Detector 2 Size(ft)		6			6			6				6
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				0.0

Lanes, Volumes, Timings
1011: Schaefer Hwy & Grand River Ave

07/07/2022

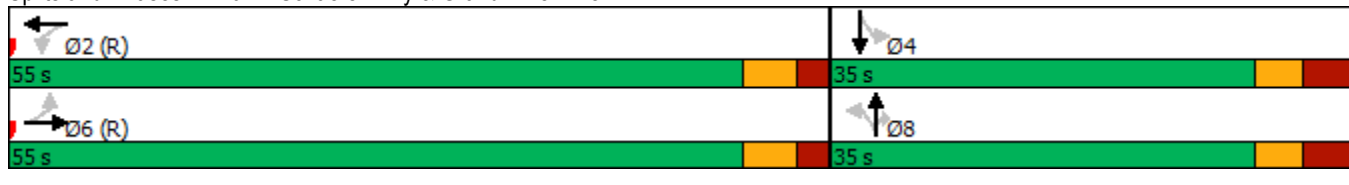


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA		Perm	NA		Perm	NA	Perm	Perm	NA	
Protected Phases		6			2			8				4
Permitted Phases	6			2			8		8	4		
Detector Phase	6	6		2	2		8	8	8	4		4
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		7.0	7.0	7.0	7.0		7.0
Minimum Split (s)	35.8	35.8		35.8	35.8		46.5	46.5	46.5	46.5		46.5
Total Split (s)	55.0	55.0		55.0	55.0		35.0	35.0	35.0	35.0		35.0
Total Split (%)	61.1%	61.1%		61.1%	61.1%		38.9%	38.9%	38.9%	38.9%		38.9%
Maximum Green (s)	49.2	49.2		49.2	49.2		28.5	28.5	28.5	28.5		28.5
Yellow Time (s)	3.6	3.6		3.6	3.6		3.2	3.2	3.2	3.2		3.2
All-Red Time (s)	2.2	2.2		2.2	2.2		3.3	3.3	3.3	3.3		3.3
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0		0.0
Total Lost Time (s)	5.8	5.8		5.8	5.8		6.5	6.5	6.5	6.5		6.5
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0		3.0
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None	None	None		None
Walk Time (s)	7.0	7.0		7.0	7.0		10.0	10.0	10.0	10.0		10.0
Flash Dont Walk (s)	23.0	23.0		23.0	23.0		30.0	30.0	30.0	30.0		30.0
Pedestrian Calls (#/hr)	0	0		0	0		0	0	0	0		0
Act Effct Green (s)	56.3	56.3		56.3	56.3		21.4	21.4	21.4	21.4		21.4
Actuated g/C Ratio	0.63	0.63		0.63	0.63		0.24	0.24	0.24	0.24		0.24
v/c Ratio	0.09	0.18		0.07	0.19		0.31	0.75	0.08	0.54		0.40
Control Delay	9.0	7.1		8.2	6.0		24.3	36.2	10.4	43.1		28.6
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.4	0.0	0.0		0.0
Total Delay	9.0	7.1		8.2	6.0		24.3	36.6	10.4	43.1		28.6
LOS	A	A		A	A		C	D	B	D		C
Approach Delay		7.2			6.1			32.7				31.2
Approach LOS		A			A			C				C

Intersection Summary

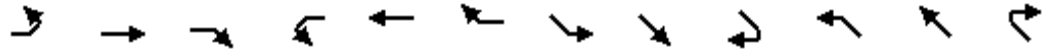
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 59 (66%), Referenced to phase 2:WBTL and 6:EBTL, Start of Green
 Natural Cycle: 85
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.75
 Intersection Signal Delay: 16.8
 Intersection LOS: B
 Intersection Capacity Utilization 65.8%
 ICU Level of Service C
 Analysis Period (min) 15

Splits and Phases: 1011: Schaefer Hwy & Grand River Ave



Lanes, Volumes, Timings
 1093: Grand River Ave & Jeffries Service Dr

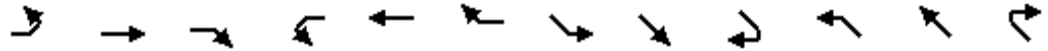
07/07/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↑↑↑↑						↑↑↑↑			↑↑↑↑	
Traffic Volume (vph)	4	273	65	0	0	0	9	420	0	0	343	14
Future Volume (vph)	4	273	65	0	0	0	9	420	0	0	343	14
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.86	0.86	0.86	1.00	1.00	1.00	0.91	0.91	1.00	1.00	0.91	0.91
Ped Bike Factor								1.00			1.00	
Frt		0.971									0.993	
Flt Protected		0.999						0.998				
Satd. Flow (prot)	0	6216	0	0	0	0	0	5127	0	0	5081	0
Flt Permitted		0.999						0.919				
Satd. Flow (perm)	0	6216	0	0	0	0	0	4721	0	0	5081	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		71									11	
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		342			423			587			812	
Travel Time (s)		6.7			8.2			11.4			15.8	
Confl. Peds. (#/hr)							3					3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.60	0.96	0.92	0.92	0.71	0.65
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	0%	1%	2%	2%	1%	8%
Adj. Flow (vph)	4	297	71	0	0	0	15	438	0	0	483	22
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	372	0	0	0	0	0	453	0	0	505	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2					1	2			2	
Detector Template	Left	Thru					Left	Thru			Thru	
Leading Detector (ft)	20	100					20	100			100	
Trailing Detector (ft)	0	0					0	0			0	
Detector 1 Position(ft)	0	0					0	0			0	
Detector 1 Size(ft)	20	6					20	6			6	
Detector 1 Type	Cl+Ex	Cl+Ex					Cl+Ex	Cl+Ex			Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0			0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0			0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0			0.0	
Detector 2 Position(ft)		94						94			94	
Detector 2 Size(ft)		6						6			6	
Detector 2 Type		Cl+Ex						Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA			NA	
Protected Phases		4						6			2	
Permitted Phases	4						6					

Lanes, Volumes, Timings
 1093: Grand River Ave & Jeffries Service Dr

07/07/2022

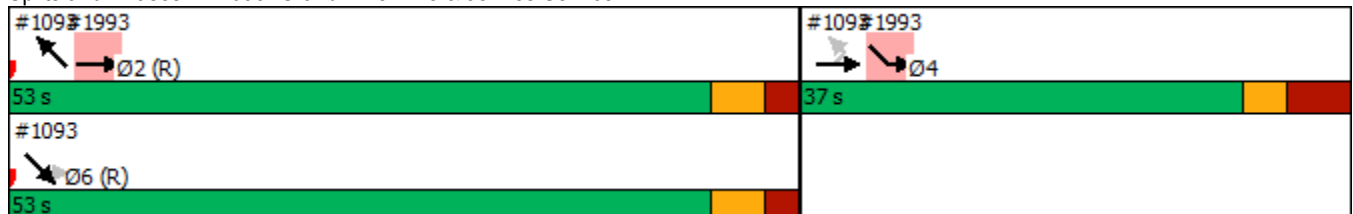


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Detector Phase	4	4					6	6			2	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0			10.0	
Minimum Split (s)	49.4	49.4					37.0	37.0			37.0	
Total Split (s)	37.0	37.0					53.0	53.0			53.0	
Total Split (%)	41.1%	41.1%					58.9%	58.9%			58.9%	
Maximum Green (s)	29.6	29.6					47.0	47.0			47.0	
Yellow Time (s)	3.0	3.0					3.6	3.6			3.6	
All-Red Time (s)	4.4	4.4					2.4	2.4			2.4	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		7.4						6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0			3.0	
Recall Mode	None	None					C-Max	C-Max			C-Max	
Walk Time (s)	9.0	9.0					7.0	7.0			7.0	
Flash Dont Walk (s)	33.0	33.0					24.0	24.0			24.0	
Pedestrian Calls (#/hr)	0	0					0	0			0	
Act Effct Green (s)		11.1						65.5			65.5	
Actuated g/C Ratio		0.12						0.73			0.73	
v/c Ratio		0.45						0.13			0.14	
Control Delay		31.3						2.2			3.8	
Queue Delay		0.0						0.0			0.0	
Total Delay		31.3						2.2			3.8	
LOS		C						A			A	
Approach Delay		31.3						2.2			3.8	
Approach LOS		C						A			A	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 54 (60%), Referenced to phase 2:NWT and 6:SETL, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.45
 Intersection Signal Delay: 11.0
 Intersection LOS: B
 Intersection Capacity Utilization 45.3%
 ICU Level of Service A
 Analysis Period (min) 15

Splits and Phases: 1093: Grand River Ave & Jeffries Service Dr



Lanes, Volumes, Timings

1193: Left Turn Bridge & Grand River Ave & I-96 Service Dr

07/07/2022



Lane Group	WBL2	WBL	WBT	WBR	SET	SER	SER2	NWL	NWT
Lane Configurations			↑↑↑	↑	↑↑↑	↑		↓	↑↑↑
Traffic Volume (vph)	24	6	195	268	406	100	3	48	300
Future Volume (vph)	24	6	195	268	406	100	3	48	300
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0		0		140		260	
Storage Lanes		0		1		1		1	
Taper Length (ft)		65						65	
Lane Util. Factor	0.91	0.91	0.86	0.86	0.86	0.86	0.91	1.00	0.91
Frt			0.944	0.850	0.996	0.850			
Flt Protected			0.996					0.950	
Satd. Flow (prot)	0	0	4519	1362	4834	1376	0	1805	5136
Flt Permitted			0.996					0.950	
Satd. Flow (perm)	0	0	4519	1362	4834	1376	0	1805	5136
Right Turn on Red				Yes			Yes		
Satd. Flow (RTOR)			146	145		126			
Link Speed (mph)			35		35				35
Link Distance (ft)			480		422				587
Travel Time (s)			9.4		8.2				11.4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.94	0.83	0.75	0.60	0.73
Heavy Vehicles (%)	2%	2%	2%	2%	1%	1%	0%	0%	1%
Adj. Flow (vph)	26	7	212	291	432	120	4	80	411
Shared Lane Traffic (%)				50%		10%			
Lane Group Flow (vph)	0	0	391	145	444	112	0	80	411
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right	Right	Left	Left
Median Width(ft)			0		12				12
Link Offset(ft)			0		0				0
Crosswalk Width(ft)			16		16				16
Two way Left Turn Lane									
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	15		9		9	9	15	
Number of Detectors	1	1	2	1	2	1		1	2
Detector Template	Left	Left	Thru	Right	Thru	Right		Left	Thru
Leading Detector (ft)	20	20	100	20	100	20		20	100
Trailing Detector (ft)	0	0	0	0	0	0		0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0
Detector 1 Size(ft)	20	20	6	20	6	20		20	6
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex
Detector 1 Channel									
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
Detector 2 Position(ft)			94		94				94
Detector 2 Size(ft)			6		6				6
Detector 2 Type			Cl+Ex		Cl+Ex				Cl+Ex
Detector 2 Channel									
Detector 2 Extend (s)			0.0		0.0				0.0
Turn Type	Perm	Perm	NA	Perm	NA	Perm		Prot	NA
Protected Phases			4		6			5	2

Lanes, Volumes, Timings

1193: Left Turn Bridge & Grand River Ave & I-96 Service Dr

07/07/2022

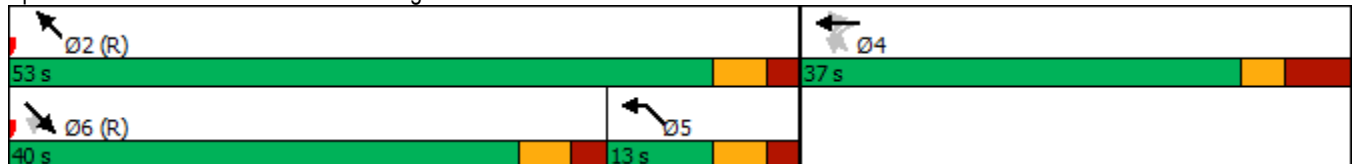


Lane Group	WBL2	WBL	WBT	WBR	SET	SER	SER2	NWL	NWT
Permitted Phases	4	4		4		6			
Detector Phase	4	4	4	4	6	6		5	2
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0		7.0	10.0
Minimum Split (s)	55.5	55.5	55.5	55.5	37.9	37.9		12.9	37.9
Total Split (s)	37.0	37.0	37.0	37.0	40.0	40.0		13.0	53.0
Total Split (%)	41.1%	41.1%	41.1%	41.1%	44.4%	44.4%		14.4%	58.9%
Maximum Green (s)	29.5	29.5	29.5	29.5	34.1	34.1		7.1	47.1
Yellow Time (s)	3.0	3.0	3.0	3.0	3.6	3.6		3.6	3.6
All-Red Time (s)	4.5	4.5	4.5	4.5	2.3	2.3		2.3	2.3
Lost Time Adjust (s)			0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)			7.5	7.5	5.9	5.9		5.9	5.9
Lead/Lag					Lead	Lead		Lag	
Lead-Lag Optimize?					Yes	Yes		Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max		None	C-Max
Walk Time (s)	9.0	9.0	9.0	9.0	7.0	7.0			7.0
Flash Dont Walk (s)	39.0	39.0	39.0	39.0	25.0	25.0			25.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0			0
Act Effct Green (s)			11.9	11.9	54.3	54.3		7.1	64.7
Actuated g/C Ratio			0.13	0.13	0.60	0.60		0.08	0.72
v/c Ratio			0.54	0.48	0.15	0.13		0.56	0.11
Control Delay			25.1	11.6	6.9	0.8		53.5	0.7
Queue Delay			0.0	0.0	0.0	0.0		0.0	0.0
Total Delay			25.1	11.6	6.9	0.8		53.5	0.7
LOS			C	B	A	A		D	A
Approach Delay			21.5		5.7				9.3
Approach LOS			C		A				A

Intersection Summary

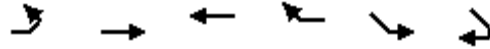
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 66 (73%), Referenced to phase 2:NWT and 6:SET, Start of Green
 Natural Cycle: 110
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.56
 Intersection Signal Delay: 12.1
 Intersection Capacity Utilization 38.9%
 Analysis Period (min) 15
 Intersection LOS: B
 ICU Level of Service A

Splits and Phases: 1193: Left Turn Bridge & Grand River Ave & I-96 Service Dr



Lanes, Volumes, Timings
 1993: Jeffries Service Dr & Left Turn Bridge

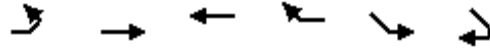
07/07/2022



Lane Group	EBL	EBT	WBT	WBR	SEL	SER	Ø6
Lane Configurations		↑↑↑			↑↑		
Traffic Volume (vph)	0	236	0	0	106	0	
Future Volume (vph)	0	236	0	0	106	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	0.91	1.00	1.00	0.97	1.00	
Frt							
Flt Protected					0.950		
Satd. Flow (prot)	0	5085	0	0	3433	0	
Flt Permitted					0.950		
Satd. Flow (perm)	0	5085	0	0	3433	0	
Right Turn on Red				Yes	Yes	Yes	
Satd. Flow (RTOR)					1127		
Link Speed (mph)		35	35		30		
Link Distance (ft)		191	342		422		
Travel Time (s)		3.7	6.7		9.6		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	257	0	0	115	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	257	0	0	115	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		0	0		24		
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		16	16		16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15			9	15	9	
Number of Detectors		2			1		
Detector Template		Thru			Left		
Leading Detector (ft)		100			20		
Trailing Detector (ft)		0			0		
Detector 1 Position(ft)		0			0		
Detector 1 Size(ft)		6			20		
Detector 1 Type		Cl+Ex			Cl+Ex		
Detector 1 Channel							
Detector 1 Extend (s)		0.0			0.0		
Detector 1 Queue (s)		0.0			0.0		
Detector 1 Delay (s)		0.0			0.0		
Detector 2 Position(ft)		94					
Detector 2 Size(ft)		6					
Detector 2 Type		Cl+Ex					
Detector 2 Channel							
Detector 2 Extend (s)		0.0					
Turn Type		NA			Prot		
Protected Phases		2			4	6	
Permitted Phases							
Detector Phase		2			4		
Switch Phase							
Minimum Initial (s)		10.0			10.0	10.0	

Lanes, Volumes, Timings
 1993: Jeffries Service Dr & Left Turn Bridge

07/07/2022



Lane Group	EBL	EBT	WBT	WBR	SEL	SER	Ø6
Minimum Split (s)		37.0			49.4		37.0
Total Split (s)		53.0			37.0		53.0
Total Split (%)		58.9%			41.1%		59%
Maximum Green (s)		47.0			29.6		47.0
Yellow Time (s)		3.6			3.0		3.6
All-Red Time (s)		2.4			4.4		2.4
Lost Time Adjust (s)		0.0			0.0		
Total Lost Time (s)		6.0			7.4		
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)		3.0			3.0		3.0
Recall Mode		C-Max			None		C-Max
Walk Time (s)		7.0			9.0		7.0
Flash Dont Walk (s)		24.0			33.0		24.0
Pedestrian Calls (#/hr)		0			0		0
Act Effct Green (s)		65.5			11.1		
Actuated g/C Ratio		0.73			0.12		
v/c Ratio		0.07			0.08		
Control Delay		2.0			0.1		
Queue Delay		0.0			0.0		
Total Delay		2.0			0.1		
LOS		A			A		
Approach Delay		2.0			0.1		
Approach LOS		A			A		

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	54 (60%), Referenced to phase 2:NWT and 6:SETL, Start of Green
Natural Cycle:	90
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.45
Intersection Signal Delay:	1.4
Intersection LOS:	A
Intersection Capacity Utilization:	27.8%
ICU Level of Service:	A
Analysis Period (min):	15

Splits and Phases: 1993: Jeffries Service Dr & Left Turn Bridge



Lanes, Volumes, Timings
2609: Schaefer Hwy & Schoolcraft Rd

07/07/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	20	89	4	24	94	33	12	330	45	36	317	26
Future Volume (vph)	20	89	4	24	94	33	12	330	45	36	317	26
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	60		0	60		0	80		0	80		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00	1.00		0.99	0.99		1.00	1.00		0.99	1.00	
Fr _t		0.993			0.964			0.978			0.984	
Fl _t Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1849	0	1805	1808	0	1805	1789	0	1805	1849	0
Fl _t Permitted	0.659			0.663			0.435			0.401		
Satd. Flow (perm)	1250	1849	0	1249	1808	0	826	1789	0	758	1849	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		3			19			14			9	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		699			1470			1209			215	
Travel Time (s)		15.9			33.4			27.5			4.9	
Confl. Peds. (#/hr)	1		6	6		1	1		6	6		1
Peak Hour Factor	0.60	0.63	0.60	0.72	0.80	0.89	0.75	0.91	0.72	0.88	0.90	0.63
Heavy Vehicles (%)	0%	2%	0%	0%	1%	0%	0%	4%	0%	0%	1%	0%
Adj. Flow (vph)	33	141	7	33	118	37	16	363	63	41	352	41
Shared Lane Traffic (%)												
Lane Group Flow (vph)	33	148	0	33	155	0	16	426	0	41	393	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	

Lanes, Volumes, Timings
 2609: Schaefer Hwy & Schoolcraft Rd

07/07/2022

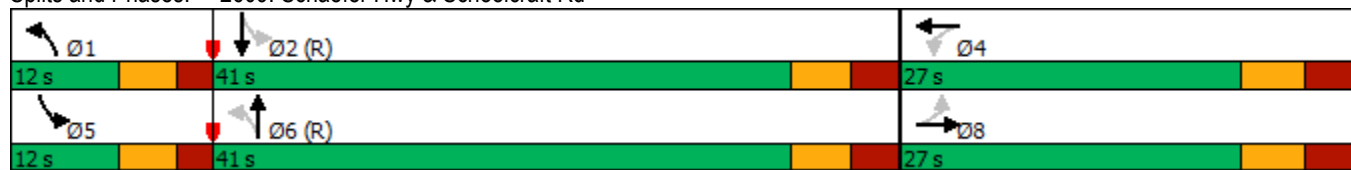


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		
Detector Phase	8	8		4	4		1	6		5	2	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0		5.0	9.0		5.0	9.0	
Minimum Split (s)	28.8	28.8		28.8	28.8		10.6	26.6		10.6	26.6	
Total Split (s)	27.0	27.0		27.0	27.0		12.0	41.0		12.0	41.0	
Total Split (%)	33.8%	33.8%		33.8%	33.8%		15.0%	51.3%		15.0%	51.3%	
Maximum Green (s)	20.2	20.2		20.2	20.2		6.4	34.4		6.4	34.4	
Yellow Time (s)	3.8	3.8		3.8	3.8		3.5	3.6		3.5	3.6	
All-Red Time (s)	3.0	3.0		3.0	3.0		2.1	3.0		2.1	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.8	6.8		6.8	6.8		5.6	6.6		5.6	6.6	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	Max	Max		Max	Max		Max	C-Max		Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0			13.0			13.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	20.2	20.2		20.2	20.2		41.8	34.4		41.8	34.4	
Actuated g/C Ratio	0.25	0.25		0.25	0.25		0.52	0.43		0.52	0.43	
v/c Ratio	0.10	0.32		0.10	0.33		0.03	0.55		0.09	0.49	
Control Delay	24.1	26.0		24.1	23.6		7.1	19.7		7.5	18.7	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	24.1	26.0		24.1	23.6		7.1	19.7		7.5	18.7	
LOS	C	C		C	C		A	B		A	B	
Approach Delay		25.7			23.7			19.2			17.6	
Approach LOS		C			C			B			B	

Intersection Summary

Area Type: Other
 Cycle Length: 80
 Actuated Cycle Length: 80
 Offset: 0 (0%), Referenced to phase 2:SBTL and 6:NBTL, Start of Green
 Natural Cycle: 70
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.55
 Intersection Signal Delay: 20.3
 Intersection LOS: C
 Intersection Capacity Utilization 58.5%
 ICU Level of Service B
 Analysis Period (min) 15

Splits and Phases: 2609: Schaefer Hwy & Schoolcraft Rd



Lanes, Volumes, Timings
3011: Schaefer Hwy & I-96 Service Dr

07/07/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				↙	↕		↙	↕			↕	
Traffic Volume (vph)	0	0	0	98	145	3	148	319	0	0	198	159
Future Volume (vph)	0	0	0	98	145	3	148	319	0	0	198	159
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	90		0	0		0
Storage Lanes	0		0	1		0	1		0	0		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95
Ped Bike Factor							1.00					0.99
Fr _t					0.997							0.932
Fl _t Protected				0.950			0.950					
Satd. Flow (prot)	0	0	0	1770	3529	0	1752	3574	0	0	3319	0
Fl _t Permitted				0.950			0.500					
Satd. Flow (perm)	0	0	0	1770	3529	0	919	3574	0	0	3319	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					2							196
Link Speed (mph)		35			35			30				30
Link Distance (ft)		298			375			384				332
Travel Time (s)		5.8			7.3			8.7				7.5
Confl. Peds. (#/hr)							4					4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.87	0.76	0.92	0.92	0.84	0.81
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	3%	1%	2%	2%	1%	0%
Adj. Flow (vph)	0	0	0	107	158	3	170	420	0	0	236	196
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	107	161	0	170	420	0	0	432	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors				1	2		1	2				2
Detector Template				Left	Thru		Left	Thru				Thru
Leading Detector (ft)				20	100		20	100				100
Trailing Detector (ft)				0	0		0	0				0
Detector 1 Position(ft)				0	0		0	0				0
Detector 1 Size(ft)				20	6		20	6				6
Detector 1 Type				Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex				Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)				0.0	0.0		0.0	0.0				0.0
Detector 1 Queue (s)				0.0	0.0		0.0	0.0				0.0
Detector 1 Delay (s)				0.0	0.0		0.0	0.0				0.0
Detector 2 Position(ft)					94			94				94
Detector 2 Size(ft)					6			6				6
Detector 2 Type					Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)					0.0			0.0				0.0

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	

Lanes, Volumes, Timings
3011: Schaefer Hwy & I-96 Service Dr

07/07/2022

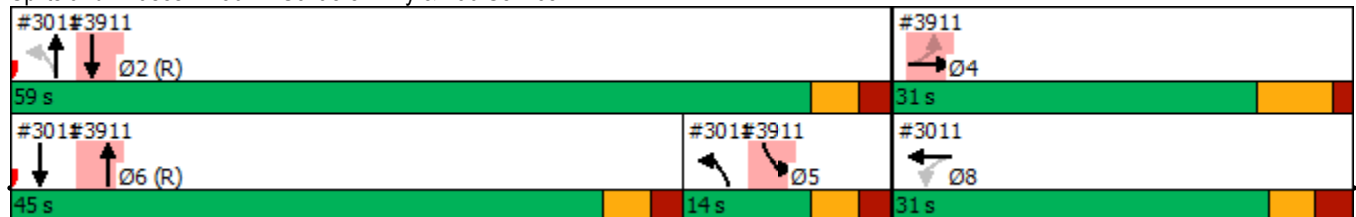


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type				Perm	NA		pm+pt	NA				NA
Protected Phases					8		5	2				6
Permitted Phases				8			2					
Detector Phase				8	8		5	2				6
Switch Phase												
Minimum Initial (s)				10.0	10.0		7.0	10.0				10.0
Minimum Split (s)				28.7	28.7		12.4	23.4				23.4
Total Split (s)				31.0	31.0		14.0	59.0				45.0
Total Split (%)				34.4%	34.4%		15.6%	65.6%				50.0%
Maximum Green (s)				25.3	25.3		8.6	53.6				39.6
Yellow Time (s)				3.0	3.0		3.2	3.2				3.2
All-Red Time (s)				2.7	2.7		2.2	2.2				2.2
Lost Time Adjust (s)				0.0	0.0		0.0	0.0				0.0
Total Lost Time (s)				5.7	5.7		5.4	5.4				5.4
Lead/Lag							Lag					Lead
Lead-Lag Optimize?							Yes					Yes
Vehicle Extension (s)				3.0	3.0		3.0	3.0				3.0
Recall Mode				None	None		None	C-Max				C-Max
Walk Time (s)				7.0	7.0			7.0				7.0
Flash Dont Walk (s)				16.0	16.0			11.0				11.0
Pedestrian Calls (#/hr)				0	0			0				0
Act Effct Green (s)				12.6	12.6		66.3	66.3				52.3
Actuated g/C Ratio				0.14	0.14		0.74	0.74				0.58
v/c Ratio				0.43	0.33		0.22	0.16				0.21
Control Delay				33.1	28.3		2.9	2.1				8.9
Queue Delay				0.0	0.0		0.0	0.0				0.0
Total Delay				33.1	28.3		2.9	2.1				8.9
LOS				C	C		A	A				A
Approach Delay					30.2			2.3				8.9
Approach LOS					C			A				A

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	87 (97%), Referenced to phase 2:NBTL and 6:SBT, Start of Green
Natural Cycle:	70
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.59
Intersection Signal Delay:	10.3
Intersection LOS:	B
Intersection Capacity Utilization:	45.3%
ICU Level of Service:	A
Analysis Period (min):	15

Splits and Phases: 3011: Schaefer Hwy & I-96 Service Dr



Lane Group	Ø4
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
Minimum Split (s)	30.5
Total Split (s)	31.0
Total Split (%)	34%
Maximum Green (s)	24.5
Yellow Time (s)	5.0
All-Red Time (s)	1.5
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	17.0
Pedestrian Calls (#/hr)	0
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Lanes, Volumes, Timings
3911: Schaefer Hwy & Jeffries Service Dr

07/07/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔↕↔						↕↔		↔	↕↕	
Traffic Volume (vph)	145	111	155	0	0	0	0	321	81	44	252	0
Future Volume (vph)	145	111	155	0	0	0	0	321	81	44	252	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	0		0	90		0
Storage Lanes	0		0	0		0	0		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	0.91	0.91	0.91	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00
Ped Bike Factor								1.00		1.00		
Fr t		0.944						0.969				
Fl t Protected		0.983								0.950		
Satd. Flow (prot)	0	4719	0	0	0	0	0	3424	0	1770	3574	0
Fl t Permitted		0.983								0.950		
Satd. Flow (perm)	0	4719	0	0	0	0	0	3424	0	1765	3574	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		168						47				
Link Speed (mph)		35			35			30				30
Link Distance (ft)		417			354			291				384
Travel Time (s)		8.1			6.9			6.6				8.7
Confl. Peds. (#/hr)									4	4		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.75	0.72	0.81	0.79	0.92
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	1%	2%	1%	2%
Adj. Flow (vph)	158	121	168	0	0	0	0	428	113	54	319	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	447	0	0	0	0	0	541	0	54	319	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane								Yes				Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2						2		1	2	
Detector Template	Left	Thru						Thru		Left	Thru	
Leading Detector (ft)	20	100						100		20	100	
Trailing Detector (ft)	0	0						0		0	0	
Detector 1 Position(ft)	0	0						0		0	0	
Detector 1 Size(ft)	20	6						6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex						Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0						0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0						0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0						0.0		0.0	0.0	
Detector 2 Position(ft)		94						94			94	
Detector 2 Size(ft)		6						6			6	
Detector 2 Type		Cl+Ex						Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	

Lane Group	Ø8
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	

Lanes, Volumes, Timings
 3911: Schaefer Hwy & Jeffries Service Dr

07/07/2022

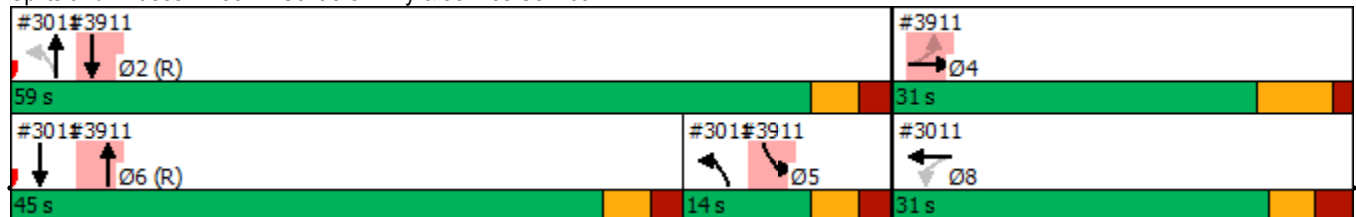


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA						NA		Prot	NA	
Protected Phases		4						6		5	2	
Permitted Phases	4											
Detector Phase	4	4						6		5	2	
Switch Phase												
Minimum Initial (s)	10.0	10.0						10.0		7.0	10.0	
Minimum Split (s)	30.5	30.5						23.4		12.4	23.4	
Total Split (s)	31.0	31.0						45.0		14.0	59.0	
Total Split (%)	34.4%	34.4%						50.0%		15.6%	65.6%	
Maximum Green (s)	24.5	24.5						39.6		8.6	53.6	
Yellow Time (s)	5.0	5.0						3.2		3.2	3.2	
All-Red Time (s)	1.5	1.5						2.2		2.2	2.2	
Lost Time Adjust (s)		0.0						0.0		0.0	0.0	
Total Lost Time (s)		6.5						5.4		5.4	5.4	
Lead/Lag								Lead		Lag		
Lead-Lag Optimize?								Yes		Yes		
Vehicle Extension (s)	3.0	3.0						3.0		3.0	3.0	
Recall Mode	None	None						C-Max		None	C-Max	
Walk Time (s)	7.0	7.0						7.0			7.0	
Flash Dont Walk (s)	17.0	17.0						11.0			11.0	
Pedestrian Calls (#/hr)	0	0						0			0	
Act Effct Green (s)		11.8						52.3		8.6	66.3	
Actuated g/C Ratio		0.13						0.58		0.10	0.74	
v/c Ratio		0.59						0.27		0.32	0.12	
Control Delay		25.6						9.2		46.0	3.9	
Queue Delay		0.0						0.0		0.0	0.0	
Total Delay		25.6						9.2		46.0	3.9	
LOS		C						A		D	A	
Approach Delay		25.6						9.2			10.0	
Approach LOS		C						A			A	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	87 (97%), Referenced to phase 2:NBTL and 6:SBT, Start of Green
Natural Cycle:	70
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.59
Intersection Signal Delay:	14.8
Intersection LOS:	B
Intersection Capacity Utilization:	45.3%
ICU Level of Service:	A
Analysis Period (min):	15

Splits and Phases: 3911: Schaefer Hwy & Jeffries Service Dr



Lane Group	Ø8
Turn Type	
Protected Phases	8
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
Minimum Split (s)	28.7
Total Split (s)	31.0
Total Split (%)	34%
Maximum Green (s)	25.3
Yellow Time (s)	3.0
All-Red Time (s)	2.7
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	16.0
Pedestrian Calls (#/hr)	0
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Lanes, Volumes, Timings
4001: Schaefer Hwy & Lyndon St

07/07/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	30	84	26	46	106	24	29	312	42	32	307	26
Future Volume (vph)	30	84	26	46	106	24	29	312	42	32	307	26
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	80		0	80		0	80		0	80		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	1.00	1.00		1.00	1.00		0.99	1.00		1.00	1.00	
Fr _t		0.957			0.975			0.979			0.983	
Fl _t Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1809	0	1805	1848	0	1805	3479	0	1752	3537	0
Fl _t Permitted	0.662			0.676			0.526			0.495		
Satd. Flow (perm)	1256	1809	0	1278	1848	0	994	3479	0	912	3537	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		34			17			31			24	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		648			545			2274			685	
Travel Time (s)		14.7			12.4			51.7			15.6	
Confl. Peds. (#/hr)	2		6	6		2	6		1	1		6
Peak Hour Factor	0.73	0.92	0.72	0.73	0.85	0.95	0.68	0.91	0.77	0.60	0.91	0.60
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	1%	3%	3%	0%	0%
Adj. Flow (vph)	41	91	36	63	125	25	43	343	55	53	337	43
Shared Lane Traffic (%)												
Lane Group Flow (vph)	41	127	0	63	150	0	43	398	0	53	380	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	

Lanes, Volumes, Timings
4001: Schaefer Hwy & Lyndon St

07/07/2022

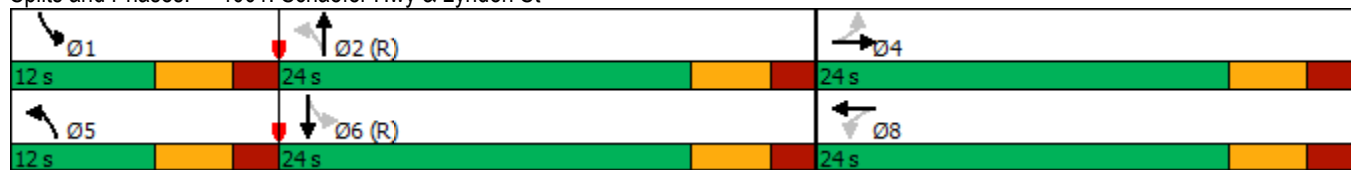


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		4.0	15.0		4.0	15.0	
Minimum Split (s)	24.6	24.6		24.6	24.6		9.6	26.6		9.6	26.6	
Total Split (s)	24.0	24.0		24.0	24.0		12.0	24.0		12.0	24.0	
Total Split (%)	40.0%	40.0%		40.0%	40.0%		20.0%	40.0%		20.0%	40.0%	
Maximum Green (s)	18.4	18.4		18.4	18.4		6.4	18.4		6.4	18.4	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	2.1	2.1		2.1	2.1		2.1	2.1		2.1	2.1	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.6	5.6		5.6	5.6		5.6	5.6		5.6	5.6	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	Min	Min		Min	Min		None	C-Max		None	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	12.0	12.0		12.0	12.0			14.0			14.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	10.9	10.9		10.9	10.9		34.4	30.5		35.7	32.9	
Actuated g/C Ratio	0.18	0.18		0.18	0.18		0.57	0.51		0.60	0.55	
v/c Ratio	0.18	0.36		0.27	0.43		0.07	0.22		0.08	0.19	
Control Delay	22.2	18.8		23.8	23.0		5.2	9.6		5.2	8.5	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	22.2	18.8		23.8	23.0		5.2	9.6		5.2	8.5	
LOS	C	B		C	C		A	A		A	A	
Approach Delay		19.6			23.2			9.2			8.1	
Approach LOS		B			C			A			A	

Intersection Summary

Area Type: Other
 Cycle Length: 60
 Actuated Cycle Length: 60
 Offset: 14 (23%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 65
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.43
 Intersection Signal Delay: 12.6
 Intersection LOS: B
 Intersection Capacity Utilization 57.5%
 ICU Level of Service B
 Analysis Period (min) 15

Splits and Phases: 4001: Schaefer Hwy & Lyndon St



Intersection												
Int Delay, s/veh	0.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	8	158	4	0	142	0	0	0	0	0	0	8
Future Vol, veh/h	8	158	4	0	142	0	0	0	0	0	0	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	60	60	60	60	60	60	25	25	25	25	25	60
Heavy Vehicles, %	0	2	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	13	263	7	0	237	0	0	0	0	0	0	13

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	237	0	0	270	0	0	537	530	267	530	533	237
Stage 1	-	-	-	-	-	-	293	293	-	237	237	-
Stage 2	-	-	-	-	-	-	244	237	-	293	296	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1342	-	-	1305	-	-	458	457	777	463	456	807
Stage 1	-	-	-	-	-	-	719	674	-	771	713	-
Stage 2	-	-	-	-	-	-	764	713	-	719	672	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1342	-	-	1305	-	-	447	452	777	459	451	807
Mov Cap-2 Maneuver	-	-	-	-	-	-	447	452	-	459	451	-
Stage 1	-	-	-	-	-	-	711	667	-	763	713	-
Stage 2	-	-	-	-	-	-	751	713	-	711	665	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.4	0	0	9.5
HCM LOS			A	A

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	1342	-	-	1305	-	-	807
HCM Lane V/C Ratio	-	0.01	-	-	-	-	-	0.017
HCM Control Delay (s)	0	7.7	0	-	0	-	-	9.5
HCM Lane LOS	A	A	A	-	A	-	-	A
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	0.1

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	2	1	6	3	1	7	4	108	17	5	246	3
Future Vol, veh/h	2	1	6	3	1	7	4	108	17	5	246	3
Conflicting Peds, #/hr	0	0	1	1	0	0	2	0	0	0	0	2
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	50	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	60	60	60	60	60	60	100	81	60	60	95	75
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	3	2	10	5	2	12	4	133	28	8	259	4

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	441	448	264	439	436	147	265	0	0	161	0	0
Stage 1	279	279	-	155	155	-	-	-	-	-	-	-
Stage 2	162	169	-	284	281	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	530	509	780	532	517	905	1311	-	-	1430	-	-
Stage 1	732	683	-	852	773	-	-	-	-	-	-	-
Stage 2	845	763	-	727	682	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	517	503	778	520	511	905	1309	-	-	1430	-	-
Mov Cap-2 Maneuver	517	503	-	520	511	-	-	-	-	-	-	-
Stage 1	728	678	-	849	771	-	-	-	-	-	-	-
Stage 2	830	761	-	711	677	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	10.6		10.2		0.2		0.2	
HCM LOS	B		B					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1309	-	-	663	711	1430	-	-
HCM Lane V/C Ratio	0.003	-	-	0.023	0.026	0.006	-	-
HCM Control Delay (s)	7.8	-	-	10.6	10.2	7.5	-	-
HCM Lane LOS	A	-	-	B	B	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0.1	0.1	0	-	-

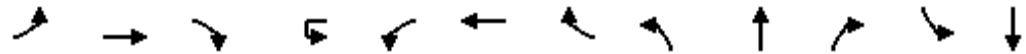


INNOVATIVE IDEAS
EXCEPTIONAL DESIGN
UNMATCHED CLIENT SERVICE

BUILD CONDITIONS

Lanes, Volumes, Timings
 1011: Schaefer Hwy & Grand River Ave

08/08/2022

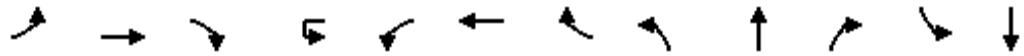


Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	60	460	100	1	27	516	139	79	336	10	68	402
Future Volume (vph)	60	460	100	1	27	516	139	79	336	10	68	402
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	115		0		95		0	155		0	80	
Storage Lanes	1		0		0		0	1		1	1	
Taper Length (ft)	65				65			25			65	
Lane Util. Factor	1.00	0.91	0.91	0.91	1.00	0.91	0.91	1.00	1.00	1.00	1.00	0.95
Ped Bike Factor	1.00	1.00			1.00	0.99		1.00		0.99	1.00	1.00
Frt		0.971				0.963				0.850		0.985
Flt Protected	0.950				0.950			0.950			0.950	
Satd. Flow (prot)	1805	4943	0	0	1805	4807	0	1805	1792	1615	1583	3334
Flt Permitted	0.358				0.406			0.404			0.316	
Satd. Flow (perm)	678	4943	0	0	771	4807	0	767	1792	1593	526	3334
Right Turn on Red			Yes				Yes			Yes		
Satd. Flow (RTOR)		72				105				34		18
Link Speed (mph)		35				35			30			30
Link Distance (ft)		932				427			332			1209
Travel Time (s)		18.2				8.3			7.5			27.5
Confl. Peds. (#/hr)	5		3		3		5	3		2	2	
Peak Hour Factor	0.71	0.92	0.83	0.25	0.78	0.94	0.78	0.83	0.89	0.63	0.81	0.93
Heavy Vehicles (%)	0%	2%	0%	0%	0%	2%	7%	0%	6%	0%	14%	7%
Adj. Flow (vph)	85	500	120	4	35	549	178	95	378	16	84	432
Shared Lane Traffic (%)												
Lane Group Flow (vph)	85	620	0	0	39	727	0	95	378	16	84	480
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	R NA	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)		12				12			12			12
Link Offset(ft)		0				0			0			0
Crosswalk Width(ft)		16				16			16			16
Two way Left Turn Lane												Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	9	15		9	15		9	15	
Number of Detectors	1	2		1	1	2		1	2	1	1	2
Detector Template	Left	Thru		Left	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	100		20	20	100		20	100	20	20	100
Trailing Detector (ft)	0	0		0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0		0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	6		20	20	6		20	6	20	20	6
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)		94				94			94			94
Detector 2 Size(ft)		6				6			6			6
Detector 2 Type		Cl+Ex				Cl+Ex			Cl+Ex			Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0				0.0			0.0			0.0

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	44
Future Volume (vph)	44
Ideal Flow (vphpl)	1900
Storage Length (ft)	240
Storage Lanes	1
Taper Length (ft)	
Lane Util. Factor	0.95
Ped Bike Factor	
Flt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	3
Peak Hour Factor	0.91
Heavy Vehicles (%)	2%
Adj. Flow (vph)	48
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	

Lanes, Volumes, Timings
1011: Schaefer Hwy & Grand River Ave

08/08/2022

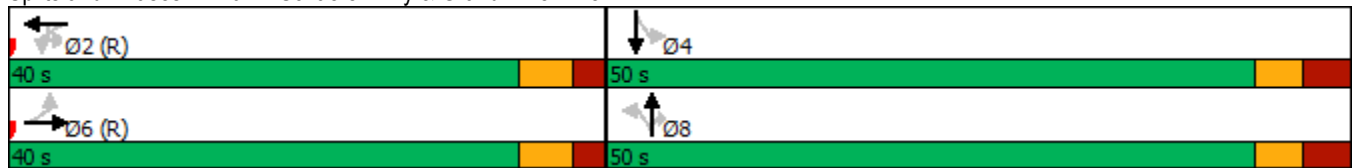


Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Turn Type	Perm	NA		Perm	Perm	NA		Perm	NA	Perm	Perm	NA
Protected Phases		6				2			8			4
Permitted Phases	6			2	2			8		8	4	
Detector Phase	6	6		2	2	2		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0		7.0	7.0	7.0	7.0	7.0
Minimum Split (s)	35.8	35.8		35.8	35.8	35.8		46.5	46.5	46.5	46.5	46.5
Total Split (s)	40.0	40.0		40.0	40.0	40.0		50.0	50.0	50.0	50.0	50.0
Total Split (%)	44.4%	44.4%		44.4%	44.4%	44.4%		55.6%	55.6%	55.6%	55.6%	55.6%
Maximum Green (s)	34.2	34.2		34.2	34.2	34.2		43.5	43.5	43.5	43.5	43.5
Yellow Time (s)	3.6	3.6		3.6	3.6	3.6		3.2	3.2	3.2	3.2	3.2
All-Red Time (s)	2.2	2.2		2.2	2.2	2.2		3.3	3.3	3.3	3.3	3.3
Lost Time Adjust (s)	0.0	0.0			0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.8	5.8			5.8	5.8		6.5	6.5	6.5	6.5	6.5
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max		C-Max	C-Max	C-Max		None	None	None	None	None
Walk Time (s)	7.0	7.0		7.0	7.0	7.0		10.0	10.0	10.0	10.0	10.0
Flash Dont Walk (s)	23.0	23.0		23.0	23.0	23.0		30.0	30.0	30.0	30.0	30.0
Pedestrian Calls (#/hr)	0	0		0	0	0		0	0	0	0	0
Act Effct Green (s)	52.0	52.0			52.0	52.0		25.7	25.7	25.7	25.7	25.7
Actuated g/C Ratio	0.58	0.58			0.58	0.58		0.29	0.29	0.29	0.29	0.29
v/c Ratio	0.22	0.21			0.09	0.26		0.43	0.74	0.03	0.56	0.50
Control Delay	13.4	9.2			9.5	7.3		20.5	27.6	3.0	40.4	26.5
Queue Delay	0.0	0.0			0.0	0.0		0.0	0.1	0.0	0.0	0.0
Total Delay	13.4	9.2			9.5	7.3		20.5	27.8	3.0	40.4	26.5
LOS	B	A			A	A		C	C	A	D	C
Approach Delay		9.7				7.4			25.6			28.6
Approach LOS		A				A			C			C

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 59 (66%), Referenced to phase 2:WBTL and 6:EBTL, Start of Green
 Natural Cycle: 85
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.74
 Intersection Signal Delay: 16.3
 Intersection LOS: B
 Intersection Capacity Utilization 78.4%
 ICU Level of Service D
 Analysis Period (min) 15

Splits and Phases: 1011: Schaefer Hwy & Grand River Ave

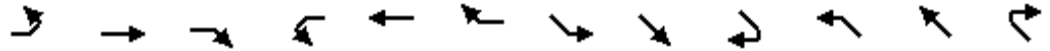




Lane Group	SBR
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Walk Time (s)	
Flash Dont Walk (s)	
Pedestrian Calls (#/hr)	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Lanes, Volumes, Timings
 1093: Grand River Ave & Jeffries Service Dr

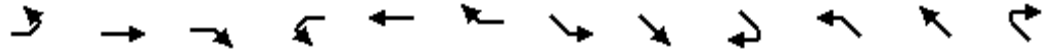
08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↑↑↑↑						↑↑↑↑			↑↑↑↑	
Traffic Volume (vph)	6	319	73	0	0	0	5	462	0	0	421	20
Future Volume (vph)	6	319	73	0	0	0	5	462	0	0	421	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.86	0.86	0.86	1.00	1.00	1.00	0.91	0.91	1.00	1.00	0.91	0.91
Ped Bike Factor								1.00			1.00	
Frt		0.973									0.986	
Flt Protected		0.999						0.998				
Satd. Flow (prot)	0	5922	0	0	0	0	0	5030	0	0	4994	0
Flt Permitted		0.999						0.920				
Satd. Flow (perm)	0	5922	0	0	0	0	0	4637	0	0	4994	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		79									20	
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		329			379			582			812	
Travel Time (s)		6.4			7.4			11.3			15.8	
Confl. Peds. (#/hr)							1					1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.31	0.91	0.92	0.92	0.92	0.43
Heavy Vehicles (%)	2%	7%	9%	2%	2%	2%	0%	3%	2%	2%	2%	5%
Adj. Flow (vph)	7	347	79	0	0	0	16	508	0	0	458	47
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	433	0	0	0	0	0	524	0	0	505	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2					1	2			2	
Detector Template	Left	Thru					Left	Thru			Thru	
Leading Detector (ft)	20	100					20	100			100	
Trailing Detector (ft)	0	0					0	0			0	
Detector 1 Position(ft)	0	0					0	0			0	
Detector 1 Size(ft)	20	6					20	6			6	
Detector 1 Type	Cl+Ex	Cl+Ex					Cl+Ex	Cl+Ex			Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0			0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0			0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0			0.0	
Detector 2 Position(ft)		94						94			94	
Detector 2 Size(ft)		6						6			6	
Detector 2 Type		Cl+Ex						Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA			NA	
Protected Phases		4						6			2	
Permitted Phases	4						6					

Lanes, Volumes, Timings
 1093: Grand River Ave & Jeffries Service Dr

08/08/2022

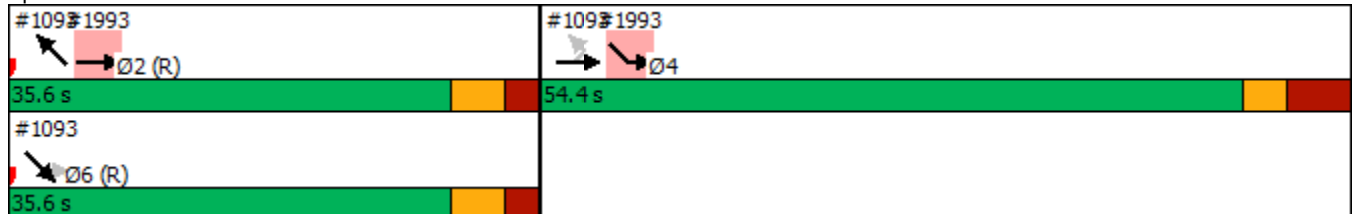


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Detector Phase	4	4					6	6				2
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0				10.0
Minimum Split (s)	57.4	57.4					37.0	37.0				37.0
Total Split (s)	54.4	54.4					35.6	35.6				35.6
Total Split (%)	60.4%	60.4%					39.6%	39.6%				39.6%
Maximum Green (s)	47.0	47.0					29.6	29.6				29.6
Yellow Time (s)	3.0	3.0					3.6	3.6				3.6
All-Red Time (s)	4.4	4.4					2.4	2.4				2.4
Lost Time Adjust (s)		0.0						0.0				0.0
Total Lost Time (s)		7.4						6.0				6.0
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0				3.0
Recall Mode	None	None					C-Max	C-Max				C-Max
Walk Time (s)	9.0	9.0					7.0	7.0				7.0
Flash Dont Walk (s)	33.0	33.0					24.0	24.0				24.0
Pedestrian Calls (#/hr)	0	0					0	0				0
Act Effct Green (s)		12.0						64.6				64.6
Actuated g/C Ratio		0.13						0.72				0.72
v/c Ratio		0.50						0.16				0.14
Control Delay		34.6						1.7				4.1
Queue Delay		0.0						0.0				0.0
Total Delay		34.6						1.7				4.1
LOS		C						A				A
Approach Delay		34.6						1.7				4.1
Approach LOS		C						A				A

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 78 (87%), Referenced to phase 2:NWT and 6:SETL, Start of Green
 Natural Cycle: 95
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.50
 Intersection Signal Delay: 12.3
 Intersection LOS: B
 Intersection Capacity Utilization 45.3%
 ICU Level of Service A
 Analysis Period (min) 15

Splits and Phases: 1093: Grand River Ave & Jeffries Service Dr



Lanes, Volumes, Timings

1193: Left Turn Bridge & Grand River Ave & I-96 WB Service Rd/I-96 WB Service Dr 08/08/2022



Lane Group	WBL2	WBL	WBT	WBR	SET	SER	SER2	NWL	NWT
Lane Configurations			↑↑↑	↑	↑↑↑	↑		↑	↑↑↑
Traffic Volume (vph)	42	5	193	316	425	107	6	61	366
Future Volume (vph)	42	5	193	316	425	107	6	61	366
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0		0		140		260	
Storage Lanes		0		1		1		1	
Taper Length (ft)		65						65	
Lane Util. Factor	0.91	0.91	0.86	0.86	0.86	0.86	0.91	1.00	0.91
Frt			0.940	0.850	0.994	0.850			
Flt Protected			0.994					0.950	
Satd. Flow (prot)	0	0	4394	1335	4725	1317	0	1805	5036
Flt Permitted			0.994					0.950	
Satd. Flow (perm)	0	0	4394	1335	4725	1317	0	1805	5036
Right Turn on Red				Yes			Yes		
Satd. Flow (RTOR)			172	171		126			
Link Speed (mph)			35		35				35
Link Distance (ft)			675		427				582
Travel Time (s)			13.1		8.3				11.3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.93	0.74	0.50	0.78	0.89
Heavy Vehicles (%)	2%	2%	5%	4%	3%	6%	0%	0%	3%
Adj. Flow (vph)	46	5	210	343	457	145	12	78	411
Shared Lane Traffic (%)				50%		14%			
Lane Group Flow (vph)	0	0	433	171	477	137	0	78	411
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right	Right	Left	Left
Median Width(ft)			0		12				12
Link Offset(ft)			0		0				0
Crosswalk Width(ft)			16		16				16
Two way Left Turn Lane									
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	15		9		9	9	15	
Number of Detectors	1	1	2	1	2	1		1	2
Detector Template	Left	Left	Thru	Right	Thru	Right		Left	Thru
Leading Detector (ft)	20	20	100	20	100	20		20	100
Trailing Detector (ft)	0	0	0	0	0	0		0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0
Detector 1 Size(ft)	20	20	6	20	6	20		20	6
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex
Detector 1 Channel									
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
Detector 2 Position(ft)			94		94				94
Detector 2 Size(ft)			6		6				6
Detector 2 Type			Cl+Ex		Cl+Ex				Cl+Ex
Detector 2 Channel									
Detector 2 Extend (s)			0.0		0.0				0.0
Turn Type	Perm	Perm	NA	Perm	NA	Perm		Prot	NA
Protected Phases			4		6			5	2

Lanes, Volumes, Timings

1193: Left Turn Bridge & Grand River Ave & I-96 WB Service Rd/I-96 WB Service Dr 08/08/2022

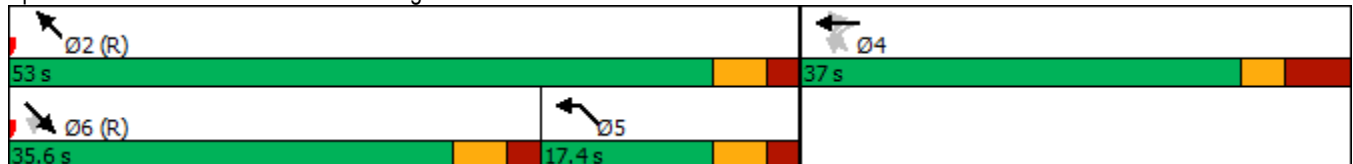


Lane Group	WBL2	WBL	WBT	WBR	SET	SER	SER2	NWL	NWT
Permitted Phases	4	4		4		6			
Detector Phase	4	4	4	4	6	6		5	2
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0		7.0	10.0
Minimum Split (s)	55.5	55.5	55.5	55.5	37.9	37.9		12.9	37.9
Total Split (s)	37.0	37.0	37.0	37.0	35.6	35.6		17.4	53.0
Total Split (%)	41.1%	41.1%	41.1%	41.1%	39.6%	39.6%		19.3%	58.9%
Maximum Green (s)	29.5	29.5	29.5	29.5	29.7	29.7		11.5	47.1
Yellow Time (s)	3.0	3.0	3.0	3.0	3.6	3.6		3.6	3.6
All-Red Time (s)	4.5	4.5	4.5	4.5	2.3	2.3		2.3	2.3
Lost Time Adjust (s)			0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)			7.5	7.5	5.9	5.9		5.9	5.9
Lead/Lag					Lead	Lead		Lag	
Lead-Lag Optimize?					Yes	Yes		Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max		None	C-Max
Walk Time (s)	9.0	9.0	9.0	9.0	7.0	7.0			7.0
Flash Dont Walk (s)	39.0	39.0	39.0	39.0	25.0	25.0			25.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0			0
Act Effct Green (s)			12.6	12.6	50.0	50.0		10.6	64.0
Actuated g/C Ratio			0.14	0.14	0.56	0.56		0.12	0.71
v/c Ratio			0.57	0.51	0.18	0.17		0.37	0.11
Control Delay			24.0	11.2	7.4	1.5		38.0	0.9
Queue Delay			0.0	0.0	0.0	0.0		0.0	0.0
Total Delay			24.0	11.2	7.4	1.5		38.0	0.9
LOS			C	B	A	A		D	A
Approach Delay			20.4		6.1				6.8
Approach LOS			C		A				A

Intersection Summary

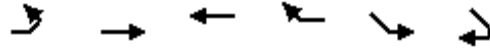
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 72 (80%), Referenced to phase 2:NWT and 6:SET, Start of Green
 Natural Cycle: 110
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.57
 Intersection Signal Delay: 11.4
 Intersection Capacity Utilization 39.3%
 Analysis Period (min) 15
 Intersection LOS: B
 ICU Level of Service A

Splits and Phases: 1193: Left Turn Bridge & Grand River Ave & I-96 WB Service Rd/I-96 WB Service Dr



Lanes, Volumes, Timings
 1993: Jeffries Service Dr & Left Turn Bridge

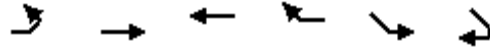
08/08/2022



Lane Group	EBL	EBT	WBT	WBR	SEL	SER	Ø6
Lane Configurations		↑↑↑			↑↑		
Traffic Volume (vph)	0	286	0	0	112	0	
Future Volume (vph)	0	286	0	0	112	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (ft)	0			0	50	0	
Storage Lanes	0			0	0	0	
Taper Length (ft)	65				65		
Lane Util. Factor	1.00	0.91	1.00	1.00	0.97	1.00	
Frt							
Flt Protected					0.950		
Satd. Flow (prot)	0	5085	0	0	3433	0	
Flt Permitted					0.950		
Satd. Flow (perm)	0	5085	0	0	3433	0	
Right Turn on Red				Yes	Yes	Yes	
Satd. Flow (RTOR)					621		
Link Speed (mph)		35	35		30		
Link Distance (ft)		184	329		419		
Travel Time (s)		3.6	6.4		9.5		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	311	0	0	122	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	311	0	0	122	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		0	0		24		
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		16	16		16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15			9	15	9	
Number of Detectors		2			1		
Detector Template		Thru			Left		
Leading Detector (ft)		100			20		
Trailing Detector (ft)		0			0		
Detector 1 Position(ft)		0			0		
Detector 1 Size(ft)		6			20		
Detector 1 Type		Cl+Ex			Cl+Ex		
Detector 1 Channel							
Detector 1 Extend (s)		0.0			0.0		
Detector 1 Queue (s)		0.0			0.0		
Detector 1 Delay (s)		0.0			0.0		
Detector 2 Position(ft)		94					
Detector 2 Size(ft)		6					
Detector 2 Type		Cl+Ex					
Detector 2 Channel							
Detector 2 Extend (s)		0.0					
Turn Type		NA			Prot		
Protected Phases		2			4		6
Permitted Phases							

Lanes, Volumes, Timings
 1993: Jeffries Service Dr & Left Turn Bridge

08/08/2022

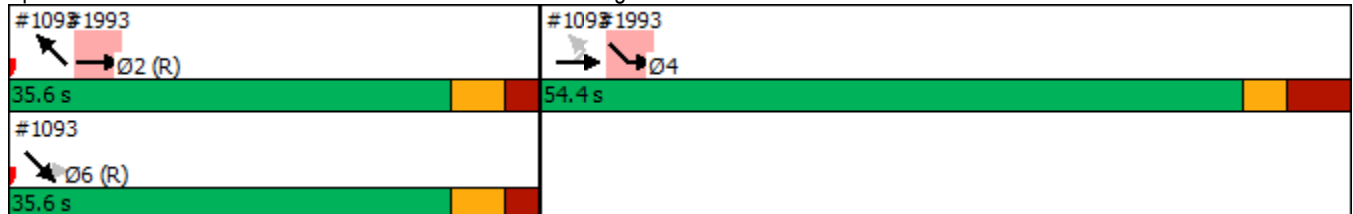


Lane Group	EBL	EBT	WBT	WBR	SEL	SER	Ø6
Detector Phase		2			4		
Switch Phase							
Minimum Initial (s)		10.0			10.0		10.0
Minimum Split (s)		37.0			57.4		37.0
Total Split (s)		35.6			54.4		35.6
Total Split (%)		39.6%			60.4%		40%
Maximum Green (s)		29.6			47.0		29.6
Yellow Time (s)		3.6			3.0		3.6
All-Red Time (s)		2.4			4.4		2.4
Lost Time Adjust (s)		0.0			0.0		
Total Lost Time (s)		6.0			7.4		
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)		3.0			3.0		3.0
Recall Mode		C-Max			None		C-Max
Walk Time (s)		7.0			9.0		7.0
Flash Dont Walk (s)		24.0			33.0		24.0
Pedestrian Calls (#/hr)		0			0		0
Act Effct Green (s)		64.6			12.0		
Actuated g/C Ratio		0.72			0.13		
v/c Ratio		0.09			0.12		
Control Delay		3.1			1.3		
Queue Delay		0.0			0.0		
Total Delay		3.1			1.3		
LOS		A			A		
Approach Delay		3.1			1.3		
Approach LOS		A			A		

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	78 (87%), Referenced to phase 2:NWT and 6:SETL, Start of Green
Natural Cycle:	95
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.50
Intersection Signal Delay:	2.6
Intersection LOS:	A
Intersection Capacity Utilization:	27.8%
ICU Level of Service:	A
Analysis Period (min):	15

Splits and Phases: 1993: Jeffries Service Dr & Left Turn Bridge



Lanes, Volumes, Timings
2609: Schaefer Hwy & Schoolcraft Rd

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	42	93	17	49	80	44	17	461	57	35	448	36
Future Volume (vph)	42	93	17	49	80	44	17	461	57	35	448	36
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	60		0	60		0	80		250	80		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor								1.00		1.00		
Fr _t		0.966			0.950			0.981				0.985
Fl _t Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1378	1821	0	1656	1714	0	1805	1769	0	1530	1693	0
Fl _t Permitted	0.650			0.669			0.152			0.398		
Satd. Flow (perm)	943	1821	0	1166	1714	0	289	1769	0	639	1693	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		18			31			11				8
Link Speed (mph)		30			30			30				30
Link Distance (ft)		699			1470			1209				229
Travel Time (s)		15.9			33.4			27.5				5.2
Confl. Peds. (#/hr)									4	4		
Peak Hour Factor	0.63	0.88	0.54	0.84	0.71	0.79	0.57	0.94	0.81	0.66	0.91	0.68
Heavy Vehicles (%)	31%	1%	0%	9%	1%	14%	0%	5%	5%	18%	8%	34%
Adj. Flow (vph)	67	106	31	58	113	56	30	490	70	53	492	53
Shared Lane Traffic (%)												
Lane Group Flow (vph)	67	137	0	58	169	0	30	560	0	53	545	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	

Lanes, Volumes, Timings
2609: Schaefer Hwy & Schoolcraft Rd

08/08/2022



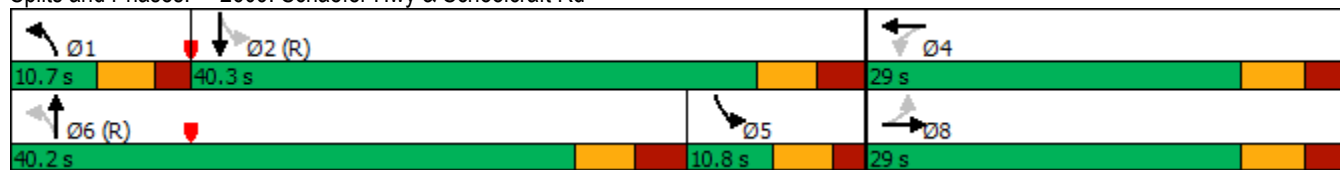
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		
Detector Phase	8	8		4	4		1	6		5	2	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0		5.0	9.0		5.0	9.0	
Minimum Split (s)	28.8	28.8		28.8	28.8		10.6	26.6		10.6	26.6	
Total Split (s)	29.0	29.0		29.0	29.0		10.7	40.2		10.8	40.3	
Total Split (%)	36.3%	36.3%		36.3%	36.3%		13.4%	50.3%		13.5%	50.4%	
Maximum Green (s)	22.2	22.2		22.2	22.2		5.1	33.6		5.2	33.7	
Yellow Time (s)	3.8	3.8		3.8	3.8		3.5	3.6		3.5	3.6	
All-Red Time (s)	3.0	3.0		3.0	3.0		2.1	3.0		2.1	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.8	6.8		6.8	6.8		5.6	6.6		5.6	6.6	
Lead/Lag							Lead	Lead		Lag	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	Max	Max		Max	Max		Max	C-Max		Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0			13.0			13.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	22.2	22.2		22.2	22.2		34.6	33.6		34.7	33.7	
Actuated g/C Ratio	0.28	0.28		0.28	0.28		0.43	0.42		0.43	0.42	
v/c Ratio	0.26	0.26		0.18	0.34		0.14	0.75		0.16	0.76	
Control Delay	25.8	21.1		23.8	21.0		14.5	26.8		11.1	22.4	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	25.8	21.1		23.8	21.0		14.5	26.8		11.1	22.4	
LOS	C	C		C	C		B	C		B	C	
Approach Delay		22.7			21.7			26.2			21.4	
Approach LOS		C			C			C			C	

Intersection Summary

Area Type: Other
 Cycle Length: 80
 Actuated Cycle Length: 80
 Offset: 48 (60%), Referenced to phase 2:SBTL and 6:NBTL, Start of Green
 Natural Cycle: 75
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.76
 Intersection Signal Delay: 23.3
 Intersection Capacity Utilization 57.8%
 Analysis Period (min) 15

Intersection LOS: C
 ICU Level of Service B

Splits and Phases: 2609: Schaefer Hwy & Schoolcraft Rd



Lanes, Volumes, Timings
3011: Schaefer Hwy & I-96 WB Service Rd

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				↙	↕		↙	↕			↕	
Traffic Volume (vph)	0	0	0	128	129	3	191	422	0	0	309	220
Future Volume (vph)	0	0	0	128	129	3	191	422	0	0	309	220
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Lane Width (ft)	12	12	12	12	11	12	11	11	12	12	11	12
Storage Length (ft)	0		0	0		0	250		0	0		0
Storage Lanes	0		0	1		0	1		0	0		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95
Ped Bike Factor							1.00					0.99
Frt					0.997							0.937
Flt Protected				0.950			0.950					
Satd. Flow (prot)	0	0	0	1863	3568	0	1766	3566	0	0	3231	0
Flt Permitted				0.950			0.422					
Satd. Flow (perm)	0	0	0	1863	3568	0	783	3566	0	0	3231	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					2							237
Link Speed (mph)		25			25			25				35
Link Distance (ft)		298			364			384				332
Travel Time (s)		8.1			9.9			10.5				6.5
Confl. Peds. (#/hr)							4					4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.83	0.91	0.92	0.92	0.95	0.93
Heavy Vehicles (%)	2%	2%	2%	2%	2%	33%	4%	3%	2%	2%	7%	4%
Adj. Flow (vph)	0	0	0	139	140	3	230	464	0	0	325	237
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	139	143	0	230	464	0	0	562	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												
Headway Factor	0.94	0.94	0.94	0.94	0.98	0.94	0.98	0.98	0.94	0.94	0.98	0.94
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors				1	2		1	2				2
Detector Template				Left	Thru		Left	Thru				Thru
Leading Detector (ft)				20	100		20	100				100
Trailing Detector (ft)				0	0		0	0				0
Detector 1 Position(ft)				0	0		0	0				0
Detector 1 Size(ft)				20	6		20	6				6
Detector 1 Type				Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex				Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)				0.0	0.0		0.0	0.0				0.0
Detector 1 Queue (s)				0.0	0.0		0.0	0.0				0.0
Detector 1 Delay (s)				0.0	0.0		0.0	0.0				0.0
Detector 2 Position(ft)					94			94				94
Detector 2 Size(ft)					6			6				6
Detector 2 Type					Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	

Lanes, Volumes, Timings
3011: Schaefer Hwy & I-96 WB Service Rd

08/08/2022

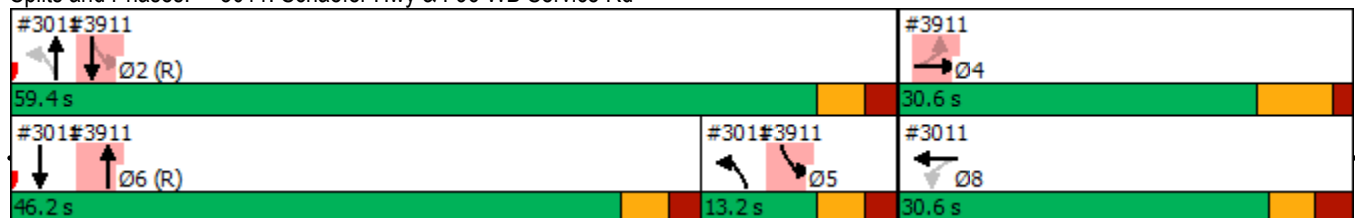


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)					0.0			0.0			0.0	
Turn Type				Perm	NA		pm+pt	NA			NA	
Protected Phases					8		5	2			6	
Permitted Phases				8			2					
Detector Phase				8	8		5	2			6	
Switch Phase												
Minimum Initial (s)				10.0	10.0		7.0	10.0			10.0	
Minimum Split (s)				28.7	28.7		12.4	23.4			23.4	
Total Split (s)				30.6	30.6		13.2	59.4			46.2	
Total Split (%)				34.0%	34.0%		14.7%	66.0%			51.3%	
Maximum Green (s)				24.9	24.9		7.8	54.0			40.8	
Yellow Time (s)				3.0	3.0		3.2	3.2			3.2	
All-Red Time (s)				2.7	2.7		2.2	2.2			2.2	
Lost Time Adjust (s)				0.0	0.0		0.0	0.0			0.0	
Total Lost Time (s)				5.7	5.7		5.4	5.4			5.4	
Lead/Lag							Lag				Lead	
Lead-Lag Optimize?							Yes				Yes	
Vehicle Extension (s)				3.0	3.0		3.0	3.0			3.0	
Recall Mode				None	None		None	C-Max			C-Max	
Walk Time (s)				7.0	7.0			7.0			7.0	
Flash Dont Walk (s)				16.0	16.0			11.0			11.0	
Pedestrian Calls (#/hr)				0	0			0			0	
Act Effct Green (s)				15.7	15.7		63.2	63.2			50.0	
Actuated g/C Ratio				0.17	0.17		0.70	0.70			0.56	
v/c Ratio				0.43	0.23		0.36	0.19			0.30	
Control Delay				22.1	16.8		4.6	2.7			3.4	
Queue Delay				0.0	0.0		0.0	0.0			0.1	
Total Delay				22.1	16.8		4.6	2.7			3.6	
LOS				C	B		A	A			A	
Approach Delay					19.4			3.3			3.6	
Approach LOS					B			A			A	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 79 (88%), Referenced to phase 2:NBTL and 6:SBT, Start of Green
 Natural Cycle: 80
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.67
 Intersection Signal Delay: 6.3
 Intersection LOS: A
 Intersection Capacity Utilization 59.5%
 ICU Level of Service B
 Analysis Period (min) 15

Splits and Phases: 3011: Schaefer Hwy & I-96 WB Service Rd



Lane Group	Ø4
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
Minimum Split (s)	30.5
Total Split (s)	30.6
Total Split (%)	34%
Maximum Green (s)	24.1
Yellow Time (s)	5.0
All-Red Time (s)	1.5
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	17.0
Pedestrian Calls (#/hr)	0
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Lanes, Volumes, Timings
3911: Schaefer Hwy & Jeffries Service Dr

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕↕↕						↔		↕	↕↕	
Traffic Volume (vph)	213	110	245	0	0	0	0	400	100	76	361	0
Future Volume (vph)	213	110	245	0	0	0	0	400	100	76	361	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	0		0	90		0
Storage Lanes	0		0	0		0	0		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	0.91	0.91	0.91	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00
Ped Bike Factor								1.00				
Frt		0.935						0.972				
Flt Protected		0.982								0.950		
Satd. Flow (prot)	0	4549	0	0	0	0	0	1721	0	1703	3406	0
Flt Permitted		0.982								0.322		
Satd. Flow (perm)	0	4549	0	0	0	0	0	1721	0	577	3406	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		208						19				
Link Speed (mph)		35			35			30				30
Link Distance (ft)		417			372			621				384
Travel Time (s)		8.1			7.2			14.1				8.7
Confl. Peds. (#/hr)									6	6		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.84	0.80	0.80	0.92	0.92
Heavy Vehicles (%)	2%	7%	6%	2%	2%	2%	2%	6%	10%	6%	6%	2%
Bus Blockages (#/hr)	3	0	0	0	0	0	0	0	0	0	0	0
Adj. Flow (vph)	232	120	266	0	0	0	0	476	125	95	392	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	618	0	0	0	0	0	601	0	95	392	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2						2		1	2	
Detector Template	Left	Thru						Thru		Left	Thru	
Leading Detector (ft)	20	100						100		20	100	
Trailing Detector (ft)	0	0						0		0	0	
Detector 1 Position(ft)	0	0						0		0	0	
Detector 1 Size(ft)	20	6						6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex						Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0						0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0						0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0						0.0		0.0	0.0	
Detector 2 Position(ft)		94						94			94	
Detector 2 Size(ft)		6						6			6	
Detector 2 Type		Cl+Ex						Cl+Ex			Cl+Ex	
Detector 2 Channel												

Lane Group	Ø8
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Bus Blockages (#/hr)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	

Lanes, Volumes, Timings
 3911: Schaefer Hwy & Jeffries Service Dr

08/08/2022

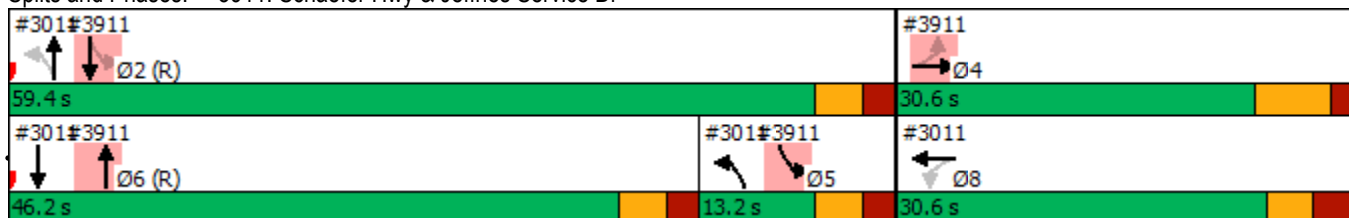


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA						NA	pm+pt		NA	
Protected Phases		4						6		5	2	
Permitted Phases	4									2		
Detector Phase	4	4						6		5	2	
Switch Phase												
Minimum Initial (s)	10.0	10.0						10.0		7.0	10.0	
Minimum Split (s)	30.5	30.5						23.4		12.4	23.4	
Total Split (s)	30.6	30.6						46.2		13.2	59.4	
Total Split (%)	34.0%	34.0%						51.3%		14.7%	66.0%	
Maximum Green (s)	24.1	24.1						40.8		7.8	54.0	
Yellow Time (s)	5.0	5.0						3.2		3.2	3.2	
All-Red Time (s)	1.5	1.5						2.2		2.2	2.2	
Lost Time Adjust (s)		0.0						0.0		0.0	0.0	
Total Lost Time (s)		6.5						5.4		5.4	5.4	
Lead/Lag								Lead		Lag		
Lead-Lag Optimize?								Yes		Yes		
Vehicle Extension (s)	3.0	3.0						3.0		3.0	3.0	
Recall Mode	None	None						C-Max		None	C-Max	
Walk Time (s)	7.0	7.0						7.0			7.0	
Flash Dont Walk (s)	17.0	17.0						11.0			11.0	
Pedestrian Calls (#/hr)	0	0						0			0	
Act Effct Green (s)		14.9						50.0		63.2	63.2	
Actuated g/C Ratio		0.17						0.56		0.70	0.70	
v/c Ratio		0.67						0.62		0.19	0.16	
Control Delay		26.3						17.6		6.7	5.0	
Queue Delay		0.0						0.0		0.0	0.0	
Total Delay		26.3						17.6		6.7	5.0	
LOS		C						B		A	A	
Approach Delay		26.3						17.6			5.4	
Approach LOS		C						B			A	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 79 (88%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 80
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.67
 Intersection Signal Delay: 17.3
 Intersection LOS: B
 Intersection Capacity Utilization 59.5%
 ICU Level of Service B
 Analysis Period (min) 15

Splits and Phases: 3911: Schaefer Hwy & Jeffries Service Dr



Lane Group	Ø8
Detector 2 Extend (s)	
Turn Type	
Protected Phases	8
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
Minimum Split (s)	28.7
Total Split (s)	30.6
Total Split (%)	34%
Maximum Green (s)	24.9
Yellow Time (s)	3.0
All-Red Time (s)	2.7
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	16.0
Pedestrian Calls (#/hr)	0
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Lanes, Volumes, Timings
4001: Schaefer Hwy & Lyndon St

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	23	120	28	56	81	27	36	471	54	47	369	35
Future Volume (vph)	23	120	28	56	81	27	36	471	54	47	369	35
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	80		0	80		0	80		0	80		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor		1.00		1.00			0.99	1.00		1.00	1.00	
Fr _t		0.972			0.963			0.982			0.983	
Fl _t Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1785	0	1656	1812	0	1752	3328	0	1770	3212	0
Fl _t Permitted	0.631			0.523			0.481			0.398		
Satd. Flow (perm)	1199	1785	0	910	1812	0	882	3328	0	739	3212	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		15			21			22			21	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		648			545			853			685	
Travel Time (s)		14.7			12.4			19.4			15.6	
Confl. Peds. (#/hr)			2	2			6		5	5		6
Peak Hour Factor	0.69	0.76	0.75	0.75	0.70	0.72	0.67	0.88	0.73	0.75	0.88	0.65
Heavy Vehicles (%)	0%	3%	4%	9%	0%	4%	3%	7%	0%	2%	11%	3%
Adj. Flow (vph)	33	158	37	75	116	38	54	535	74	63	419	54
Shared Lane Traffic (%)												
Lane Group Flow (vph)	33	195	0	75	154	0	54	609	0	63	473	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	

Lanes, Volumes, Timings
4001: Schaefer Hwy & Lyndon St

08/08/2022

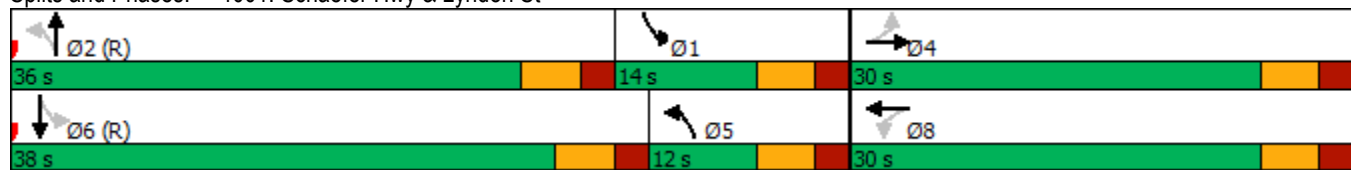


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		4.0	15.0		4.0	15.0	
Minimum Split (s)	24.6	24.6		24.6	24.6		9.6	26.6		9.6	26.6	
Total Split (s)	30.0	30.0		30.0	30.0		12.0	36.0		14.0	38.0	
Total Split (%)	37.5%	37.5%		37.5%	37.5%		15.0%	45.0%		17.5%	47.5%	
Maximum Green (s)	24.4	24.4		24.4	24.4		6.4	30.4		8.4	32.4	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	2.1	2.1		2.1	2.1		2.1	2.1		2.1	2.1	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.6	5.6		5.6	5.6		5.6	5.6		5.6	5.6	
Lead/Lag							Lag	Lead		Lag	Lead	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	Min	Min		Min	Min		None	C-Max		None	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	12.0	12.0		12.0	12.0			14.0			14.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	13.8	13.8		13.8	13.8		50.4	44.7		53.9	48.1	
Actuated g/C Ratio	0.17	0.17		0.17	0.17		0.63	0.56		0.67	0.60	
v/c Ratio	0.16	0.61		0.48	0.47		0.09	0.33		0.11	0.24	
Control Delay	28.3	36.0		39.3	29.6		4.0	4.5		5.9	9.1	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	28.3	36.0		39.3	29.6		4.0	4.5		5.9	9.1	
LOS	C	D		D	C		A	A		A	A	
Approach Delay		34.9			32.8			4.5			8.7	
Approach LOS		C			C			A			A	

Intersection Summary

Area Type: Other
 Cycle Length: 80
 Actuated Cycle Length: 80
 Offset: 16 (20%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 65
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.61
 Intersection Signal Delay: 13.9
 Intersection LOS: B
 Intersection Capacity Utilization 56.7%
 ICU Level of Service B
 Analysis Period (min) 15

Splits and Phases: 4001: Schaefer Hwy & Lyndon St



Intersection												
Int Delay, s/veh	1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	5	178	2	1	166	4	1	0	6	5	0	6
Future Vol, veh/h	5	178	2	1	166	4	1	0	6	5	0	6
Conflicting Peds, #/hr	3	0	1	1	0	3	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	63	94	50	25	86	50	25	25	50	63	25	50
Heavy Vehicles, %	0	7	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	8	189	4	4	193	8	4	0	12	8	0	12

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	204	0	0	194	0	0	419	420	192	421	418	200
Stage 1	-	-	-	-	-	-	208	208	-	208	208	-
Stage 2	-	-	-	-	-	-	211	212	-	213	210	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1380	-	-	1391	-	-	548	528	855	546	529	846
Stage 1	-	-	-	-	-	-	799	734	-	799	734	-
Stage 2	-	-	-	-	-	-	796	731	-	794	732	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1376	-	-	1390	-	-	535	521	854	533	522	844
Mov Cap-2 Maneuver	-	-	-	-	-	-	535	521	-	533	522	-
Stage 1	-	-	-	-	-	-	793	728	-	791	730	-
Stage 2	-	-	-	-	-	-	782	727	-	777	726	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0.1			10			10.4		
HCM LOS							B			B		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	743	1376	-	-	1390	-	-	685
HCM Lane V/C Ratio	0.022	0.006	-	-	0.003	-	-	0.029
HCM Control Delay (s)	10	7.6	0	-	7.6	0	-	10.4
HCM Lane LOS	B	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.1

Intersection												
Int Delay, s/veh	1.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	3	1	8	7	6	7	2	302	10	7	282	1
Future Vol, veh/h	3	1	8	7	6	7	2	302	10	7	282	1
Conflicting Peds, #/hr	0	0	0	0	0	0	1	0	1	1	0	1
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	50	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	60	60	60	60	60	60	100	85	63	60	87	60
Heavy Vehicles, %	0	0	0	0	0	0	0	5	0	0	3	0
Mvmt Flow	5	2	13	12	10	12	2	355	16	12	324	2

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	728	726	326	725	719	364	327	0	0	372	0	0
Stage 1	350	350	-	368	368	-	-	-	-	-	-	-
Stage 2	378	376	-	357	351	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	341	354	720	343	357	685	1244	-	-	1198	-	-
Stage 1	671	636	-	656	625	-	-	-	-	-	-	-
Stage 2	648	620	-	665	636	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	325	349	719	332	352	684	1243	-	-	1197	-	-
Mov Cap-2 Maneuver	325	349	-	332	352	-	-	-	-	-	-	-
Stage 1	669	629	-	654	623	-	-	-	-	-	-	-
Stage 2	626	618	-	644	629	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	12.2		14.5		0		0.3	
HCM LOS	B		B					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1243	-	-	517	414	1197	-
HCM Lane V/C Ratio	0.002	-	-	0.039	0.081	0.01	-
HCM Control Delay (s)	7.9	-	-	12.2	14.5	8	-
HCM Lane LOS	A	-	-	B	B	A	-
HCM 95th %tile Q(veh)	0	-	-	0.1	0.3	0	-

Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↙	↗	↑↓			↑↓
Traffic Vol, veh/h	3	3	544	3	2	516
Future Vol, veh/h	3	3	544	3	2	516
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	100	100	2	100	100	2
Mvmt Flow	3	3	591	3	2	561

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	878	297	0	0	594
Stage 1	593	-	-	-	-
Stage 2	285	-	-	-	-
Critical Hdwy	8.8	8.9	-	-	6.1
Critical Hdwy Stg 1	7.8	-	-	-	-
Critical Hdwy Stg 2	7.8	-	-	-	-
Follow-up Hdwy	4.5	4.3	-	-	3.2
Pot Cap-1 Maneuver	154	477	-	-	529
Stage 1	313	-	-	-	-
Stage 2	513	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	153	477	-	-	529
Mov Cap-2 Maneuver	153	-	-	-	-
Stage 1	313	-	-	-	-
Stage 2	510	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	20.8	0	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	153	477	529
HCM Lane V/C Ratio	-	-	0.021	0.007	0.004
HCM Control Delay (s)	-	-	29	12.6	11.8
HCM Lane LOS	-	-	D	B	B
HCM 95th %tile Q(veh)	-	-	0.1	0	0

Intersection						
Int Delay, s/veh	1.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	66	20	541	6	1	452
Future Vol, veh/h	66	20	541	6	1	452
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	150	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	7	0	0	8
Mvmt Flow	72	22	588	7	1	491

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	840	298	0	0	595
Stage 1	592	-	-	-	-
Stage 2	248	-	-	-	-
Critical Hdwy	6.8	6.9	-	-	4.1
Critical Hdwy Stg 1	5.8	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2
Pot Cap-1 Maneuver	308	704	-	-	991
Stage 1	521	-	-	-	-
Stage 2	776	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	308	704	-	-	991
Mov Cap-2 Maneuver	308	-	-	-	-
Stage 1	521	-	-	-	-
Stage 2	775	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	17.9	0	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	308	704	991
HCM Lane V/C Ratio	-	-	0.233	0.031	0.001
HCM Control Delay (s)	-	-	20.2	10.3	8.6
HCM Lane LOS	-	-	C	B	A
HCM 95th %tile Q(veh)	-	-	0.9	0.1	0

Lanes, Volumes, Timings
1011: Schaefer Hwy & Grand River Ave

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↕↕↕		↘	↕↕↕		↘	↕	↗	↘	↕↕	
Traffic Volume (vph)	36	433	66	20	446	103	55	251	19	65	291	26
Future Volume (vph)	36	433	66	20	446	103	55	251	19	65	291	26
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	115		0	95		0	155		0	80		240
Storage Lanes	1		0	0		0	1		1	1		1
Taper Length (ft)	65			65			25			65		
Lane Util. Factor	1.00	0.91	0.91	1.00	0.91	0.91	1.00	1.00	1.00	1.00	0.95	0.95
Ped Bike Factor		1.00		1.00					0.99	1.00		
Fr _t		0.976			0.969				0.850		0.987	
Fl _t Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	5009	0	1805	4925	0	1805	1827	1615	1736	3499	0
Fl _t Permitted	0.410			0.421			0.494			0.341		
Satd. Flow (perm)	779	5009	0	799	4925	0	939	1827	1593	622	3499	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		48			81				38			16
Link Speed (mph)		35			35			30				30
Link Distance (ft)		932			422			332				1209
Travel Time (s)		18.2			8.2			7.5				27.5
Confl. Peds. (#/hr)			2	2					2	2		
Peak Hour Factor	0.80	0.88	0.72	0.59	0.92	0.82	0.74	0.74	0.50	0.76	0.86	0.80
Heavy Vehicles (%)	0%	1%	0%	0%	0%	10%	0%	4%	0%	4%	2%	0%
Adj. Flow (vph)	45	492	92	34	485	126	74	339	38	86	338	33
Shared Lane Traffic (%)												
Lane Group Flow (vph)	45	584	0	34	611	0	74	339	38	86	371	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2	1	1		2
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right	Left		Thru
Leading Detector (ft)	20	100		20	100		20	100	20	20		100
Trailing Detector (ft)	0	0		0	0		0	0	0	0		0
Detector 1 Position(ft)	0	0		0	0		0	0	0	0		0
Detector 1 Size(ft)	20	6		20	6		20	6	20	20		6
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0		0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0		0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0		0.0
Detector 2 Position(ft)		94			94			94				94
Detector 2 Size(ft)		6			6			6				6
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				0.0

Lanes, Volumes, Timings
 1011: Schaefer Hwy & Grand River Ave

08/08/2022

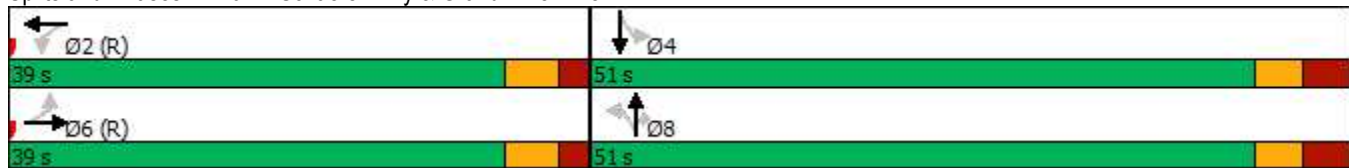


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA		Perm	NA		Perm	NA	Perm	Perm	NA	
Protected Phases		6			2			8				4
Permitted Phases	6			2			8		8	4		
Detector Phase	6	6		2	2		8	8	8	4		4
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		7.0	7.0	7.0	7.0		7.0
Minimum Split (s)	35.8	35.8		35.8	35.8		46.5	46.5	46.5	46.5		46.5
Total Split (s)	39.0	39.0		39.0	39.0		51.0	51.0	51.0	51.0		51.0
Total Split (%)	43.3%	43.3%		43.3%	43.3%		56.7%	56.7%	56.7%	56.7%		56.7%
Maximum Green (s)	33.2	33.2		33.2	33.2		44.5	44.5	44.5	44.5		44.5
Yellow Time (s)	3.6	3.6		3.6	3.6		3.2	3.2	3.2	3.2		3.2
All-Red Time (s)	2.2	2.2		2.2	2.2		3.3	3.3	3.3	3.3		3.3
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0		0.0
Total Lost Time (s)	5.8	5.8		5.8	5.8		6.5	6.5	6.5	6.5		6.5
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0		3.0
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None	None	None		None
Walk Time (s)	7.0	7.0		7.0	7.0		10.0	10.0	10.0	10.0		10.0
Flash Dont Walk (s)	23.0	23.0		23.0	23.0		30.0	30.0	30.0	30.0		30.0
Pedestrian Calls (#/hr)	0	0		0	0		0	0	0	0		0
Act Effct Green (s)	54.5	54.5		54.5	54.5		23.2	23.2	23.2	23.2		23.2
Actuated g/C Ratio	0.61	0.61		0.61	0.61		0.26	0.26	0.26	0.26		0.26
v/c Ratio	0.10	0.19		0.07	0.20		0.31	0.72	0.09	0.54		0.41
Control Delay	10.3	8.3		8.2	6.3		18.5	29.3	3.4	39.7		26.7
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.1	0.0	0.0		0.0
Total Delay	10.3	8.3		8.2	6.3		18.5	29.4	3.4	39.7		26.7
LOS	B	A		A	A		B	C	A	D		C
Approach Delay		8.4			6.4			25.4				29.1
Approach LOS		A			A			C				C

Intersection Summary

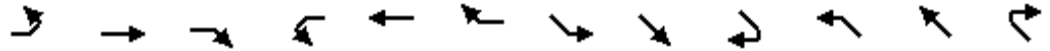
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 60 (67%), Referenced to phase 2:WBTL and 6:EBTL, Start of Green
 Natural Cycle: 85
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.72
 Intersection Signal Delay: 15.7
 Intersection LOS: B
 Intersection Capacity Utilization 65.9%
 ICU Level of Service C
 Analysis Period (min) 15

Splits and Phases: 1011: Schaefer Hwy & Grand River Ave



Lanes, Volumes, Timings
 1093: Grand River Ave & Jeffries Service Dr

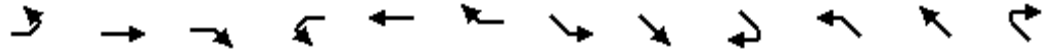
08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↑↑↑↑						↑↑↑↑			↑↑↑↑	
Traffic Volume (vph)	4	278	65	0	0	0	9	428	0	0	344	14
Future Volume (vph)	4	278	65	0	0	0	9	428	0	0	344	14
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.86	0.86	0.86	1.00	1.00	1.00	0.91	0.91	1.00	1.00	0.91	0.91
Ped Bike Factor								1.00			1.00	
Frt		0.972									0.993	
Flt Protected		0.999						0.998				
Satd. Flow (prot)	0	6296	0	0	0	0	0	5127	0	0	5034	0
Flt Permitted		0.999						0.917				
Satd. Flow (perm)	0	6296	0	0	0	0	0	4711	0	0	5034	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		71									9	
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		342			379			587			812	
Travel Time (s)		6.7			7.4			11.4			15.8	
Confl. Peds. (#/hr)							3					3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.56	0.96	0.92	0.92	0.71	0.65
Heavy Vehicles (%)	0%	1%	0%	2%	2%	2%	0%	1%	2%	2%	2%	8%
Adj. Flow (vph)	4	302	71	0	0	0	16	446	0	0	485	22
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	377	0	0	0	0	0	462	0	0	507	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2					1	2			2	
Detector Template	Left	Thru					Left	Thru			Thru	
Leading Detector (ft)	20	100					20	100			100	
Trailing Detector (ft)	0	0					0	0			0	
Detector 1 Position(ft)	0	0					0	0			0	
Detector 1 Size(ft)	20	6					20	6			6	
Detector 1 Type	Cl+Ex	Cl+Ex					Cl+Ex	Cl+Ex			Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0			0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0			0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0			0.0	
Detector 2 Position(ft)		94						94			94	
Detector 2 Size(ft)		6						6			6	
Detector 2 Type		Cl+Ex						Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA			NA	
Protected Phases		4						6			2	
Permitted Phases	4						6					

Lanes, Volumes, Timings
 1093: Grand River Ave & Jeffries Service Dr

08/08/2022

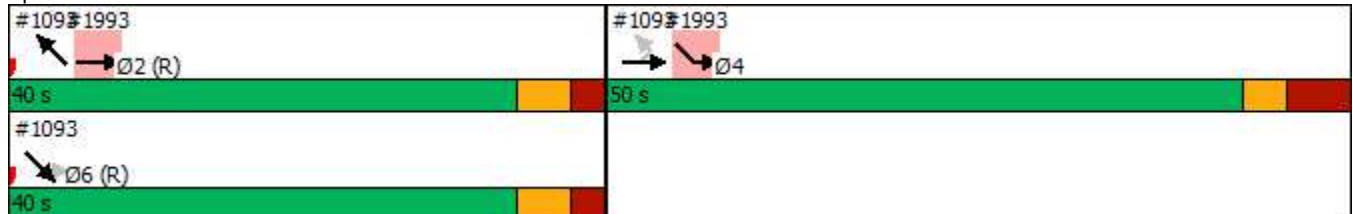


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Detector Phase	4	4					6	6				2
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0				10.0
Minimum Split (s)	49.4	49.4					37.0	37.0				37.0
Total Split (s)	50.0	50.0					40.0	40.0				40.0
Total Split (%)	55.6%	55.6%					44.4%	44.4%				44.4%
Maximum Green (s)	42.6	42.6					34.0	34.0				34.0
Yellow Time (s)	3.0	3.0					3.6	3.6				3.6
All-Red Time (s)	4.4	4.4					2.4	2.4				2.4
Lost Time Adjust (s)		0.0						0.0				0.0
Total Lost Time (s)		7.4						6.0				6.0
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0				3.0
Recall Mode	None	None					C-Max	C-Max				C-Max
Walk Time (s)	9.0	9.0					7.0	7.0				7.0
Flash Dont Walk (s)	33.0	33.0					24.0	24.0				24.0
Pedestrian Calls (#/hr)	0	0					0	0				0
Act Effct Green (s)		11.1						65.5				65.5
Actuated g/C Ratio		0.12						0.73				0.73
v/c Ratio		0.45						0.13				0.14
Control Delay		32.8						2.8				3.9
Queue Delay		0.0						0.0				0.0
Total Delay		32.8						2.8				3.9
LOS		C						A				A
Approach Delay		32.8						2.8				3.9
Approach LOS		C						A				A

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 71 (79%), Referenced to phase 2:NWT and 6:SETL, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.45
 Intersection Signal Delay: 11.6
 Intersection Capacity Utilization 45.3%
 Analysis Period (min) 15
 Intersection LOS: B
 ICU Level of Service A

Splits and Phases: 1093: Grand River Ave & Jeffries Service Dr



Lanes, Volumes, Timings

1193: Left Turn Bridge & Grand River Ave & I-96 Service Dr

08/08/2022



Lane Group	WBL2	WBL	WBT	WBR	SET	SER	SER2	NWL	NWT
Lane Configurations			↑↑↑	↑	↑↑↑	↑		↓	↑↑↑
Traffic Volume (vph)	24	6	195	268	414	100	3	48	301
Future Volume (vph)	24	6	195	268	414	100	3	48	301
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0		0		140		260	
Storage Lanes		0		1		1		1	
Taper Length (ft)		65						65	
Lane Util. Factor	0.91	0.91	0.86	0.86	0.86	0.86	0.91	1.00	0.91
Frt			0.944	0.850	0.996	0.850			
Flt Protected			0.996					0.950	
Satd. Flow (prot)	0	0	4544	1362	4834	1376	0	1805	5085
Flt Permitted			0.996					0.950	
Satd. Flow (perm)	0	0	4544	1362	4834	1376	0	1805	5085
Right Turn on Red				Yes			Yes		
Satd. Flow (RTOR)			146	145		126			
Link Speed (mph)			35		35				35
Link Distance (ft)			480		422				587
Travel Time (s)			9.4		8.2				11.4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.94	0.83	0.75	0.58	0.73
Heavy Vehicles (%)	2%	0%	1%	2%	1%	1%	0%	0%	2%
Adj. Flow (vph)	26	7	212	291	440	120	4	83	412
Shared Lane Traffic (%)				50%		10%			
Lane Group Flow (vph)	0	0	391	145	452	112	0	83	412
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right	Right	Left	Left
Median Width(ft)			0		12				12
Link Offset(ft)			0		0				0
Crosswalk Width(ft)			16		16				16
Two way Left Turn Lane									
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	15		9		9	9	15	
Number of Detectors	1	1	2	1	2	1		1	2
Detector Template	Left	Left	Thru	Right	Thru	Right		Left	Thru
Leading Detector (ft)	20	20	100	20	100	20		20	100
Trailing Detector (ft)	0	0	0	0	0	0		0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0
Detector 1 Size(ft)	20	20	6	20	6	20		20	6
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex
Detector 1 Channel									
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
Detector 2 Position(ft)			94		94				94
Detector 2 Size(ft)			6		6				6
Detector 2 Type			Cl+Ex		Cl+Ex				Cl+Ex
Detector 2 Channel									
Detector 2 Extend (s)			0.0		0.0				0.0
Turn Type	Perm	Perm	NA	Perm	NA	Perm		Prot	NA
Protected Phases			4		6			5	2

Lanes, Volumes, Timings

1193: Left Turn Bridge & Grand River Ave & I-96 Service Dr

08/08/2022

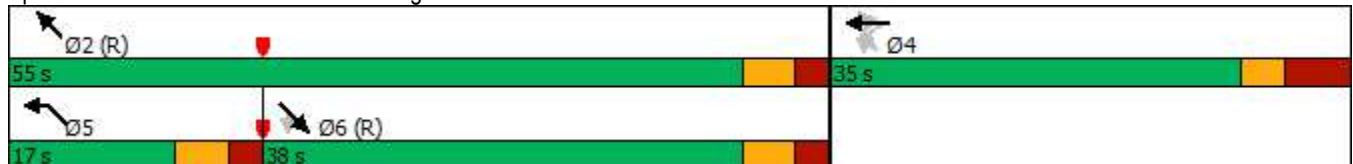


Lane Group	WBL2	WBL	WBT	WBR	SET	SER	SER2	NWL	NWT
Permitted Phases	4	4		4		6			
Detector Phase	4	4	4	4	6	6		5	2
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0		7.0	10.0
Minimum Split (s)	55.5	55.5	55.5	55.5	37.9	37.9		12.9	37.9
Total Split (s)	35.0	35.0	35.0	35.0	38.0	38.0		17.0	55.0
Total Split (%)	38.9%	38.9%	38.9%	38.9%	42.2%	42.2%		18.9%	61.1%
Maximum Green (s)	27.5	27.5	27.5	27.5	32.1	32.1		11.1	49.1
Yellow Time (s)	3.0	3.0	3.0	3.0	3.6	3.6		3.6	3.6
All-Red Time (s)	4.5	4.5	4.5	4.5	2.3	2.3		2.3	2.3
Lost Time Adjust (s)			0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)			7.5	7.5	5.9	5.9		5.9	5.9
Lead/Lag					Lag	Lag		Lead	
Lead-Lag Optimize?					Yes	Yes		Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max		None	C-Max
Walk Time (s)	9.0	9.0	9.0	9.0	7.0	7.0			7.0
Flash Dont Walk (s)	39.0	39.0	39.0	39.0	25.0	25.0			25.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0			0
Act Effct Green (s)			11.9	11.9	51.7	51.7		9.7	64.7
Actuated g/C Ratio			0.13	0.13	0.57	0.57		0.11	0.72
v/c Ratio			0.54	0.48	0.16	0.13		0.43	0.11
Control Delay			25.1	11.7	11.3	6.1		40.3	0.9
Queue Delay			0.0	0.0	0.0	0.0		0.0	0.0
Total Delay			25.1	11.7	11.3	6.1		40.3	0.9
LOS			C	B	B	A		D	A
Approach Delay			21.5		10.3				7.5
Approach LOS			C		B				A

Intersection Summary

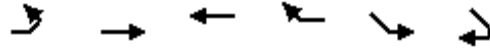
Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	86 (96%), Referenced to phase 2:NWT and 6:SET, Start of Green
Natural Cycle:	110
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.54
Intersection Signal Delay:	13.2
Intersection LOS:	B
Intersection Capacity Utilization	39.0%
ICU Level of Service	A
Analysis Period (min)	15

Splits and Phases: 1193: Left Turn Bridge & Grand River Ave & I-96 Service Dr



Lanes, Volumes, Timings
 1993: Jeffries Service Dr & Left Turn Bridge

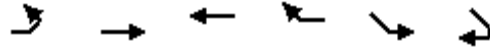
08/08/2022



Lane Group	EBL	EBT	WBT	WBR	SEL	SER	Ø6
Lane Configurations		↑↑↑			↑↑		
Traffic Volume (vph)	0	237	0	0	106	0	
Future Volume (vph)	0	237	0	0	106	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	0.91	1.00	1.00	0.97	1.00	
Frt							
Flt Protected					0.950		
Satd. Flow (prot)	0	5085	0	0	3433	0	
Flt Permitted					0.950		
Satd. Flow (perm)	0	5085	0	0	3433	0	
Right Turn on Red				Yes	Yes	Yes	
Satd. Flow (RTOR)					878		
Link Speed (mph)		35	35		30		
Link Distance (ft)		191	342		422		
Travel Time (s)		3.7	6.7		9.6		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	258	0	0	115	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	258	0	0	115	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		0	0		24		
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		16	16		16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15			9	15	9	
Number of Detectors		2			1		
Detector Template		Thru			Left		
Leading Detector (ft)		100			20		
Trailing Detector (ft)		0			0		
Detector 1 Position(ft)		0			0		
Detector 1 Size(ft)		6			20		
Detector 1 Type		Cl+Ex			Cl+Ex		
Detector 1 Channel							
Detector 1 Extend (s)		0.0			0.0		
Detector 1 Queue (s)		0.0			0.0		
Detector 1 Delay (s)		0.0			0.0		
Detector 2 Position(ft)		94					
Detector 2 Size(ft)		6					
Detector 2 Type		Cl+Ex					
Detector 2 Channel							
Detector 2 Extend (s)		0.0					
Turn Type		NA			Prot		
Protected Phases		2			4	6	
Permitted Phases							
Detector Phase		2			4		
Switch Phase							
Minimum Initial (s)		10.0			10.0	10.0	

Lanes, Volumes, Timings
 1993: Jeffries Service Dr & Left Turn Bridge

08/08/2022

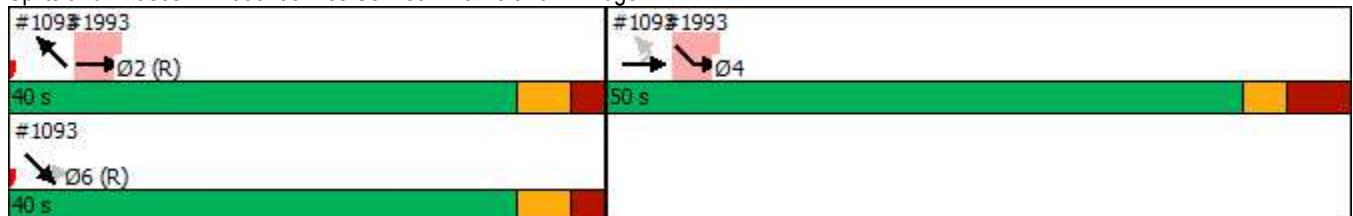


Lane Group	EBL	EBT	WBT	WBR	SEL	SER	Ø6
Minimum Split (s)		37.0			49.4		37.0
Total Split (s)		40.0			50.0		40.0
Total Split (%)		44.4%			55.6%		44%
Maximum Green (s)		34.0			42.6		34.0
Yellow Time (s)		3.6			3.0		3.6
All-Red Time (s)		2.4			4.4		2.4
Lost Time Adjust (s)		0.0			0.0		
Total Lost Time (s)		6.0			7.4		
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)		3.0			3.0		3.0
Recall Mode		C-Max			None		C-Max
Walk Time (s)		7.0			9.0		7.0
Flash Dont Walk (s)		24.0			33.0		24.0
Pedestrian Calls (#/hr)		0			0		0
Act Effct Green (s)		65.5			11.1		
Actuated g/C Ratio		0.73			0.12		
v/c Ratio		0.07			0.10		
Control Delay		2.6			0.5		
Queue Delay		0.0			0.0		
Total Delay		2.6			0.5		
LOS		A			A		
Approach Delay		2.6			0.5		
Approach LOS		A			A		

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 71 (79%), Referenced to phase 2:NWT and 6:SETL, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.45
 Intersection Signal Delay: 2.0
 Intersection LOS: A
 Intersection Capacity Utilization 27.8%
 ICU Level of Service A
 Analysis Period (min) 15

Splits and Phases: 1993: Jeffries Service Dr & Left Turn Bridge



Lanes, Volumes, Timings
2609: Schaefer Hwy & Schoolcraft Rd

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	23	89	4	24	94	34	12	332	45	39	353	30
Future Volume (vph)	23	89	4	24	94	34	12	332	45	39	353	30
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	60		0	60		0	80		250	80		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00	1.00		0.99	0.99		1.00	1.00		0.99	1.00	
Frt		0.988			0.963			0.978			0.984	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1228	1839	0	1805	1735	0	1805	1745	0	1671	1788	0
Flt Permitted	0.658			0.660			0.382			0.388		
Satd. Flow (perm)	849	1839	0	1243	1735	0	725	1745	0	679	1788	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		5			20			13			9	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		699			1470			1209			215	
Travel Time (s)		15.9			33.4			27.5			4.9	
Confl. Peds. (#/hr)	1		6	6		1	1		6	6		1
Peak Hour Factor	0.53	0.63	0.33	0.72	0.80	0.89	0.75	0.91	0.72	0.88	0.90	0.63
Heavy Vehicles (%)	47%	2%	0%	0%	1%	17%	0%	7%	0%	8%	2%	23%
Adj. Flow (vph)	43	141	12	33	118	38	16	365	63	44	392	48
Shared Lane Traffic (%)												
Lane Group Flow (vph)	43	153	0	33	156	0	16	428	0	44	440	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	

Lanes, Volumes, Timings
 2609: Schaefer Hwy & Schoolcraft Rd

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		
Detector Phase	8	8		4	4		1	6		5	2	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0		5.0	9.0		5.0	9.0	
Minimum Split (s)	28.8	28.8		28.8	28.8		10.6	26.6		10.6	26.6	
Total Split (s)	29.1	29.1		29.1	29.1		10.8	39.9		11.0	40.1	
Total Split (%)	36.4%	36.4%		36.4%	36.4%		13.5%	49.9%		13.8%	50.1%	
Maximum Green (s)	22.3	22.3		22.3	22.3		5.2	33.3		5.4	33.5	
Yellow Time (s)	3.8	3.8		3.8	3.8		3.5	3.6		3.5	3.6	
All-Red Time (s)	3.0	3.0		3.0	3.0		2.1	3.0		2.1	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.8	6.8		6.8	6.8		5.6	6.6		5.6	6.6	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	Max	Max		Max	Max		Max	C-Max		Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0			13.0			13.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	22.3	22.3		22.3	22.3		39.5	33.3		39.9	33.5	
Actuated g/C Ratio	0.28	0.28		0.28	0.28		0.49	0.42		0.50	0.42	
v/c Ratio	0.18	0.30		0.10	0.31		0.04	0.58		0.11	0.58	
Control Delay	24.4	23.8		22.4	21.9		8.1	21.4		8.6	21.4	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	24.4	23.8		22.4	21.9		8.1	21.4		8.6	21.4	
LOS	C	C		C	C		A	C		A	C	
Approach Delay		24.0			21.9			20.9			20.2	
Approach LOS		C			C			C			C	

Intersection Summary

Area Type: Other
 Cycle Length: 80
 Actuated Cycle Length: 80
 Offset: 0 (0%), Referenced to phase 2:SBTL and 6:NBTL, Start of Green
 Natural Cycle: 70
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.58
 Intersection Signal Delay: 21.3 Intersection LOS: C
 Intersection Capacity Utilization 59.5% ICU Level of Service B
 Analysis Period (min) 15

Splits and Phases: 2609: Schaefer Hwy & Schoolcraft Rd



Lanes, Volumes, Timings
 3011: Schaefer Hwy & I-96 Service Dr

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				↙	↕		↘	↕			↕	↘
Traffic Volume (vph)	0	0	0	98	145	3	148	321	0	0	210	166
Future Volume (vph)	0	0	0	98	145	3	148	321	0	0	210	166
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	90		0	0		0
Storage Lanes	0		0	1		0	1		0	0		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95
Ped Bike Factor							1.00					0.99
Fr _t					0.997							0.932
Fl _t Protected				0.950			0.950					
Satd. Flow (prot)	0	0	0	1787	3564	0	1752	3471	0	0	3283	0
Fl _t Permitted				0.950			0.486					
Satd. Flow (perm)	0	0	0	1787	3564	0	894	3471	0	0	3283	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					2							205
Link Speed (mph)		35			35			30				30
Link Distance (ft)		298			375			384				332
Travel Time (s)		5.8			7.3			8.7				7.5
Confl. Peds. (#/hr)							4					4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.87	0.76	0.92	0.92	0.84	0.81
Heavy Vehicles (%)	2%	2%	2%	1%	1%	0%	3%	4%	2%	2%	3%	0%
Adj. Flow (vph)	0	0	0	107	158	3	170	422	0	0	250	205
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	107	161	0	170	422	0	0	455	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors				1	2		1	2				2
Detector Template				Left	Thru		Left	Thru				Thru
Leading Detector (ft)				20	100		20	100				100
Trailing Detector (ft)				0	0		0	0				0
Detector 1 Position(ft)				0	0		0	0				0
Detector 1 Size(ft)				20	6		20	6				6
Detector 1 Type				Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex				Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)				0.0	0.0		0.0	0.0				0.0
Detector 1 Queue (s)				0.0	0.0		0.0	0.0				0.0
Detector 1 Delay (s)				0.0	0.0		0.0	0.0				0.0
Detector 2 Position(ft)					94			94				94
Detector 2 Size(ft)					6			6				6
Detector 2 Type					Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)					0.0			0.0				0.0

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	

Lanes, Volumes, Timings
 3011: Schaefer Hwy & I-96 Service Dr

08/08/2022

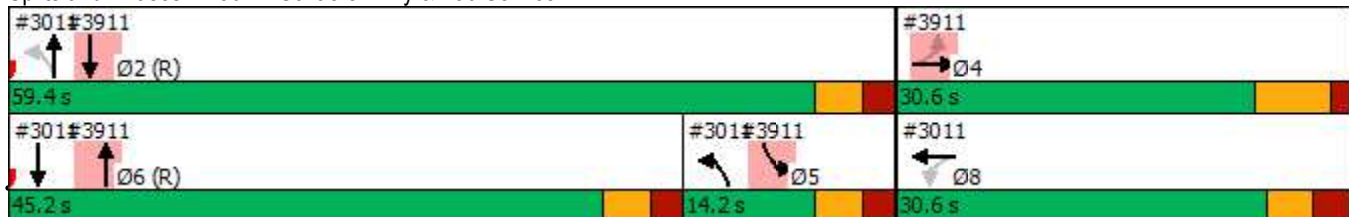


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type				Perm	NA		pm+pt	NA				NA
Protected Phases					8		5	2				6
Permitted Phases				8			2					
Detector Phase				8	8		5	2				6
Switch Phase												
Minimum Initial (s)				10.0	10.0		7.0	10.0				10.0
Minimum Split (s)				28.7	28.7		12.4	23.4				23.4
Total Split (s)				30.6	30.6		14.2	59.4				45.2
Total Split (%)				34.0%	34.0%		15.8%	66.0%				50.2%
Maximum Green (s)				24.9	24.9		8.8	54.0				39.8
Yellow Time (s)				3.0	3.0		3.2	3.2				3.2
All-Red Time (s)				2.7	2.7		2.2	2.2				2.2
Lost Time Adjust (s)				0.0	0.0		0.0	0.0				0.0
Total Lost Time (s)				5.7	5.7		5.4	5.4				5.4
Lead/Lag							Lag					Lead
Lead-Lag Optimize?							Yes					Yes
Vehicle Extension (s)				3.0	3.0		3.0	3.0				3.0
Recall Mode				None	None		None	C-Max				C-Max
Walk Time (s)				7.0	7.0			7.0				7.0
Flash Dont Walk (s)				16.0	16.0			11.0				11.0
Pedestrian Calls (#/hr)				0	0			0				0
Act Effct Green (s)				12.5	12.5		66.4	66.4				52.2
Actuated g/C Ratio				0.14	0.14		0.74	0.74				0.58
v/c Ratio				0.43	0.32		0.23	0.16				0.23
Control Delay				21.7	17.3		2.7	1.9				2.6
Queue Delay				0.0	0.0		0.0	0.0				0.0
Total Delay				21.7	17.3		2.7	1.9				2.6
LOS				C	B		A	A				A
Approach Delay					19.0			2.2				2.6
Approach LOS					B			A				A

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 87 (97%), Referenced to phase 2:NBTL and 6:SBT, Start of Green
 Natural Cycle: 75
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.58
 Intersection Signal Delay: 5.7
 Intersection LOS: A
 Intersection Capacity Utilization 50.8%
 ICU Level of Service A
 Analysis Period (min) 15

Splits and Phases: 3011: Schaefer Hwy & I-96 Service Dr



Lane Group	Ø4
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
Minimum Split (s)	30.5
Total Split (s)	30.6
Total Split (%)	34%
Maximum Green (s)	24.1
Yellow Time (s)	5.0
All-Red Time (s)	1.5
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	17.0
Pedestrian Calls (#/hr)	0
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Lanes, Volumes, Timings
3911: Schaefer Hwy & Jeffries Service Dr

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕↕↕						↔		↕	↕↕	
Traffic Volume (vph)	145	107	155	0	0	0	0	323	81	49	259	0
Future Volume (vph)	145	107	155	0	0	0	0	323	81	49	259	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	0		0	90		0
Storage Lanes	0		0	0		0	0		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	0.91	0.91	0.91	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00
Ped Bike Factor								1.00		1.00		
Frt		0.943						0.972				
Flt Protected		0.982								0.950		
Satd. Flow (prot)	0	4731	0	0	0	0	0	1766	0	1770	3505	0
Flt Permitted		0.982								0.950		
Satd. Flow (perm)	0	4731	0	0	0	0	0	1766	0	1765	3505	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		168						19				
Link Speed (mph)		35			35			30				30
Link Distance (ft)		417			354			621				384
Travel Time (s)		8.1			6.9			14.1				8.7
Confl. Peds. (#/hr)									4	4		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.75	0.72	0.81	0.79	0.92
Heavy Vehicles (%)	1%	3%	1%	2%	2%	2%	2%	5%	1%	2%	3%	2%
Adj. Flow (vph)	158	116	168	0	0	0	0	431	113	60	328	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	442	0	0	0	0	0	544	0	60	328	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2						2		1	2	
Detector Template	Left	Thru						Thru		Left	Thru	
Leading Detector (ft)	20	100						100		20	100	
Trailing Detector (ft)	0	0						0		0	0	
Detector 1 Position(ft)	0	0						0		0	0	
Detector 1 Size(ft)	20	6						6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex						Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0						0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0						0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0						0.0		0.0	0.0	
Detector 2 Position(ft)		94						94			94	
Detector 2 Size(ft)		6						6			6	
Detector 2 Type		Cl+Ex						Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	

Lane Group	Ø8
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	

Lanes, Volumes, Timings
 3911: Schaefer Hwy & Jeffries Service Dr

08/08/2022

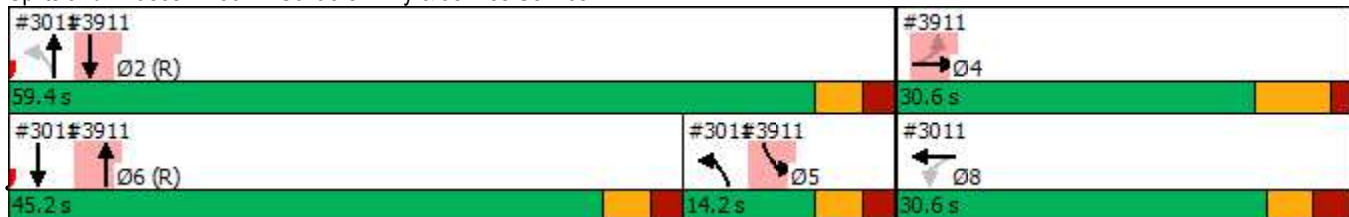


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA						NA		Prot	NA	
Protected Phases		4						6		5	2	
Permitted Phases	4											
Detector Phase	4	4						6		5	2	
Switch Phase												
Minimum Initial (s)	10.0	10.0						10.0		7.0	10.0	
Minimum Split (s)	30.5	30.5						23.4		12.4	23.4	
Total Split (s)	30.6	30.6						45.2		14.2	59.4	
Total Split (%)	34.0%	34.0%						50.2%		15.8%	66.0%	
Maximum Green (s)	24.1	24.1						39.8		8.8	54.0	
Yellow Time (s)	5.0	5.0						3.2		3.2	3.2	
All-Red Time (s)	1.5	1.5						2.2		2.2	2.2	
Lost Time Adjust (s)		0.0						0.0		0.0	0.0	
Total Lost Time (s)		6.5						5.4		5.4	5.4	
Lead/Lag								Lead		Lag		
Lead-Lag Optimize?								Yes		Yes		
Vehicle Extension (s)	3.0	3.0						3.0		3.0	3.0	
Recall Mode	None	None						C-Max		None	C-Max	
Walk Time (s)	7.0	7.0						7.0			7.0	
Flash Dont Walk (s)	17.0	17.0						11.0			11.0	
Pedestrian Calls (#/hr)	0	0						0			0	
Act Effct Green (s)		11.7						52.2		8.8	66.4	
Actuated g/C Ratio		0.13						0.58		0.10	0.74	
v/c Ratio		0.58						0.53		0.35	0.13	
Control Delay		25.5						13.7		43.5	3.6	
Queue Delay		0.0						0.0		0.0	0.0	
Total Delay		25.5						13.7		43.5	3.6	
LOS		C						B		D	A	
Approach Delay		25.5						13.7			9.8	
Approach LOS		C						B			A	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 87 (97%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 75
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.58
 Intersection Signal Delay: 16.4
 Intersection LOS: B
 Intersection Capacity Utilization 50.8%
 ICU Level of Service A
 Analysis Period (min) 15

Splits and Phases: 3911: Schaefer Hwy & Jeffries Service Dr



Lane Group	Ø8
Turn Type	
Protected Phases	8
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
Minimum Split (s)	28.7
Total Split (s)	30.6
Total Split (%)	34%
Maximum Green (s)	24.9
Yellow Time (s)	3.0
All-Red Time (s)	2.7
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	16.0
Pedestrian Calls (#/hr)	0
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Lanes, Volumes, Timings
4001: Schaefer Hwy & Lyndon St

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	30	84	27	46	106	24	32	316	45	32	312	26
Future Volume (vph)	30	84	27	46	106	24	32	316	45	32	312	26
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	80		0	80		0	80		0	80		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	1.00	0.99		1.00	1.00		1.00	1.00		1.00	1.00	
Fr _t		0.956			0.975			0.979			0.981	
Fl _t Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1807	0	1805	1848	0	1805	3365	0	1752	3245	0
Fl _t Permitted	0.662			0.674			0.520			0.514		
Satd. Flow (perm)	1256	1807	0	1275	1848	0	983	3365	0	947	3245	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		37			18			34			29	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		648			545			1423			685	
Travel Time (s)		14.7			12.4			32.3			15.6	
Confl. Peds. (#/hr)	2		6	6		2	6		1	1		6
Peak Hour Factor	0.73	0.92	0.72	0.73	0.85	0.96	0.68	0.91	0.77	0.60	0.91	0.52
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	5%	3%	3%	10%	0%
Adj. Flow (vph)	41	91	38	63	125	25	47	347	58	53	343	50
Shared Lane Traffic (%)												
Lane Group Flow (vph)	41	129	0	63	150	0	47	405	0	53	393	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	

Lanes, Volumes, Timings
4001: Schaefer Hwy & Lyndon St

08/08/2022

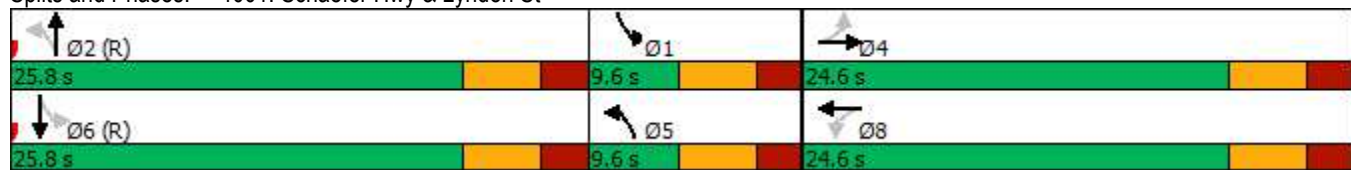


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		4.0	15.0		4.0	15.0	
Minimum Split (s)	24.6	24.6		24.6	24.6		9.6	26.6		9.6	26.6	
Total Split (s)	24.6	24.6		24.6	24.6		9.6	25.8		9.6	25.8	
Total Split (%)	41.0%	41.0%		41.0%	41.0%		16.0%	43.0%		16.0%	43.0%	
Maximum Green (s)	19.0	19.0		19.0	19.0		4.0	20.2		4.0	20.2	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	2.1	2.1		2.1	2.1		2.1	2.1		2.1	2.1	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.6	5.6		5.6	5.6		5.6	5.6		5.6	5.6	
Lead/Lag							Lag	Lead		Lag	Lead	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	Min	Min		Min	Min		None	C-Max		None	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	12.0	12.0		12.0	12.0			14.0			14.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	10.9	10.9		10.9	10.9		36.1	32.1		36.1	32.1	
Actuated g/C Ratio	0.18	0.18		0.18	0.18		0.60	0.54		0.60	0.54	
v/c Ratio	0.18	0.36		0.27	0.43		0.07	0.22		0.09	0.22	
Control Delay	22.2	18.5		23.9	22.8		5.2	8.4		5.3	8.5	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	22.2	18.5		23.9	22.8		5.2	8.4		5.3	8.5	
LOS	C	B		C	C		A	A		A	A	
Approach Delay		19.4			23.1			8.0			8.1	
Approach LOS		B			C			A			A	

Intersection Summary

Area Type: Other
 Cycle Length: 60
 Actuated Cycle Length: 60
 Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 65
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.43
 Intersection Signal Delay: 12.1
 Intersection LOS: B
 Intersection Capacity Utilization 57.5%
 ICU Level of Service B
 Analysis Period (min) 15

Splits and Phases: 4001: Schaefer Hwy & Lyndon St



Intersection												
Int Delay, s/veh	0.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	8	161	4	0	144	0	0	0	0	0	0	8
Future Vol, veh/h	8	161	4	0	144	0	0	0	0	0	0	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	25	25	25	25	25	25	25	25	25	25	25	25
Heavy Vehicles, %	0	2	0	0	5	0	0	0	0	0	0	0
Mvmt Flow	32	644	16	0	576	0	0	0	0	0	0	32

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	576	0	0	660	0	0	1308	1292	652	1292	1300	576
Stage 1	-	-	-	-	-	-	716	716	-	576	576	-
Stage 2	-	-	-	-	-	-	592	576	-	716	724	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1007	-	-	938	-	-	138	165	471	141	163	521
Stage 1	-	-	-	-	-	-	424	437	-	506	505	-
Stage 2	-	-	-	-	-	-	496	505	-	424	433	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1007	-	-	938	-	-	125	157	471	136	155	521
Mov Cap-2 Maneuver	-	-	-	-	-	-	125	157	-	136	155	-
Stage 1	-	-	-	-	-	-	403	415	-	481	505	-
Stage 2	-	-	-	-	-	-	466	505	-	403	411	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.4			0			0			12.4		
HCM LOS							A			B		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	1007	-	-	938	-	-	521
HCM Lane V/C Ratio	-	0.032	-	-	-	-	-	0.061
HCM Control Delay (s)	-	0	8.7	0	-	0	-	12.4
HCM Lane LOS	-	A	A	A	-	A	-	B
HCM 95th %tile Q(veh)	-	0.1	-	-	0	-	-	0.2

Intersection												
Int Delay, s/veh	0.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	2	1	6	3	1	7	4	208	17	5	246	3
Future Vol, veh/h	2	1	6	3	1	7	4	208	17	5	246	3
Conflicting Peds, #/hr	0	0	1	1	0	0	2	0	0	0	0	2
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	50	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	60	60	60	60	60	60	100	81	60	60	95	75
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	3	2	10	5	2	12	4	257	28	8	259	4

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	565	572	264	563	560	271	265	0	0	285	0	0
Stage 1	279	279	-	279	279	-	-	-	-	-	-	-
Stage 2	286	293	-	284	281	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	439	433	780	440	440	773	1311	-	-	1289	-	-
Stage 1	732	683	-	732	683	-	-	-	-	-	-	-
Stage 2	726	674	-	727	682	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	427	428	778	429	435	773	1309	-	-	1289	-	-
Mov Cap-2 Maneuver	427	428	-	429	435	-	-	-	-	-	-	-
Stage 1	728	678	-	730	681	-	-	-	-	-	-	-
Stage 2	711	672	-	711	677	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	11		11.2		0.1		0.2	
HCM LOS	B		B					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1309	-	-	611	600	1289	-
HCM Lane V/C Ratio	0.003	-	-	0.025	0.031	0.006	-
HCM Control Delay (s)	7.8	-	-	11	11.2	7.8	-
HCM Lane LOS	A	-	-	B	B	A	-
HCM 95th %tile Q(veh)	0	-	-	0.1	0.1	0	-

Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↘	↗	↑↓			↑↓
Traffic Vol, veh/h	1	0	383	6	5	421
Future Vol, veh/h	1	0	383	6	5	421
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	100	100	2	100	100	2
Mvmt Flow	1	0	416	7	5	458

Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	659	212	0	0	423	0
Stage 1	420	-	-	-	-	-
Stage 2	239	-	-	-	-	-
Critical Hdwy	8.8	8.9	-	-	6.1	-
Critical Hdwy Stg 1	7.8	-	-	-	-	-
Critical Hdwy Stg 2	7.8	-	-	-	-	-
Follow-up Hdwy	4.5	4.3	-	-	3.2	-
Pot Cap-1 Maneuver	235	561	-	-	659	-
Stage 1	414	-	-	-	-	-
Stage 2	551	-	-	-	-	-
Platoon blocked, %			-	-	-	-
Mov Cap-1 Maneuver	233	561	-	-	659	-
Mov Cap-2 Maneuver	233	-	-	-	-	-
Stage 1	414	-	-	-	-	-
Stage 2	545	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	20.5	0	0.2
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	233	-	659
HCM Lane V/C Ratio	-	-	0.005	-	0.008
HCM Control Delay (s)	-	-	20.5	0	10.5
HCM Lane LOS	-	-	C	A	B
HCM 95th %tile Q(veh)	-	-	0	-	0

Intersection						
Int Delay, s/veh	0.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	41	11	383	0	0	385
Future Vol, veh/h	41	11	383	0	0	385
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	6	0	0	4
Mvmt Flow	45	12	416	0	0	418

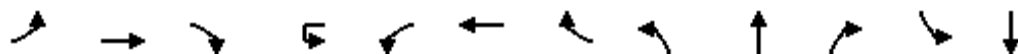
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	625	208	0	0	416	0
Stage 1	416	-	-	-	-	-
Stage 2	209	-	-	-	-	-
Critical Hdwy	6.8	6.9	-	-	4.1	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	422	804	-	-	1154	-
Stage 1	640	-	-	-	-	-
Stage 2	812	-	-	-	-	-
Platoon blocked, %			-	-	-	-
Mov Cap-1 Maneuver	422	804	-	-	1154	-
Mov Cap-2 Maneuver	422	-	-	-	-	-
Stage 1	640	-	-	-	-	-
Stage 2	812	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	13.4	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	422	804	1154
HCM Lane V/C Ratio	-	-	0.106	0.015	-
HCM Control Delay (s)	-	-	14.5	9.5	0
HCM Lane LOS	-	-	B	A	A
HCM 95th %tile Q(veh)	-	-	0.4	0	0

Lanes, Volumes, Timings
1011: Schaefer Hwy & Grand River Ave

08/08/2022

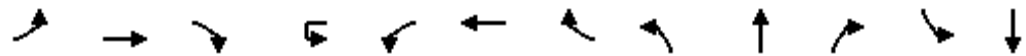


Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations	↙	↑↑↑			↙	↑↑↑		↙	↑	↗	↙	↑↑
Traffic Volume (vph)	79	564	122	1	33	631	178	96	419	13	85	481
Future Volume (vph)	79	564	122	1	33	631	178	96	419	13	85	481
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	115		0		95		0	155		0	80	
Storage Lanes	1		0		0		0	1		1	1	
Taper Length (ft)	65				65			25			65	
Lane Util. Factor	1.00	0.91	0.91	0.91	1.00	0.91	0.91	1.00	1.00	1.00	1.00	0.95
Ped Bike Factor	1.00	1.00			1.00	0.99		1.00		0.99	1.00	1.00
Frt		0.971				0.962				0.850		0.985
Flt Protected	0.950				0.950			0.950			0.950	
Satd. Flow (prot)	1805	4943	0	0	1805	4798	0	1805	1792	1615	1583	3334
Flt Permitted	0.280				0.336			0.358			0.261	
Satd. Flow (perm)	531	4943	0	0	638	4798	0	680	1792	1593	435	3334
Right Turn on Red			Yes				Yes			Yes		
Satd. Flow (RTOR)		74				112				34		18
Link Speed (mph)		35				35			30			30
Link Distance (ft)		932				427			332			1209
Travel Time (s)		18.2				8.3			7.5			27.5
Confl. Peds. (#/hr)	5		3		3		5	3		2	2	
Peak Hour Factor	0.71	0.92	0.83	0.25	0.78	0.94	0.78	0.83	0.89	0.63	0.81	0.93
Heavy Vehicles (%)	0%	2%	0%	0%	0%	2%	7%	0%	6%	0%	14%	7%
Adj. Flow (vph)	111	613	147	4	42	671	228	116	471	21	105	517
Shared Lane Traffic (%)												
Lane Group Flow (vph)	111	760	0	0	46	899	0	116	471	21	105	575
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	R NA	Left	Left	Right	Left	Left	Right	Left	Left
Median Width(ft)		12				12			12			12
Link Offset(ft)		0				0			0			0
Crosswalk Width(ft)		16				16			16			16
Two way Left Turn Lane												Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	9	15		9	15		9	15	
Number of Detectors	1	2		1	1	2		1	2	1	1	2
Detector Template	Left	Thru		Left	Left	Thru		Left	Thru	Right	Left	Thru
Leading Detector (ft)	20	100		20	20	100		20	100	20	20	100
Trailing Detector (ft)	0	0		0	0	0		0	0	0	0	0
Detector 1 Position(ft)	0	0		0	0	0		0	0	0	0	0
Detector 1 Size(ft)	20	6		20	20	6		20	6	20	20	6
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)		94				94			94			94
Detector 2 Size(ft)		6				6			6			6
Detector 2 Type		Cl+Ex				Cl+Ex			Cl+Ex			Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0				0.0			0.0			0.0

Lane Group	SBR
Lane Configurations	
Traffic Volume (vph)	53
Future Volume (vph)	53
Ideal Flow (vphpl)	1900
Storage Length (ft)	240
Storage Lanes	1
Taper Length (ft)	
Lane Util. Factor	0.95
Ped Bike Factor	
Flt	
Flt Protected	
Satd. Flow (prot)	0
Flt Permitted	
Satd. Flow (perm)	0
Right Turn on Red	Yes
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	3
Peak Hour Factor	0.91
Heavy Vehicles (%)	2%
Adj. Flow (vph)	58
Shared Lane Traffic (%)	
Lane Group Flow (vph)	0
Enter Blocked Intersection	No
Lane Alignment	Right
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	1.00
Turning Speed (mph)	9
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	

Lanes, Volumes, Timings
1011: Schaefer Hwy & Grand River Ave

08/08/2022

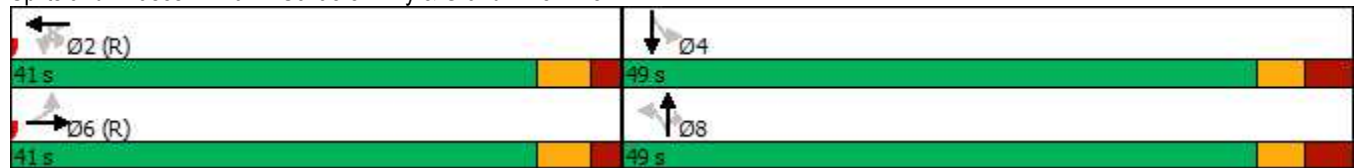


Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Turn Type	Perm	NA		Perm	Perm	NA		Perm	NA	Perm	Perm	NA
Protected Phases		6				2			8			4
Permitted Phases	6			2	2			8		8	4	
Detector Phase	6	6		2	2	2		8	8	8	4	4
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0		7.0	7.0	7.0	7.0	7.0
Minimum Split (s)	35.8	35.8		35.8	35.8	35.8		46.5	46.5	46.5	46.5	46.5
Total Split (s)	41.0	41.0		41.0	41.0	41.0		49.0	49.0	49.0	49.0	49.0
Total Split (%)	45.6%	45.6%		45.6%	45.6%	45.6%		54.4%	54.4%	54.4%	54.4%	54.4%
Maximum Green (s)	35.2	35.2		35.2	35.2	35.2		42.5	42.5	42.5	42.5	42.5
Yellow Time (s)	3.6	3.6		3.6	3.6	3.6		3.2	3.2	3.2	3.2	3.2
All-Red Time (s)	2.2	2.2		2.2	2.2	2.2		3.3	3.3	3.3	3.3	3.3
Lost Time Adjust (s)	0.0	0.0			0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.8	5.8			5.8	5.8		6.5	6.5	6.5	6.5	6.5
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max		C-Max	C-Max	C-Max		None	None	None	None	None
Walk Time (s)	7.0	7.0		7.0	7.0	7.0		10.0	10.0	10.0	10.0	10.0
Flash Dont Walk (s)	23.0	23.0		23.0	23.0	23.0		30.0	30.0	30.0	30.0	30.0
Pedestrian Calls (#/hr)	0	0		0	0	0		0	0	0	0	0
Act Effct Green (s)	46.9	46.9			46.9	46.9		30.8	30.8	30.8	30.8	30.8
Actuated g/C Ratio	0.52	0.52			0.52	0.52		0.34	0.34	0.34	0.34	0.34
v/c Ratio	0.40	0.29			0.14	0.35		0.50	0.77	0.04	0.71	0.50
Control Delay	21.7	12.4			12.6	9.8		17.8	22.8	2.2	49.7	23.3
Queue Delay	0.0	0.0			0.0	0.0		0.0	0.2	0.0	0.0	0.0
Total Delay	21.7	12.4			12.6	9.8		17.8	22.9	2.2	49.7	23.3
LOS	C	B			B	A		B	C	A	D	C
Approach Delay		13.5				9.9			21.2			27.4
Approach LOS		B				A			C			C

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 66 (73%), Referenced to phase 2:WBTL and 6:EBTL, Start of Green
 Natural Cycle: 85
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.77
 Intersection Signal Delay: 17.0
 Intersection LOS: B
 Intersection Capacity Utilization 82.4%
 ICU Level of Service E
 Analysis Period (min) 15

Splits and Phases: 1011: Schaefer Hwy & Grand River Ave

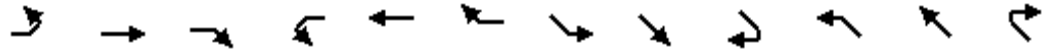




Lane Group	SBR
Turn Type	
Protected Phases	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	
Minimum Split (s)	
Total Split (s)	
Total Split (%)	
Maximum Green (s)	
Yellow Time (s)	
All-Red Time (s)	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	
Recall Mode	
Walk Time (s)	
Flash Dont Walk (s)	
Pedestrian Calls (#/hr)	
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Lanes, Volumes, Timings
1093: Grand River Ave & Jeffries Service Dr

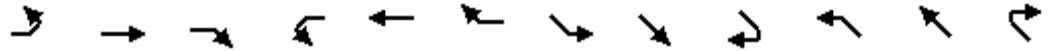
08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↑↑↑↑						↑↑↑↑			↑↑↑↑	
Traffic Volume (vph)	8	389	83	0	0	0	6	568	0	0	518	24
Future Volume (vph)	8	389	83	0	0	0	6	568	0	0	518	24
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.86	0.86	0.86	1.00	1.00	1.00	0.91	0.91	1.00	1.00	0.91	0.91
Ped Bike Factor								1.00			1.00	
Frt		0.974									0.986	
Flt Protected		0.999						0.999				
Satd. Flow (prot)	0	5929	0	0	0	0	0	5035	0	0	4995	0
Flt Permitted		0.999						0.914				
Satd. Flow (perm)	0	5929	0	0	0	0	0	4607	0	0	4995	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		88									20	
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		329			379			582			812	
Travel Time (s)		6.4			7.4			11.3			15.8	
Confl. Peds. (#/hr)							1					1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.31	0.91	0.92	0.92	0.92	0.43
Heavy Vehicles (%)	2%	7%	9%	2%	2%	2%	0%	3%	2%	2%	2%	5%
Adj. Flow (vph)	9	423	90	0	0	0	19	624	0	0	563	56
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	522	0	0	0	0	0	643	0	0	619	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2					1	2			2	
Detector Template	Left	Thru					Left	Thru			Thru	
Leading Detector (ft)	20	100					20	100			100	
Trailing Detector (ft)	0	0					0	0			0	
Detector 1 Position(ft)	0	0					0	0			0	
Detector 1 Size(ft)	20	6					20	6			6	
Detector 1 Type	Cl+Ex	Cl+Ex					Cl+Ex	Cl+Ex			Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0			0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0			0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0			0.0	
Detector 2 Position(ft)		94						94			94	
Detector 2 Size(ft)		6						6			6	
Detector 2 Type		Cl+Ex						Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA			NA	
Protected Phases		4						6			2	
Permitted Phases	4						6					

Lanes, Volumes, Timings
 1093: Grand River Ave & Jeffries Service Dr

08/08/2022

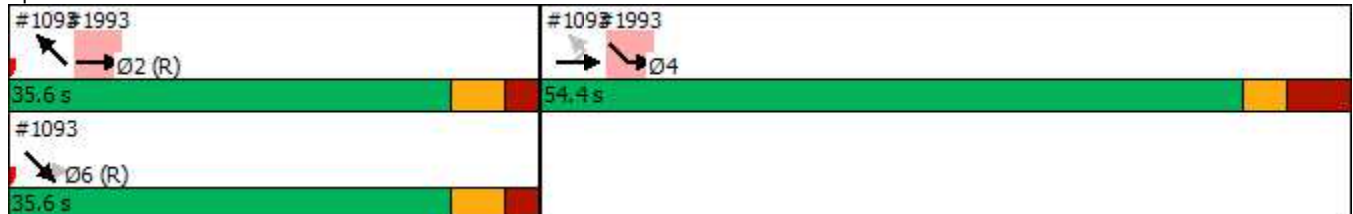


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Detector Phase	4	4					6	6			2	
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0			10.0	
Minimum Split (s)	57.4	57.4					37.0	37.0			37.0	
Total Split (s)	54.4	54.4					35.6	35.6			35.6	
Total Split (%)	60.4%	60.4%					39.6%	39.6%			39.6%	
Maximum Green (s)	47.0	47.0					29.6	29.6			29.6	
Yellow Time (s)	3.0	3.0					3.6	3.6			3.6	
All-Red Time (s)	4.4	4.4					2.4	2.4			2.4	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		7.4						6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0			3.0	
Recall Mode	None	None					C-Max	C-Max			C-Max	
Walk Time (s)	9.0	9.0					7.0	7.0			7.0	
Flash Dont Walk (s)	33.0	33.0					24.0	24.0			24.0	
Pedestrian Calls (#/hr)	0	0					0	0			0	
Act Effct Green (s)		13.6						63.0			63.0	
Actuated g/C Ratio		0.15						0.70			0.70	
v/c Ratio		0.54						0.20			0.18	
Control Delay		35.6						1.3			4.9	
Queue Delay		0.0						0.0			0.0	
Total Delay		35.6						1.3			4.9	
LOS		D						A			A	
Approach Delay		35.6						1.3			4.9	
Approach LOS		D						A			A	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 5 (6%), Referenced to phase 2:NWT and 6:SETL, Start of Green
 Natural Cycle: 95
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.54
 Intersection Signal Delay: 12.6
 Intersection Capacity Utilization 45.3%
 Analysis Period (min) 15
 Intersection LOS: B
 ICU Level of Service A

Splits and Phases: 1093: Grand River Ave & Jeffries Service Dr



Lanes, Volumes, Timings

1193: Left Turn Bridge & Grand River Ave & I-96 WB Service Rd/I-96 WB Service Dr 08/08/2022



Lane Group	WBL2	WBL	WBT	WBR	SET	SER	SER2	NWL	NWT
Lane Configurations			↑↑↑	↑	↑↑↑	↑		↑	↑↑↑
Traffic Volume (vph)	51	6	235	391	523	131	8	75	451
Future Volume (vph)	51	6	235	391	523	131	8	75	451
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0		0		140		260	
Storage Lanes		0		1		1		1	
Taper Length (ft)		65						65	
Lane Util. Factor	0.91	0.91	0.86	0.86	0.86	0.86	0.91	1.00	0.91
Frt			0.940	0.850	0.994	0.850			
Flt Protected			0.994					0.950	
Satd. Flow (prot)	0	0	4394	1335	4725	1317	0	1805	5036
Flt Permitted			0.994					0.950	
Satd. Flow (perm)	0	0	4394	1335	4725	1317	0	1805	5036
Right Turn on Red				Yes			Yes		
Satd. Flow (RTOR)			205	212		126			
Link Speed (mph)			35		35				35
Link Distance (ft)			675		427				582
Travel Time (s)			13.1		8.3				11.3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.93	0.74	0.50	0.78	0.89
Heavy Vehicles (%)	2%	2%	5%	4%	3%	6%	0%	0%	3%
Adj. Flow (vph)	55	7	255	425	562	177	16	96	507
Shared Lane Traffic (%)				50%		14%			
Lane Group Flow (vph)	0	0	530	212	587	168	0	96	507
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right	Right	Left	Left
Median Width(ft)			0		12				12
Link Offset(ft)			0		0				0
Crosswalk Width(ft)			16		16				16
Two way Left Turn Lane									
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	15		9		9	9	15	
Number of Detectors	1	1	2	1	2	1		1	2
Detector Template	Left	Left	Thru	Right	Thru	Right		Left	Thru
Leading Detector (ft)	20	20	100	20	100	20		20	100
Trailing Detector (ft)	0	0	0	0	0	0		0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0
Detector 1 Size(ft)	20	20	6	20	6	20		20	6
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex
Detector 1 Channel									
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
Detector 2 Position(ft)			94		94				94
Detector 2 Size(ft)			6		6				6
Detector 2 Type			Cl+Ex		Cl+Ex				Cl+Ex
Detector 2 Channel									
Detector 2 Extend (s)			0.0		0.0				0.0
Turn Type	Perm	Perm	NA	Perm	NA	Perm		Prot	NA
Protected Phases			4		6			5	2

Lanes, Volumes, Timings

1193: Left Turn Bridge & Grand River Ave & I-96 WB Service Rd/I-96 WB Service Dr 08/08/2022



Lane Group	WBL2	WBL	WBT	WBR	SET	SER	SER2	NWL	NWT
Permitted Phases	4	4		4		6			
Detector Phase	4	4	4	4	6	6		5	2
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0		7.0	10.0
Minimum Split (s)	55.5	55.5	55.5	55.5	37.9	37.9		12.9	37.9
Total Split (s)	38.0	38.0	38.0	38.0	35.6	35.6		16.4	52.0
Total Split (%)	42.2%	42.2%	42.2%	42.2%	39.6%	39.6%		18.2%	57.8%
Maximum Green (s)	30.5	30.5	30.5	30.5	29.7	29.7		10.5	46.1
Yellow Time (s)	3.0	3.0	3.0	3.0	3.6	3.6		3.6	3.6
All-Red Time (s)	4.5	4.5	4.5	4.5	2.3	2.3		2.3	2.3
Lost Time Adjust (s)			0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)			7.5	7.5	5.9	5.9		5.9	5.9
Lead/Lag					Lead	Lead		Lag	
Lead-Lag Optimize?					Yes	Yes		Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max		None	C-Max
Walk Time (s)	9.0	9.0	9.0	9.0	7.0	7.0			7.0
Flash Dont Walk (s)	39.0	39.0	39.0	39.0	25.0	25.0			25.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0			0
Act Effct Green (s)			14.5	14.5	49.0	49.0		9.8	62.1
Actuated g/C Ratio			0.16	0.16	0.54	0.54		0.11	0.69
v/c Ratio			0.60	0.54	0.23	0.22		0.49	0.15
Control Delay			23.7	10.1	9.6	3.0		42.0	1.2
Queue Delay			0.0	0.0	0.0	0.0		0.0	0.0
Total Delay			23.7	10.1	9.6	3.0		42.0	1.2
LOS			C	B	A	A		D	A
Approach Delay			19.8		8.1				7.7
Approach LOS			B		A				A

Intersection Summary

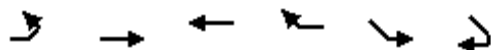
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 88 (98%), Referenced to phase 2:NWT and 6:SET, Start of Green
 Natural Cycle: 110
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.60
 Intersection Signal Delay: 12.1
 Intersection Capacity Utilization 41.7%
 Analysis Period (min) 15
 Intersection LOS: B
 ICU Level of Service A

Splits and Phases: 1193: Left Turn Bridge & Grand River Ave & I-96 WB Service Rd/I-96 WB Service Dr



Lanes, Volumes, Timings
 1993: Jeffries Service Dr & Left Turn Bridge

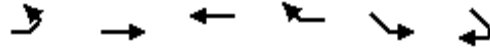
08/08/2022



Lane Group	EBL	EBT	WBT	WBR	SEL	SER	Ø6
Lane Configurations		↑↑↑			↑↑		
Traffic Volume (vph)	0	347	0	0	137	0	
Future Volume (vph)	0	347	0	0	137	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (ft)	0			0	50	0	
Storage Lanes	0			0	0	0	
Taper Length (ft)	65				65		
Lane Util. Factor	1.00	0.91	1.00	1.00	0.97	1.00	
Frt							
Flt Protected					0.950		
Satd. Flow (prot)	0	5085	0	0	3433	0	
Flt Permitted					0.950		
Satd. Flow (perm)	0	5085	0	0	3433	0	
Right Turn on Red				Yes	Yes	Yes	
Satd. Flow (RTOR)					474		
Link Speed (mph)		35	35		30		
Link Distance (ft)		184	329		419		
Travel Time (s)		3.6	6.4		9.5		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	377	0	0	149	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	377	0	0	149	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		0	0		24		
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		16	16		16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15			9	15	9	
Number of Detectors		2			1		
Detector Template		Thru			Left		
Leading Detector (ft)		100			20		
Trailing Detector (ft)		0			0		
Detector 1 Position(ft)		0			0		
Detector 1 Size(ft)		6			20		
Detector 1 Type		Cl+Ex			Cl+Ex		
Detector 1 Channel							
Detector 1 Extend (s)		0.0			0.0		
Detector 1 Queue (s)		0.0			0.0		
Detector 1 Delay (s)		0.0			0.0		
Detector 2 Position(ft)		94					
Detector 2 Size(ft)		6					
Detector 2 Type		Cl+Ex					
Detector 2 Channel							
Detector 2 Extend (s)		0.0					
Turn Type		NA			Prot		
Protected Phases		2			4		6
Permitted Phases							

Lanes, Volumes, Timings
 1993: Jeffries Service Dr & Left Turn Bridge

08/08/2022

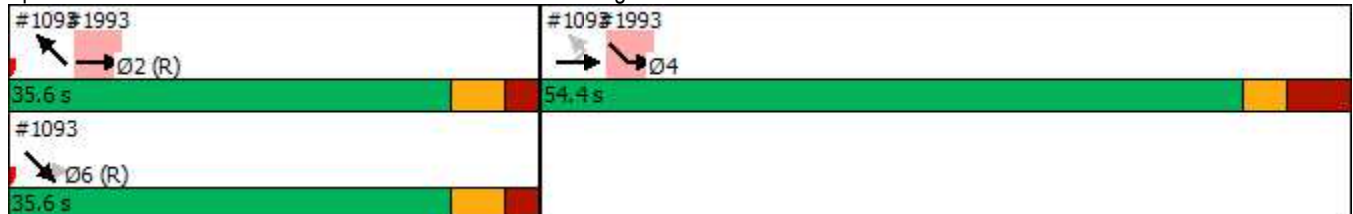


Lane Group	EBL	EBT	WBT	WBR	SEL	SER	Ø6
Detector Phase		2			4		
Switch Phase							
Minimum Initial (s)		10.0			10.0		10.0
Minimum Split (s)		37.0			57.4		37.0
Total Split (s)		35.6			54.4		35.6
Total Split (%)		39.6%			60.4%		40%
Maximum Green (s)		29.6			47.0		29.6
Yellow Time (s)		3.6			3.0		3.6
All-Red Time (s)		2.4			4.4		2.4
Lost Time Adjust (s)		0.0			0.0		
Total Lost Time (s)		6.0			7.4		
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)		3.0			3.0		3.0
Recall Mode		C-Max			None		C-Max
Walk Time (s)		7.0			9.0		7.0
Flash Dont Walk (s)		24.0			33.0		24.0
Pedestrian Calls (#/hr)		0			0		0
Act Effct Green (s)		63.0			13.6		
Actuated g/C Ratio		0.70			0.15		
v/c Ratio		0.11			0.16		
Control Delay		4.5			3.8		
Queue Delay		0.0			0.0		
Total Delay		4.5			3.8		
LOS		A			A		
Approach Delay		4.5			3.8		
Approach LOS		A			A		

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	5 (6%), Referenced to phase 2:NWT and 6:SETL, Start of Green
Natural Cycle:	95
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.54
Intersection Signal Delay:	4.3
Intersection LOS:	A
Intersection Capacity Utilization:	27.8%
ICU Level of Service:	A
Analysis Period (min):	15

Splits and Phases: 1993: Jeffries Service Dr & Left Turn Bridge



Lanes, Volumes, Timings
2609: Schaefer Hwy & Schoolcraft Rd

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	50	113	17	60	98	55	20	586	70	43	543	42
Future Volume (vph)	50	113	17	60	98	55	20	586	70	43	543	42
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	60		0	60		0	80		250	80		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor								1.00		1.00		
Fr _t		0.971			0.950			0.982				0.986
Fl _t Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1378	1830	0	1656	1713	0	1805	1771	0	1530	1696	0
Fl _t Permitted	0.606			0.656			0.139			0.224		
Satd. Flow (perm)	879	1830	0	1143	1713	0	264	1771	0	360	1696	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		15			32			11				8
Link Speed (mph)		30			30			30				30
Link Distance (ft)		699			1470			1209				229
Travel Time (s)		15.9			33.4			27.5				5.2
Confl. Peds. (#/hr)									4	4		
Peak Hour Factor	0.63	0.88	0.54	0.84	0.71	0.79	0.57	0.94	0.81	0.66	0.91	0.68
Heavy Vehicles (%)	31%	1%	0%	9%	1%	14%	0%	5%	5%	18%	8%	34%
Adj. Flow (vph)	79	128	31	71	138	70	35	623	86	65	597	62
Shared Lane Traffic (%)												
Lane Group Flow (vph)	79	159	0	71	208	0	35	709	0	65	659	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	

Lanes, Volumes, Timings
2609: Schaefer Hwy & Schoolcraft Rd

08/08/2022

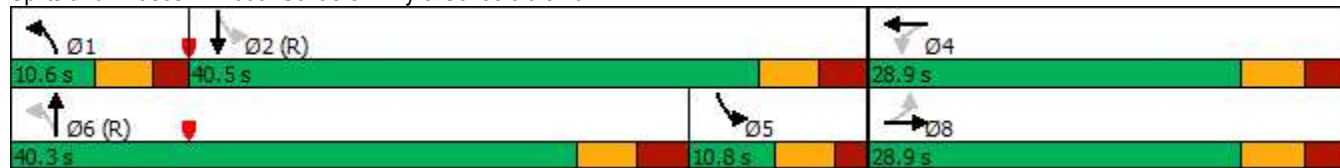


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		
Detector Phase	8	8		4	4		1	6		5	2	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0		5.0	9.0		5.0	9.0	
Minimum Split (s)	28.8	28.8		28.8	28.8		10.6	26.6		10.6	26.6	
Total Split (s)	28.9	28.9		28.9	28.9		10.6	40.3		10.8	40.5	
Total Split (%)	36.1%	36.1%		36.1%	36.1%		13.3%	50.4%		13.5%	50.6%	
Maximum Green (s)	22.1	22.1		22.1	22.1		5.0	33.7		5.2	33.9	
Yellow Time (s)	3.8	3.8		3.8	3.8		3.5	3.6		3.5	3.6	
All-Red Time (s)	3.0	3.0		3.0	3.0		2.1	3.0		2.1	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.8	6.8		6.8	6.8		5.6	6.6		5.6	6.6	
Lead/Lag							Lead	Lead		Lag	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	Max	Max		Max	Max		Max	C-Max		Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0			13.0			13.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	22.1	22.1		22.1	22.1		34.7	33.7		34.9	33.9	
Actuated g/C Ratio	0.28	0.28		0.28	0.28		0.43	0.42		0.44	0.42	
v/c Ratio	0.33	0.31		0.23	0.42		0.17	0.94		0.28	0.91	
Control Delay	27.6	22.6		24.7	23.0		15.0	45.4		13.5	35.3	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	27.6	22.6		24.7	23.0		15.0	45.4		13.5	35.3	
LOS	C	C		C	C		B	D		B	D	
Approach Delay		24.3			23.4			44.0			33.4	
Approach LOS		C			C			D			C	

Intersection Summary

Area Type: Other
 Cycle Length: 80
 Actuated Cycle Length: 80
 Offset: 33 (41%), Referenced to phase 2:SBTL and 6:NBTL, Start of Green
 Natural Cycle: 80
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.94
 Intersection Signal Delay: 34.9
 Intersection LOS: C
 Intersection Capacity Utilization 66.1%
 ICU Level of Service C
 Analysis Period (min) 15

Splits and Phases: 2609: Schaefer Hwy & Schoolcraft Rd



Lanes, Volumes, Timings
3011: Schaefer Hwy & I-96 WB Service Rd

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				↙	↕		↙	↕			↕	
Traffic Volume (vph)	0	0	0	156	158	4	234	524	0	0	368	268
Future Volume (vph)	0	0	0	156	158	4	234	524	0	0	368	268
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Lane Width (ft)	12	12	12	12	11	12	11	11	12	12	11	12
Storage Length (ft)	0		0	0		0	250		0	0		0
Storage Lanes	0		0	1		0	1		0	0		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95
Ped Bike Factor							1.00					0.99
Frt					0.997							0.936
Flt Protected				0.950			0.950					
Satd. Flow (prot)	0	0	0	1863	3566	0	1766	3566	0	0	3227	0
Flt Permitted				0.950			0.354					
Satd. Flow (perm)	0	0	0	1863	3566	0	657	3566	0	0	3227	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					2							274
Link Speed (mph)		25			25			25				35
Link Distance (ft)		298			364			384				332
Travel Time (s)		8.1			9.9			10.5				6.5
Confl. Peds. (#/hr)							4					4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.83	0.91	0.92	0.92	0.95	0.93
Heavy Vehicles (%)	2%	2%	2%	2%	2%	33%	4%	3%	2%	2%	7%	4%
Adj. Flow (vph)	0	0	0	170	172	4	282	576	0	0	387	288
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	170	176	0	282	576	0	0	675	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												
Headway Factor	0.94	0.94	0.94	0.94	0.98	0.94	0.98	0.98	0.94	0.94	0.98	0.94
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors				1	2		1	2				2
Detector Template				Left	Thru		Left	Thru				Thru
Leading Detector (ft)				20	100		20	100				100
Trailing Detector (ft)				0	0		0	0				0
Detector 1 Position(ft)				0	0		0	0				0
Detector 1 Size(ft)				20	6		20	6				6
Detector 1 Type				Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex				Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)				0.0	0.0		0.0	0.0				0.0
Detector 1 Queue (s)				0.0	0.0		0.0	0.0				0.0
Detector 1 Delay (s)				0.0	0.0		0.0	0.0				0.0
Detector 2 Position(ft)					94			94				94
Detector 2 Size(ft)					6			6				6
Detector 2 Type					Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	

Lanes, Volumes, Timings
 3011: Schaefer Hwy & I-96 WB Service Rd

08/08/2022

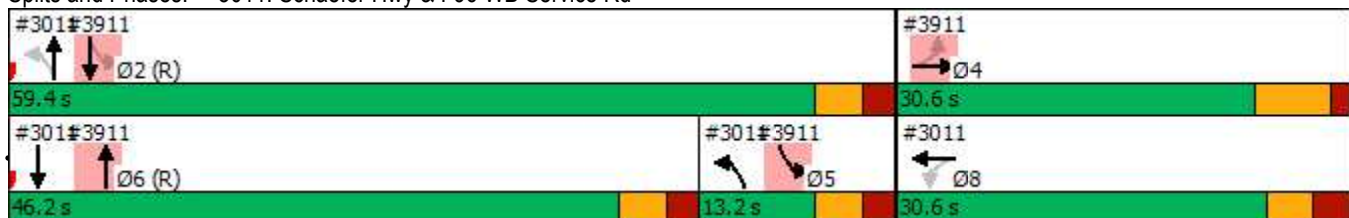


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)					0.0			0.0			0.0	
Turn Type				Perm	NA		pm+pt	NA			NA	
Protected Phases					8		5	2			6	
Permitted Phases				8			2					
Detector Phase				8	8		5	2				6
Switch Phase												
Minimum Initial (s)				10.0	10.0		7.0	10.0				10.0
Minimum Split (s)				28.7	28.7		12.4	23.4				23.4
Total Split (s)				30.6	30.6		13.2	59.4				46.2
Total Split (%)				34.0%	34.0%		14.7%	66.0%				51.3%
Maximum Green (s)				24.9	24.9		7.8	54.0				40.8
Yellow Time (s)				3.0	3.0		3.2	3.2				3.2
All-Red Time (s)				2.7	2.7		2.2	2.2				2.2
Lost Time Adjust (s)				0.0	0.0		0.0	0.0				0.0
Total Lost Time (s)				5.7	5.7		5.4	5.4				5.4
Lead/Lag							Lag				Lead	
Lead-Lag Optimize?							Yes				Yes	
Vehicle Extension (s)				3.0	3.0		3.0	3.0				3.0
Recall Mode				None	None		None	C-Max				C-Max
Walk Time (s)				7.0	7.0			7.0				7.0
Flash Dont Walk (s)				16.0	16.0			11.0				11.0
Pedestrian Calls (#/hr)				0	0			0				0
Act Effct Green (s)				19.3	19.3		59.6	59.6				46.4
Actuated g/C Ratio				0.21	0.21		0.66	0.66				0.52
v/c Ratio				0.43	0.23		0.53	0.24				0.38
Control Delay				17.0	13.0		8.3	3.5				4.5
Queue Delay				0.0	0.0		0.0	0.0				0.1
Total Delay				17.0	13.0		8.3	3.5				4.6
LOS				B	B		A	A				A
Approach Delay					15.0			5.1				4.6
Approach LOS					B			A				A

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 89 (99%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.83
 Intersection Signal Delay: 6.7
 Intersection LOS: A
 Intersection Capacity Utilization 68.5%
 ICU Level of Service C
 Analysis Period (min) 15

Splits and Phases: 3011: Schaefer Hwy & I-96 WB Service Rd



Lane Group	Ø4
Detector 2 Extend (s)	
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
Minimum Split (s)	30.5
Total Split (s)	30.6
Total Split (%)	34%
Maximum Green (s)	24.1
Yellow Time (s)	5.0
All-Red Time (s)	1.5
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	17.0
Pedestrian Calls (#/hr)	0
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Lanes, Volumes, Timings
3911: Schaefer Hwy & Jeffries Service Dr

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕↕↕						↔		↕	↕↕	
Traffic Volume (vph)	265	135	298	0	0	0	0	494	122	86	439	0
Future Volume (vph)	265	135	298	0	0	0	0	494	122	86	439	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	0		0	90		0
Storage Lanes	0		0	0		0	0		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	0.91	0.91	0.91	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00
Ped Bike Factor								1.00				
Frt		0.936						0.972				
Flt Protected		0.981								0.950		
Satd. Flow (prot)	0	4550	0	0	0	0	0	1721	0	1703	3406	0
Flt Permitted		0.981								0.198		
Satd. Flow (perm)	0	4550	0	0	0	0	0	1721	0	355	3406	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		203						19				
Link Speed (mph)		35			35			30				30
Link Distance (ft)		417			372			621				384
Travel Time (s)		8.1			7.2			14.1				8.7
Confl. Peds. (#/hr)									6	6		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.84	0.80	0.80	0.92	0.92
Heavy Vehicles (%)	2%	7%	6%	2%	2%	2%	2%	6%	10%	6%	6%	2%
Bus Blockages (#/hr)	3	0	0	0	0	0	0	0	0	0	0	0
Adj. Flow (vph)	288	147	324	0	0	0	0	588	153	108	477	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	759	0	0	0	0	0	741	0	108	477	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2						2		1	2	
Detector Template	Left	Thru						Thru		Left	Thru	
Leading Detector (ft)	20	100						100		20	100	
Trailing Detector (ft)	0	0						0		0	0	
Detector 1 Position(ft)	0	0						0		0	0	
Detector 1 Size(ft)	20	6						6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex						Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0						0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0						0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0						0.0		0.0	0.0	
Detector 2 Position(ft)		94						94			94	
Detector 2 Size(ft)		6						6			6	
Detector 2 Type		Cl+Ex						Cl+Ex			Cl+Ex	
Detector 2 Channel												

Lane Group	Ø8
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Bus Blockages (#/hr)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	

Lanes, Volumes, Timings
 3911: Schaefer Hwy & Jeffries Service Dr

08/08/2022

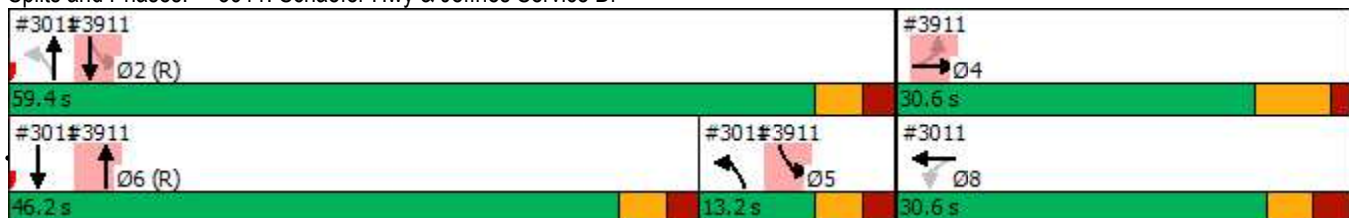


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA						NA	pm+pt		NA	
Protected Phases		4						6		5	2	
Permitted Phases	4									2		
Detector Phase	4	4						6		5	2	
Switch Phase												
Minimum Initial (s)	10.0	10.0						10.0		7.0	10.0	
Minimum Split (s)	30.5	30.5						23.4		12.4	23.4	
Total Split (s)	30.6	30.6						46.2		13.2	59.4	
Total Split (%)	34.0%	34.0%						51.3%		14.7%	66.0%	
Maximum Green (s)	24.1	24.1						40.8		7.8	54.0	
Yellow Time (s)	5.0	5.0						3.2		3.2	3.2	
All-Red Time (s)	1.5	1.5						2.2		2.2	2.2	
Lost Time Adjust (s)		0.0						0.0		0.0	0.0	
Total Lost Time (s)		6.5						5.4		5.4	5.4	
Lead/Lag								Lead		Lag		
Lead-Lag Optimize?								Yes		Yes		
Vehicle Extension (s)	3.0	3.0						3.0		3.0	3.0	
Recall Mode	None	None						C-Max		None	C-Max	
Walk Time (s)	7.0	7.0						7.0			7.0	
Flash Dont Walk (s)	17.0	17.0						11.0			11.0	
Pedestrian Calls (#/hr)	0	0						0			0	
Act Effct Green (s)		18.5						46.4		59.6	59.6	
Actuated g/C Ratio		0.21						0.52		0.66	0.66	
v/c Ratio		0.69						0.83		0.31	0.21	
Control Delay		27.0						29.1		12.0	6.6	
Queue Delay		0.0						0.0		0.0	0.0	
Total Delay		27.0						29.1		12.0	6.6	
LOS		C						C		B	A	
Approach Delay		27.0						29.1			7.6	
Approach LOS		C						C			A	

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 89 (99%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.83
 Intersection Signal Delay: 22.3
 Intersection LOS: C
 Intersection Capacity Utilization 68.5%
 ICU Level of Service C
 Analysis Period (min) 15

Splits and Phases: 3911: Schaefer Hwy & Jeffries Service Dr



Lane Group	Ø8
Detector 2 Extend (s)	
Turn Type	
Protected Phases	8
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
Minimum Split (s)	28.7
Total Split (s)	30.6
Total Split (%)	34%
Maximum Green (s)	24.9
Yellow Time (s)	3.0
All-Red Time (s)	2.7
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	16.0
Pedestrian Calls (#/hr)	0
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Lanes, Volumes, Timings
4001: Schaefer Hwy & Lyndon St

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	28	146	36	71	99	33	45	570	66	57	454	43
Future Volume (vph)	28	146	36	71	99	33	45	570	66	57	454	43
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	80		0	80		0	80		0	80		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor		1.00		1.00			1.00	1.00		1.00	1.00	
Frt		0.970			0.963			0.982				0.983
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1781	0	1656	1812	0	1752	3328	0	1770	3212	0
Flt Permitted	0.563			0.441			0.413			0.332		
Satd. Flow (perm)	1070	1781	0	767	1812	0	758	3328	0	617	3212	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		16			21			23			21	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		648			545			853			685	
Travel Time (s)		14.7			12.4			19.4			15.6	
Confl. Peds. (#/hr)			2	2			6		5	5		6
Peak Hour Factor	0.69	0.76	0.75	0.75	0.70	0.72	0.67	0.88	0.73	0.75	0.88	0.65
Heavy Vehicles (%)	0%	3%	4%	9%	0%	4%	3%	7%	0%	2%	11%	3%
Adj. Flow (vph)	41	192	48	95	141	46	67	648	90	76	516	66
Shared Lane Traffic (%)												
Lane Group Flow (vph)	41	240	0	95	187	0	67	738	0	76	582	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	

Lanes, Volumes, Timings
4001: Schaefer Hwy & Lyndon St

08/08/2022

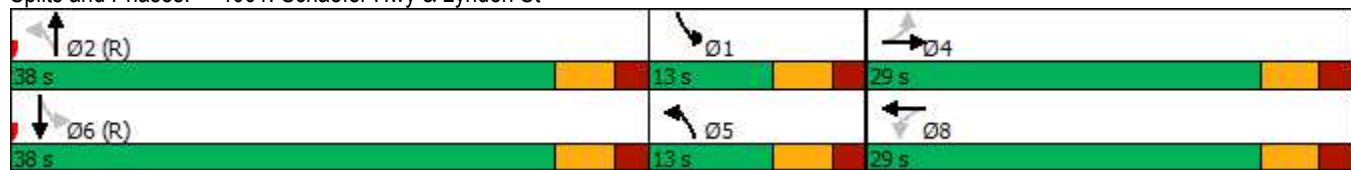


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		4.0	15.0		4.0	15.0	
Minimum Split (s)	24.6	24.6		24.6	24.6		9.6	26.6		9.6	26.6	
Total Split (s)	29.0	29.0		29.0	29.0		13.0	38.0		13.0	38.0	
Total Split (%)	36.3%	36.3%		36.3%	36.3%		16.3%	47.5%		16.3%	47.5%	
Maximum Green (s)	23.4	23.4		23.4	23.4		7.4	32.4		7.4	32.4	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	2.1	2.1		2.1	2.1		2.1	2.1		2.1	2.1	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.6	5.6		5.6	5.6		5.6	5.6		5.6	5.6	
Lead/Lag							Lag	Lead		Lag	Lead	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	Min	Min		Min	Min		None	C-Max		None	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	12.0	12.0		12.0	12.0			14.0			14.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	15.6	15.6		15.6	15.6		49.8	44.1		49.8	44.1	
Actuated g/C Ratio	0.20	0.20		0.20	0.20		0.62	0.55		0.62	0.55	
v/c Ratio	0.20	0.67		0.64	0.51		0.12	0.40		0.16	0.33	
Control Delay	27.2	36.4		47.9	29.3		4.1	4.4		7.8	11.6	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	27.2	36.4		47.9	29.3		4.1	4.4		7.8	11.6	
LOS	C	D		D	C		A	A		A	B	
Approach Delay		35.0			35.6			4.4			11.2	
Approach LOS		D			D			A			B	

Intersection Summary

Area Type:	Other
Cycle Length:	80
Actuated Cycle Length:	80
Offset:	0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle:	65
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.67
Intersection Signal Delay:	15.2
Intersection LOS:	B
Intersection Capacity Utilization	58.5%
ICU Level of Service	B
Analysis Period (min)	15

Splits and Phases: 4001: Schaefer Hwy & Lyndon St



Intersection												
Int Delay, s/veh	1.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	6	218	3	1	204	5	1	0	8	6	0	8
Future Vol, veh/h	6	218	3	1	204	5	1	0	8	6	0	8
Conflicting Peds, #/hr	3	0	1	1	0	3	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	63	94	50	25	86	50	25	25	50	63	25	50
Heavy Vehicles, %	0	7	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	10	232	6	4	237	10	4	0	16	10	0	16

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	250	0	0	239	0	0	514	514	236	516	512	245
Stage 1	-	-	-	-	-	-	256	256	-	253	253	-
Stage 2	-	-	-	-	-	-	258	258	-	263	259	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1327	-	-	1340	-	-	474	467	808	473	468	799
Stage 1	-	-	-	-	-	-	753	699	-	756	701	-
Stage 2	-	-	-	-	-	-	751	698	-	747	697	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1323	-	-	1339	-	-	460	460	807	458	461	797
Mov Cap-2 Maneuver	-	-	-	-	-	-	460	460	-	458	461	-
Stage 1	-	-	-	-	-	-	745	692	-	747	697	-
Stage 2	-	-	-	-	-	-	734	694	-	726	690	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.3	0.1	10.3	11
HCM LOS			B	B

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	701	1323	-	-	1339	-	-	625
HCM Lane V/C Ratio	0.029	0.007	-	-	0.003	-	-	0.041
HCM Control Delay (s)	10.3	7.7	0	-	7.7	0	-	11
HCM Lane LOS	B	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.1

Intersection												
Int Delay, s/veh	1.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	4	1	10	9	8	9	3	368	13	9	344	1
Future Vol, veh/h	4	1	10	9	8	9	3	368	13	9	344	1
Conflicting Peds, #/hr	0	0	0	0	0	0	1	0	1	1	0	1
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	50	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	60	60	60	60	60	60	100	85	63	60	87	60
Heavy Vehicles, %	0	0	0	0	0	0	0	5	0	0	3	0
Mvmt Flow	7	2	17	15	13	15	3	433	21	15	395	2

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	891	888	397	887	879	445	398	0	0	455	0	0
Stage 1	427	427	-	451	451	-	-	-	-	-	-	-
Stage 2	464	461	-	436	428	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	265	285	657	267	288	617	1172	-	-	1116	-	-
Stage 1	610	589	-	592	574	-	-	-	-	-	-	-
Stage 2	582	569	-	603	588	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	246	280	656	256	283	616	1171	-	-	1115	-	-
Mov Cap-2 Maneuver	246	280	-	256	283	-	-	-	-	-	-	-
Stage 1	608	581	-	590	572	-	-	-	-	-	-	-
Stage 2	553	567	-	578	580	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	13.9		17.4		0.1		0.3	
HCM LOS	B		C					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1171	-	-	428	333	1115	-
HCM Lane V/C Ratio	0.003	-	-	0.058	0.13	0.013	-
HCM Control Delay (s)	8.1	-	-	13.9	17.4	8.3	-
HCM Lane LOS	A	-	-	B	C	A	-
HCM 95th %tile Q(veh)	0	-	-	0.2	0.4	0	-

Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	4	3	688	4	3	625
Future Vol, veh/h	4	3	688	4	3	625
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	100	100	2	100	100	2
Mvmt Flow	4	3	748	4	3	679

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	1096	376	0	0	752
Stage 1	750	-	-	-	-
Stage 2	346	-	-	-	-
Critical Hdwy	8.8	8.9	-	-	6.1
Critical Hdwy Stg 1	7.8	-	-	-	-
Critical Hdwy Stg 2	7.8	-	-	-	-
Follow-up Hdwy	4.5	4.3	-	-	3.2
Pot Cap-1 Maneuver	101	410	-	-	431
Stage 1	243	-	-	-	-
Stage 2	466	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	100	410	-	-	431
Mov Cap-2 Maneuver	100	-	-	-	-
Stage 1	243	-	-	-	-
Stage 2	461	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	30.3	0	0.2
HCM LOS	D		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	100	410	431
HCM Lane V/C Ratio	-	-	0.043	0.008	0.008
HCM Control Delay (s)	-	-	42.6	13.9	13.4
HCM Lane LOS	-	-	E	B	B
HCM 95th %tile Q(veh)	-	-	0.1	0	0

Intersection						
Int Delay, s/veh	2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	76	23	658	33	9	552
Future Vol, veh/h	76	23	658	33	9	552
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	150	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	7	0	0	8
Mvmt Flow	83	25	715	36	10	600

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	1053	376	0	0	751
Stage 1	733	-	-	-	-
Stage 2	320	-	-	-	-
Critical Hdwy	6.8	6.9	-	-	4.1
Critical Hdwy Stg 1	5.8	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2
Pot Cap-1 Maneuver	225	627	-	-	868
Stage 1	442	-	-	-	-
Stage 2	715	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	221	627	-	-	868
Mov Cap-2 Maneuver	221	-	-	-	-
Stage 1	442	-	-	-	-
Stage 2	703	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	26.1	0	0.2
HCM LOS	D		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	221	627	868
HCM Lane V/C Ratio	-	-	0.374	0.04	0.011
HCM Control Delay (s)	-	-	30.7	11	9.2
HCM Lane LOS	-	-	D	B	A
HCM 95th %tile Q(veh)	-	-	1.6	0.1	0

Lanes, Volumes, Timings
 1011: Schaefer Hwy & Grand River Ave

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↕↕↕		↖	↕↕↕		↖	↕	↗	↖	↕↕	
Traffic Volume (vph)	48	528	80	24	545	130	67	311	23	78	350	29
Future Volume (vph)	48	528	80	24	545	130	67	311	23	78	350	29
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	115		0	95		0	155		0	80		240
Storage Lanes	1		0	0		0	1		1	1		1
Taper Length (ft)	65			65			25			65		
Lane Util. Factor	1.00	0.91	0.91	1.00	0.91	0.91	1.00	1.00	1.00	1.00	0.95	0.95
Ped Bike Factor		1.00		1.00					0.99	1.00		
Frt		0.977			0.968				0.850		0.988	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	5014	0	1805	4917	0	1805	1827	1615	1736	3502	0
Flt Permitted	0.345			0.363			0.446			0.286		
Satd. Flow (perm)	656	5014	0	689	4917	0	847	1827	1593	522	3502	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		47			85				46			14
Link Speed (mph)		35			35			30				30
Link Distance (ft)		932			422			332				1209
Travel Time (s)		18.2			8.2			7.5				27.5
Confl. Peds. (#/hr)			2	2					2	2		
Peak Hour Factor	0.80	0.88	0.72	0.59	0.92	0.82	0.74	0.74	0.50	0.76	0.86	0.80
Heavy Vehicles (%)	0%	1%	0%	0%	0%	10%	0%	4%	0%	4%	2%	0%
Adj. Flow (vph)	60	600	111	41	592	159	91	420	46	103	407	36
Shared Lane Traffic (%)												
Lane Group Flow (vph)	60	711	0	41	751	0	91	420	46	103	443	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2	1	1		2
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right	Left		Thru
Leading Detector (ft)	20	100		20	100		20	100	20	20		100
Trailing Detector (ft)	0	0		0	0		0	0	0	0		0
Detector 1 Position(ft)	0	0		0	0		0	0	0	0		0
Detector 1 Size(ft)	20	6		20	6		20	6	20	20		6
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0		0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0		0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0		0.0
Detector 2 Position(ft)		94			94			94				94
Detector 2 Size(ft)		6			6			6				6
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				0.0

Lanes, Volumes, Timings
 1011: Schaefer Hwy & Grand River Ave

08/08/2022

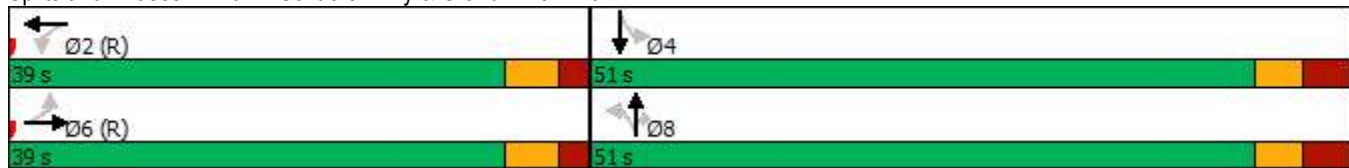


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA		Perm	NA		Perm	NA	Perm	Perm	NA	
Protected Phases		6			2			8				4
Permitted Phases	6			2			8		8	4		
Detector Phase	6	6		2	2		8	8	8	4		4
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		7.0	7.0	7.0	7.0		7.0
Minimum Split (s)	35.8	35.8		35.8	35.8		46.5	46.5	46.5	46.5		46.5
Total Split (s)	39.0	39.0		39.0	39.0		51.0	51.0	51.0	51.0		51.0
Total Split (%)	43.3%	43.3%		43.3%	43.3%		56.7%	56.7%	56.7%	56.7%		56.7%
Maximum Green (s)	33.2	33.2		33.2	33.2		44.5	44.5	44.5	44.5		44.5
Yellow Time (s)	3.6	3.6		3.6	3.6		3.2	3.2	3.2	3.2		3.2
All-Red Time (s)	2.2	2.2		2.2	2.2		3.3	3.3	3.3	3.3		3.3
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0		0.0
Total Lost Time (s)	5.8	5.8		5.8	5.8		6.5	6.5	6.5	6.5		6.5
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0		3.0
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None	None	None		None
Walk Time (s)	7.0	7.0		7.0	7.0		10.0	10.0	10.0	10.0		10.0
Flash Dont Walk (s)	23.0	23.0		23.0	23.0		30.0	30.0	30.0	30.0		30.0
Pedestrian Calls (#/hr)	0	0		0	0		0	0	0	0		0
Act Effct Green (s)	49.9	49.9		49.9	49.9		27.8	27.8	27.8	27.8		27.8
Actuated g/C Ratio	0.55	0.55		0.55	0.55		0.31	0.31	0.31	0.31		0.31
v/c Ratio	0.17	0.25		0.11	0.27		0.35	0.74	0.09	0.64		0.41
Control Delay	14.2	11.0		11.1	8.7		16.0	26.1	4.2	43.5		23.8
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.2	0.0	0.0		0.0
Total Delay	14.2	11.0		11.1	8.7		16.0	26.3	4.2	43.5		23.8
LOS	B	B		B	A		B	C	A	D		C
Approach Delay		11.3			8.8			22.8				27.5
Approach LOS		B			A			C				C

Intersection Summary

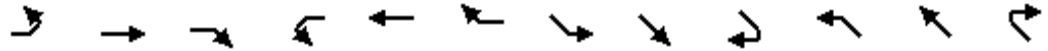
Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 61 (68%), Referenced to phase 2:WBTL and 6:EBTL, Start of Green
 Natural Cycle: 85
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.74
 Intersection Signal Delay: 16.3
 Intersection LOS: B
 Intersection Capacity Utilization 77.1%
 ICU Level of Service D
 Analysis Period (min) 15

Splits and Phases: 1011: Schaefer Hwy & Grand River Ave



Lanes, Volumes, Timings
 1093: Grand River Ave & Jeffries Service Dr

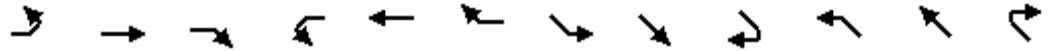
08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↑↑↑↑						↑↑↑↑			↑↑↑↑	
Traffic Volume (vph)	5	338	79	0	0	0	11	521	0	0	422	17
Future Volume (vph)	5	338	79	0	0	0	11	521	0	0	422	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.86	0.86	0.86	1.00	1.00	1.00	0.91	0.91	1.00	1.00	0.91	0.91
Ped Bike Factor								1.00			1.00	
Frt		0.972									0.994	
Flt Protected		0.999						0.998				
Satd. Flow (prot)	0	6296	0	0	0	0	0	5127	0	0	5039	0
Flt Permitted		0.999						0.908				
Satd. Flow (perm)	0	6296	0	0	0	0	0	4665	0	0	5039	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		86									8	
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		342			379			587			812	
Travel Time (s)		6.7			7.4			11.4			15.8	
Confl. Peds. (#/hr)							3					3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.56	0.96	0.92	0.92	0.71	0.65
Heavy Vehicles (%)	0%	1%	0%	2%	2%	2%	0%	1%	2%	2%	2%	8%
Adj. Flow (vph)	5	367	86	0	0	0	20	543	0	0	594	26
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	458	0	0	0	0	0	563	0	0	620	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2					1	2			2	
Detector Template	Left	Thru					Left	Thru			Thru	
Leading Detector (ft)	20	100					20	100			100	
Trailing Detector (ft)	0	0					0	0			0	
Detector 1 Position(ft)	0	0					0	0			0	
Detector 1 Size(ft)	20	6					20	6			6	
Detector 1 Type	Cl+Ex	Cl+Ex					Cl+Ex	Cl+Ex			Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0					0.0	0.0			0.0	
Detector 1 Queue (s)	0.0	0.0					0.0	0.0			0.0	
Detector 1 Delay (s)	0.0	0.0					0.0	0.0			0.0	
Detector 2 Position(ft)		94						94			94	
Detector 2 Size(ft)		6						6			6	
Detector 2 Type		Cl+Ex						Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	
Turn Type	Perm	NA					Perm	NA			NA	
Protected Phases		4						6			2	
Permitted Phases	4						6					

Lanes, Volumes, Timings
 1093: Grand River Ave & Jeffries Service Dr

08/08/2022

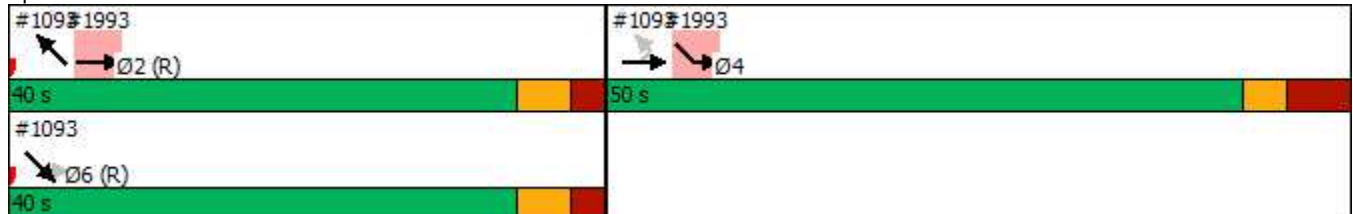


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Detector Phase	4	4					6	6				2
Switch Phase												
Minimum Initial (s)	10.0	10.0					10.0	10.0				10.0
Minimum Split (s)	49.4	49.4					37.0	37.0				37.0
Total Split (s)	50.0	50.0					40.0	40.0				40.0
Total Split (%)	55.6%	55.6%					44.4%	44.4%				44.4%
Maximum Green (s)	42.6	42.6					34.0	34.0				34.0
Yellow Time (s)	3.0	3.0					3.6	3.6				3.6
All-Red Time (s)	4.4	4.4					2.4	2.4				2.4
Lost Time Adjust (s)		0.0						0.0				0.0
Total Lost Time (s)		7.4						6.0				6.0
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0					3.0	3.0				3.0
Recall Mode	None	None					C-Max	C-Max				C-Max
Walk Time (s)	9.0	9.0					7.0	7.0				7.0
Flash Dont Walk (s)	33.0	33.0					24.0	24.0				24.0
Pedestrian Calls (#/hr)	0	0					0	0				0
Act Effct Green (s)		12.3						64.3				64.3
Actuated g/C Ratio		0.14						0.71				0.71
v/c Ratio		0.49						0.17				0.17
Control Delay		36.8						1.3				4.5
Queue Delay		0.0						0.0				0.0
Total Delay		36.8						1.3				4.5
LOS		D						A				A
Approach Delay		36.8						1.3				4.5
Approach LOS		D						A				A

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 75 (83%), Referenced to phase 2:NWT and 6:SETL, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.49
 Intersection Signal Delay: 12.4
 Intersection LOS: B
 Intersection Capacity Utilization 45.3%
 ICU Level of Service A
 Analysis Period (min) 15

Splits and Phases: 1093: Grand River Ave & Jeffries Service Dr



Lanes, Volumes, Timings

1193: Left Turn Bridge & Grand River Ave & I-96 Service Dr

08/08/2022



Lane Group	WBL2	WBL	WBT	WBR	SET	SER	SER2	NWL	NWT
Lane Configurations			↑↑↑	↑	↑↑↑	↑		↑	↑↑↑
Traffic Volume (vph)	29	8	237	330	503	122	4	58	369
Future Volume (vph)	29	8	237	330	503	122	4	58	369
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0		0		140		260	
Storage Lanes		0		1		1		1	
Taper Length (ft)		65						65	
Lane Util. Factor	0.91	0.91	0.86	0.86	0.86	0.86	0.91	1.00	0.91
Frt			0.944	0.850	0.996	0.850			
Flt Protected			0.996					0.950	
Satd. Flow (prot)	0	0	4544	1362	4834	1376	0	1805	5085
Flt Permitted			0.996					0.950	
Satd. Flow (perm)	0	0	4544	1362	4834	1376	0	1805	5085
Right Turn on Red				Yes			Yes		
Satd. Flow (RTOR)			179	179		126			
Link Speed (mph)			35		35				35
Link Distance (ft)			480		422				587
Travel Time (s)			9.4		8.2				11.4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.94	0.83	0.75	0.58	0.73
Heavy Vehicles (%)	2%	0%	1%	2%	1%	1%	0%	0%	2%
Adj. Flow (vph)	32	9	258	359	535	147	5	100	505
Shared Lane Traffic (%)				50%		10%			
Lane Group Flow (vph)	0	0	479	179	550	137	0	100	505
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right	Right	Left	Left
Median Width(ft)			0		12				12
Link Offset(ft)			0		0				0
Crosswalk Width(ft)			16		16				16
Two way Left Turn Lane									
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	15		9		9	9	15	
Number of Detectors	1	1	2	1	2	1		1	2
Detector Template	Left	Left	Thru	Right	Thru	Right		Left	Thru
Leading Detector (ft)	20	20	100	20	100	20		20	100
Trailing Detector (ft)	0	0	0	0	0	0		0	0
Detector 1 Position(ft)	0	0	0	0	0	0		0	0
Detector 1 Size(ft)	20	20	6	20	6	20		20	6
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex
Detector 1 Channel									
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0
Detector 2 Position(ft)			94		94				94
Detector 2 Size(ft)			6		6				6
Detector 2 Type			Cl+Ex		Cl+Ex				Cl+Ex
Detector 2 Channel									
Detector 2 Extend (s)			0.0		0.0				0.0
Turn Type	Perm	Perm	NA	Perm	NA	Perm		Prot	NA
Protected Phases			4		6			5	2

Lanes, Volumes, Timings

1193: Left Turn Bridge & Grand River Ave & I-96 Service Dr

08/08/2022

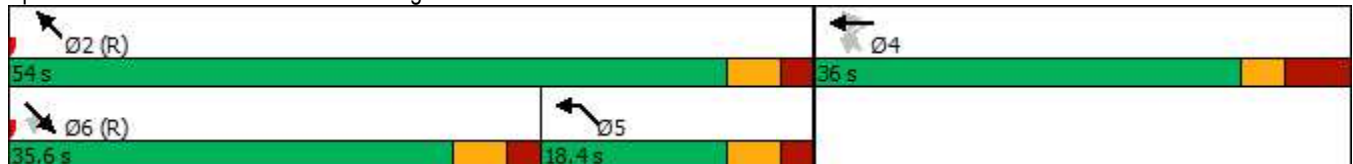


Lane Group	WBL2	WBL	WBT	WBR	SET	SER	SER2	NWL	NWT
Permitted Phases	4	4		4		6			
Detector Phase	4	4	4	4	6	6		5	2
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0		7.0	10.0
Minimum Split (s)	55.5	55.5	55.5	55.5	37.9	37.9		12.9	37.9
Total Split (s)	36.0	36.0	36.0	36.0	35.6	35.6		18.4	54.0
Total Split (%)	40.0%	40.0%	40.0%	40.0%	39.6%	39.6%		20.4%	60.0%
Maximum Green (s)	28.5	28.5	28.5	28.5	29.7	29.7		12.5	48.1
Yellow Time (s)	3.0	3.0	3.0	3.0	3.6	3.6		3.6	3.6
All-Red Time (s)	4.5	4.5	4.5	4.5	2.3	2.3		2.3	2.3
Lost Time Adjust (s)			0.0	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)			7.5	7.5	5.9	5.9		5.9	5.9
Lead/Lag					Lead	Lead		Lag	
Lead-Lag Optimize?					Yes	Yes		Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max		None	C-Max
Walk Time (s)	9.0	9.0	9.0	9.0	7.0	7.0			7.0
Flash Dont Walk (s)	39.0	39.0	39.0	39.0	25.0	25.0			25.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0			0
Act Effct Green (s)			13.2	13.2	48.7	48.7		11.4	63.4
Actuated g/C Ratio			0.15	0.15	0.54	0.54		0.13	0.70
v/c Ratio			0.58	0.51	0.21	0.17		0.44	0.14
Control Delay			24.7	10.6	7.5	1.1		37.6	0.5
Queue Delay			0.0	0.0	0.0	0.0		0.0	0.0
Total Delay			24.7	10.6	7.5	1.1		37.6	0.5
LOS			C	B	A	A		D	A
Approach Delay			20.9		6.2				6.6
Approach LOS			C		A				A

Intersection Summary

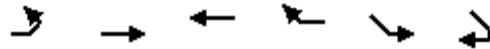
Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	72 (80%), Referenced to phase 2:NWT and 6:SET, Start of Green
Natural Cycle:	110
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.58
Intersection Signal Delay:	11.3
Intersection LOS:	B
Intersection Capacity Utilization:	40.9%
ICU Level of Service:	A
Analysis Period (min):	15

Splits and Phases: 1193: Left Turn Bridge & Grand River Ave & I-96 Service Dr



Lanes, Volumes, Timings
 1993: Jeffries Service Dr & Left Turn Bridge

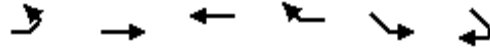
08/08/2022



Lane Group	EBL	EBT	WBT	WBR	SEL	SER	Ø6
Lane Configurations		↑↑↑			↑↑		
Traffic Volume (vph)	0	292	0	0	130	0	
Future Volume (vph)	0	292	0	0	130	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	0.91	1.00	1.00	0.97	1.00	
Frnt							
Flt Protected					0.950		
Satd. Flow (prot)	0	5085	0	0	3433	0	
Flt Permitted					0.950		
Satd. Flow (perm)	0	5085	0	0	3433	0	
Right Turn on Red				Yes	Yes	Yes	
Satd. Flow (RTOR)					715		
Link Speed (mph)		35	35		30		
Link Distance (ft)		191	342		422		
Travel Time (s)		3.7	6.7		9.6		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	317	0	0	141	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	317	0	0	141	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		0	0		24		
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		16	16		16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15			9	15	9	
Number of Detectors		2			1		
Detector Template		Thru			Left		
Leading Detector (ft)		100			20		
Trailing Detector (ft)		0			0		
Detector 1 Position(ft)		0			0		
Detector 1 Size(ft)		6			20		
Detector 1 Type		Cl+Ex			Cl+Ex		
Detector 1 Channel							
Detector 1 Extend (s)		0.0			0.0		
Detector 1 Queue (s)		0.0			0.0		
Detector 1 Delay (s)		0.0			0.0		
Detector 2 Position(ft)		94					
Detector 2 Size(ft)		6					
Detector 2 Type		Cl+Ex					
Detector 2 Channel							
Detector 2 Extend (s)		0.0					
Turn Type		NA			Prot		
Protected Phases		2			4	6	
Permitted Phases							
Detector Phase		2			4		
Switch Phase							
Minimum Initial (s)		10.0			10.0	10.0	

Lanes, Volumes, Timings
 1993: Jeffries Service Dr & Left Turn Bridge

08/08/2022

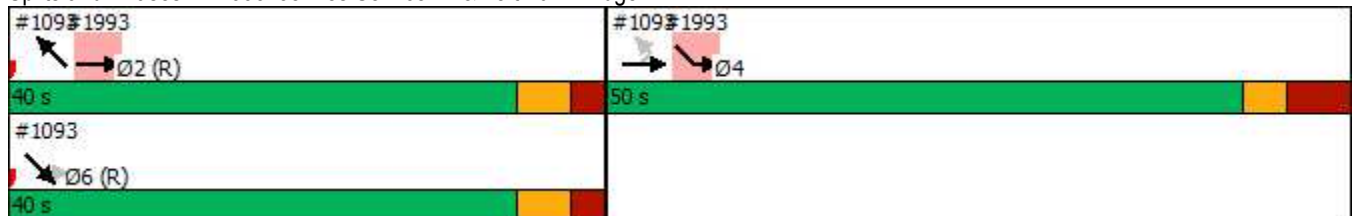


Lane Group	EBL	EBT	WBT	WBR	SEL	SER	Ø6
Minimum Split (s)		37.0			49.4		37.0
Total Split (s)		40.0			50.0		40.0
Total Split (%)		44.4%			55.6%		44%
Maximum Green (s)		34.0			42.6		34.0
Yellow Time (s)		3.6			3.0		3.6
All-Red Time (s)		2.4			4.4		2.4
Lost Time Adjust (s)		0.0			0.0		
Total Lost Time (s)		6.0			7.4		
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)		3.0			3.0		3.0
Recall Mode		C-Max			None		C-Max
Walk Time (s)		7.0			9.0		7.0
Flash Dont Walk (s)		24.0			33.0		24.0
Pedestrian Calls (#/hr)		0			0		0
Act Effct Green (s)		64.3			12.3		
Actuated g/C Ratio		0.71			0.14		
v/c Ratio		0.09			0.13		
Control Delay		2.6			1.0		
Queue Delay		0.0			0.0		
Total Delay		2.6			1.0		
LOS		A			A		
Approach Delay		2.6			1.0		
Approach LOS		A			A		

Intersection Summary

Area Type: Other
 Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 75 (83%), Referenced to phase 2:NWT and 6:SETL, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.49
 Intersection Signal Delay: 2.1
 Intersection LOS: A
 Intersection Capacity Utilization 27.8%
 ICU Level of Service A
 Analysis Period (min) 15

Splits and Phases: 1993: Jeffries Service Dr & Left Turn Bridge



Lanes, Volumes, Timings
 2609: Schaefer Hwy & Schoolcraft Rd

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	28	109	5	29	114	42	17	417	55	47	423	35
Future Volume (vph)	28	109	5	29	114	42	17	417	55	47	423	35
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	60		0	60		0	80		250	80		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00	1.00		0.99	0.99		1.00	1.00		1.00	1.00	
Fr _t		0.988			0.963			0.979			0.984	
Fl _t Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1228	1839	0	1805	1734	0	1805	1747	0	1671	1789	0
Fl _t Permitted	0.637			0.639			0.296			0.285		
Satd. Flow (perm)	822	1839	0	1204	1734	0	562	1747	0	499	1789	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		5			20			13			9	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		699			1470			1209			215	
Travel Time (s)		15.9			33.4			27.5			4.9	
Confl. Peds. (#/hr)	1		6	6		1	1		6	6		1
Peak Hour Factor	0.53	0.63	0.33	0.72	0.80	0.89	0.75	0.91	0.72	0.88	0.90	0.63
Heavy Vehicles (%)	47%	2%	0%	0%	1%	17%	0%	7%	0%	8%	2%	23%
Adj. Flow (vph)	53	173	15	40	143	47	23	458	76	53	470	56
Shared Lane Traffic (%)												
Lane Group Flow (vph)	53	188	0	40	190	0	23	534	0	53	526	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	

Lanes, Volumes, Timings
 2609: Schaefer Hwy & Schoolcraft Rd

08/08/2022

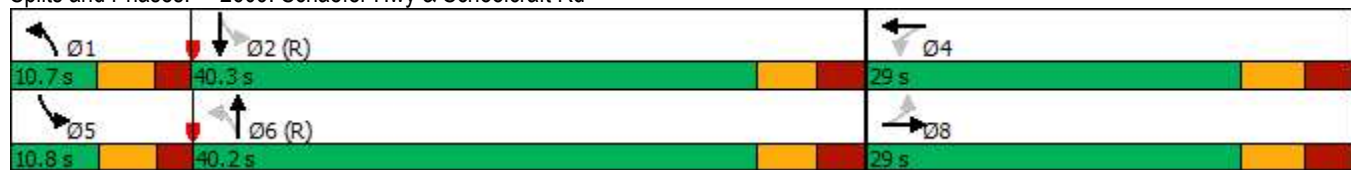


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		
Detector Phase	8	8		4	4		1	6		5	2	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0		5.0	9.0		5.0	9.0	
Minimum Split (s)	28.8	28.8		28.8	28.8		10.6	26.6		10.6	26.6	
Total Split (s)	29.0	29.0		29.0	29.0		10.7	40.2		10.8	40.3	
Total Split (%)	36.3%	36.3%		36.3%	36.3%		13.4%	50.3%		13.5%	50.4%	
Maximum Green (s)	22.2	22.2		22.2	22.2		5.1	33.6		5.2	33.7	
Yellow Time (s)	3.8	3.8		3.8	3.8		3.5	3.6		3.5	3.6	
All-Red Time (s)	3.0	3.0		3.0	3.0		2.1	3.0		2.1	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.8	6.8		6.8	6.8		5.6	6.6		5.6	6.6	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	Max	Max		Max	Max		Max	C-Max		Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0			13.0			13.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	22.2	22.2		22.2	22.2		39.7	33.6		39.9	33.7	
Actuated g/C Ratio	0.28	0.28		0.28	0.28		0.50	0.42		0.50	0.42	
v/c Ratio	0.23	0.37		0.12	0.38		0.06	0.72		0.16	0.69	
Control Delay	25.6	25.1		22.9	23.6		8.2	25.5		9.2	24.5	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	25.6	25.1		22.9	23.6		8.2	25.5		9.2	24.5	
LOS	C	C		C	C		A	C		A	C	
Approach Delay		25.2			23.4			24.8			23.1	
Approach LOS		C			C			C			C	

Intersection Summary

Area Type: Other
 Cycle Length: 80
 Actuated Cycle Length: 80
 Offset: 0 (0%), Referenced to phase 2:SBTL and 6:NBTL, Start of Green
 Natural Cycle: 70
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.72
 Intersection Signal Delay: 24.1
 Intersection LOS: C
 Intersection Capacity Utilization 68.6%
 ICU Level of Service C
 Analysis Period (min) 15

Splits and Phases: 2609: Schaefer Hwy & Schoolcraft Rd



Lanes, Volumes, Timings
3011: Schaefer Hwy & I-96 Service Dr

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				↙	↕		↙	↕			↕	
Traffic Volume (vph)	0	0	0	119	176	4	180	397	0	0	253	202
Future Volume (vph)	0	0	0	119	176	4	180	397	0	0	253	202
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	90		0	0		0
Storage Lanes	0		0	1		0	1		0	0		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	0.95
Ped Bike Factor							1.00					0.99
Frt					0.997							0.932
Flt Protected				0.950			0.950					
Satd. Flow (prot)	0	0	0	1787	3564	0	1752	3471	0	0	3283	0
Flt Permitted				0.950			0.431					
Satd. Flow (perm)	0	0	0	1787	3564	0	793	3471	0	0	3283	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					2							249
Link Speed (mph)		35			35			30				30
Link Distance (ft)		298			375			384				332
Travel Time (s)		5.8			7.3			8.7				7.5
Confl. Peds. (#/hr)							4					4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.87	0.76	0.92	0.92	0.84	0.81
Heavy Vehicles (%)	2%	2%	2%	1%	1%	0%	3%	4%	2%	2%	3%	0%
Adj. Flow (vph)	0	0	0	129	191	4	207	522	0	0	301	249
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	129	195	0	207	522	0	0	550	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors				1	2		1	2				2
Detector Template				Left	Thru		Left	Thru				Thru
Leading Detector (ft)				20	100		20	100				100
Trailing Detector (ft)				0	0		0	0				0
Detector 1 Position(ft)				0	0		0	0				0
Detector 1 Size(ft)				20	6		20	6				6
Detector 1 Type				Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex				Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)				0.0	0.0		0.0	0.0				0.0
Detector 1 Queue (s)				0.0	0.0		0.0	0.0				0.0
Detector 1 Delay (s)				0.0	0.0		0.0	0.0				0.0
Detector 2 Position(ft)					94			94				94
Detector 2 Size(ft)					6			6				6
Detector 2 Type					Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)					0.0			0.0				0.0

Lane Group	Ø4
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	

Lanes, Volumes, Timings
3011: Schaefer Hwy & I-96 Service Dr

08/08/2022

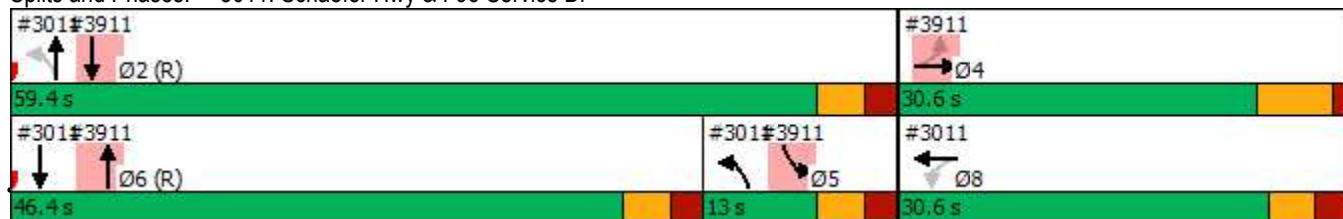


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type				Perm	NA		pm+pt	NA				NA
Protected Phases					8		5	2				6
Permitted Phases				8			2					
Detector Phase				8	8		5	2				6
Switch Phase												
Minimum Initial (s)				10.0	10.0		7.0	10.0				10.0
Minimum Split (s)				28.7	28.7		12.4	23.4				23.4
Total Split (s)				30.6	30.6		13.0	59.4				46.4
Total Split (%)				34.0%	34.0%		14.4%	66.0%				51.6%
Maximum Green (s)				24.9	24.9		7.6	54.0				41.0
Yellow Time (s)				3.0	3.0		3.2	3.2				3.2
All-Red Time (s)				2.7	2.7		2.2	2.2				2.2
Lost Time Adjust (s)				0.0	0.0		0.0	0.0				0.0
Total Lost Time (s)				5.7	5.7		5.4	5.4				5.4
Lead/Lag							Lag					Lead
Lead-Lag Optimize?							Yes					Yes
Vehicle Extension (s)				3.0	3.0		3.0	3.0				3.0
Recall Mode				None	None		None	C-Max				C-Max
Walk Time (s)				7.0	7.0			7.0				7.0
Flash Dont Walk (s)				16.0	16.0			11.0				11.0
Pedestrian Calls (#/hr)				0	0			0				0
Act Effct Green (s)				14.5	14.5		64.4	64.4				51.4
Actuated g/C Ratio				0.16	0.16		0.72	0.72				0.57
v/c Ratio				0.45	0.34		0.32	0.21				0.28
Control Delay				23.8	19.3		3.7	2.4				3.1
Queue Delay				0.0	0.0		0.0	0.0				0.2
Total Delay				23.8	19.3		3.7	2.4				3.2
LOS				C	B		A	A				A
Approach Delay					21.1			2.8				3.2
Approach LOS					C			A				A

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	80 (89%), Referenced to phase 2:NBTL and 6:SBT, Start of Green
Natural Cycle:	80
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.66
Intersection Signal Delay:	6.6
Intersection LOS:	A
Intersection Capacity Utilization:	57.8%
ICU Level of Service:	B
Analysis Period (min):	15

Splits and Phases: 3011: Schaefer Hwy & I-96 Service Dr



Lane Group	Ø4
Turn Type	
Protected Phases	4
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
Minimum Split (s)	30.5
Total Split (s)	30.6
Total Split (%)	34%
Maximum Green (s)	24.1
Yellow Time (s)	5.0
All-Red Time (s)	1.5
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	17.0
Pedestrian Calls (#/hr)	0
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Lanes, Volumes, Timings
3911: Schaefer Hwy & Jeffries Service Dr

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕↕↕						↔		↙	↕↕	
Traffic Volume (vph)	179	135	189	0	0	0	0	398	99	58	314	0
Future Volume (vph)	179	135	189	0	0	0	0	398	99	58	314	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	0		0	90		0
Storage Lanes	0		0	0		0	0		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	0.91	0.91	0.91	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00
Ped Bike Factor								1.00		1.00		
Fr _t		0.944						0.972				
Fl _t Protected		0.982								0.950		
Satd. Flow (prot)	0	4736	0	0	0	0	0	1766	0	1770	3505	0
Fl _t Permitted		0.982								0.950		
Satd. Flow (perm)	0	4736	0	0	0	0	0	1766	0	1765	3505	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		165						19				
Link Speed (mph)		35			35			30				30
Link Distance (ft)		417			354			621				384
Travel Time (s)		8.1			6.9			14.1				8.7
Confl. Peds. (#/hr)									4	4		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.75	0.72	0.81	0.79	0.92
Heavy Vehicles (%)	1%	3%	1%	2%	2%	2%	2%	5%	1%	2%	3%	2%
Adj. Flow (vph)	195	147	205	0	0	0	0	531	138	72	397	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	547	0	0	0	0	0	669	0	72	397	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2						2		1	2	
Detector Template	Left	Thru						Thru		Left	Thru	
Leading Detector (ft)	20	100						100		20	100	
Trailing Detector (ft)	0	0						0		0	0	
Detector 1 Position(ft)	0	0						0		0	0	
Detector 1 Size(ft)	20	6						6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex						Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0						0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0						0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0						0.0		0.0	0.0	
Detector 2 Position(ft)		94						94			94	
Detector 2 Size(ft)		6						6			6	
Detector 2 Type		Cl+Ex						Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0						0.0			0.0	

Lane Group	Ø8
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Ped Bike Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Confl. Peds. (#/hr)	
Peak Hour Factor	
Heavy Vehicles (%)	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	

Lanes, Volumes, Timings
 3911: Schaefer Hwy & Jeffries Service Dr

08/08/2022

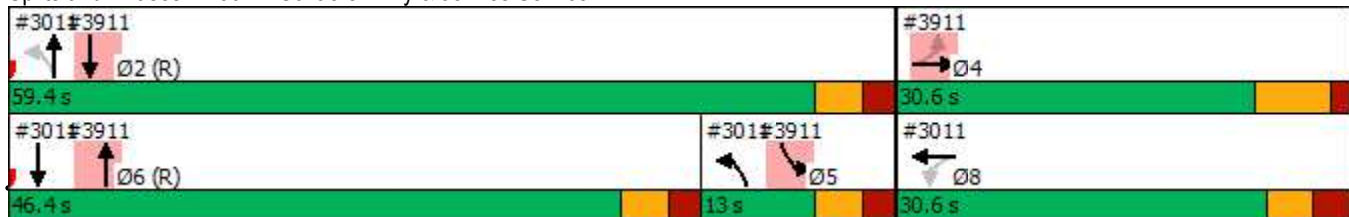


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA						NA		Prot	NA	
Protected Phases		4						6		5	2	
Permitted Phases	4											
Detector Phase	4	4						6		5	2	
Switch Phase												
Minimum Initial (s)	10.0	10.0						10.0		7.0	10.0	
Minimum Split (s)	30.5	30.5						23.4		12.4	23.4	
Total Split (s)	30.6	30.6						46.4		13.0	59.4	
Total Split (%)	34.0%	34.0%						51.6%		14.4%	66.0%	
Maximum Green (s)	24.1	24.1						41.0		7.6	54.0	
Yellow Time (s)	5.0	5.0						3.2		3.2	3.2	
All-Red Time (s)	1.5	1.5						2.2		2.2	2.2	
Lost Time Adjust (s)		0.0						0.0		0.0	0.0	
Total Lost Time (s)		6.5						5.4		5.4	5.4	
Lead/Lag								Lead		Lag		
Lead-Lag Optimize?								Yes		Yes		
Vehicle Extension (s)	3.0	3.0						3.0		3.0	3.0	
Recall Mode	None	None						C-Max		None	C-Max	
Walk Time (s)	7.0	7.0						7.0			7.0	
Flash Dont Walk (s)	17.0	17.0						11.0			11.0	
Pedestrian Calls (#/hr)	0	0						0			0	
Act Effct Green (s)		13.7						51.4		7.6	64.4	
Actuated g/C Ratio		0.15						0.57		0.08	0.72	
v/c Ratio		0.64						0.66		0.48	0.16	
Control Delay		27.9						17.5		48.7	4.6	
Queue Delay		0.0						0.0		0.0	0.0	
Total Delay		27.9						17.5		48.7	4.6	
LOS		C						B		D	A	
Approach Delay		27.9						17.5			11.4	
Approach LOS		C						B			B	

Intersection Summary

Area Type:	Other
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	80 (89%), Referenced to phase 2:NBTL and 6:SBT, Start of Green
Natural Cycle:	80
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.66
Intersection Signal Delay:	19.2
Intersection LOS:	B
Intersection Capacity Utilization:	57.8%
ICU Level of Service:	B
Analysis Period (min):	15

Splits and Phases: 3911: Schaefer Hwy & Jeffries Service Dr



Lane Group	Ø8
Turn Type	
Protected Phases	8
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	10.0
Minimum Split (s)	28.7
Total Split (s)	30.6
Total Split (%)	34%
Maximum Green (s)	24.9
Yellow Time (s)	3.0
All-Red Time (s)	2.7
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	16.0
Pedestrian Calls (#/hr)	0
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Intersection Summary	

Lanes, Volumes, Timings
4001: Schaefer Hwy & Lyndon St

08/08/2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	37	103	32	57	130	29	39	386	54	39	382	32
Future Volume (vph)	37	103	32	57	130	29	39	386	54	39	382	32
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	80		0	80		0	80		0	80		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	65			65			65			65		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Fr _t		0.958			0.975			0.979			0.981	
Fl _t Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1811	0	1805	1848	0	1805	3365	0	1752	3246	0
Fl _t Permitted	0.642			0.658			0.477			0.471		
Satd. Flow (perm)	1218	1811	0	1244	1848	0	902	3365	0	868	3246	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		34			17			33			29	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		648			545			1423			685	
Travel Time (s)		14.7			12.4			32.3			15.6	
Confl. Peds. (#/hr)	2		6	6		2	6		1	1		6
Peak Hour Factor	0.73	0.92	0.72	0.73	0.85	0.96	0.68	0.91	0.77	0.60	0.91	0.52
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	5%	3%	3%	10%	0%
Adj. Flow (vph)	51	112	44	78	153	30	57	424	70	65	420	62
Shared Lane Traffic (%)												
Lane Group Flow (vph)	51	156	0	78	183	0	57	494	0	65	482	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	

Lanes, Volumes, Timings
4001: Schaefer Hwy & Lyndon St

08/08/2022

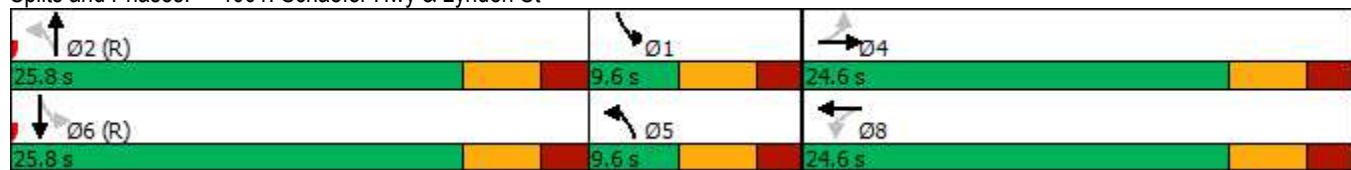


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		4.0	15.0		4.0	15.0	
Minimum Split (s)	24.6	24.6		24.6	24.6		9.6	26.6		9.6	26.6	
Total Split (s)	24.6	24.6		24.6	24.6		9.6	25.8		9.6	25.8	
Total Split (%)	41.0%	41.0%		41.0%	41.0%		16.0%	43.0%		16.0%	43.0%	
Maximum Green (s)	19.0	19.0		19.0	19.0		4.0	20.2		4.0	20.2	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	2.1	2.1		2.1	2.1		2.1	2.1		2.1	2.1	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.6	5.6		5.6	5.6		5.6	5.6		5.6	5.6	
Lead/Lag							Lag	Lead		Lag	Lead	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	Min	Min		Min	Min		None	C-Max		None	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	12.0	12.0		12.0	12.0			14.0			14.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	11.8	11.8		11.8	11.8		35.2	31.2		35.2	31.2	
Actuated g/C Ratio	0.20	0.20		0.20	0.20		0.59	0.52		0.59	0.52	
v/c Ratio	0.21	0.41		0.32	0.49		0.10	0.28		0.11	0.28	
Control Delay	21.5	19.1		23.6	23.4		6.1	9.5		6.3	9.7	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	21.5	19.1		23.6	23.4		6.1	9.5		6.3	9.7	
LOS	C	B		C	C		A	A		A	A	
Approach Delay		19.7			23.5			9.2			9.3	
Approach LOS		B			C			A			A	

Intersection Summary

Area Type:	Other
Cycle Length:	60
Actuated Cycle Length:	60
Offset:	0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle:	65
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.49
Intersection Signal Delay:	13.0
Intersection LOS:	B
Intersection Capacity Utilization:	57.5%
ICU Level of Service:	B
Analysis Period (min):	15

Splits and Phases: 4001: Schaefer Hwy & Lyndon St



Intersection												
Int Delay, s/veh	0.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	10	197	5	0	175	0	0	0	0	0	0	10
Future Vol, veh/h	10	197	5	0	175	0	0	0	0	0	0	10
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	25	25	25	25	25	25	25	25	25	25	25	25
Heavy Vehicles, %	0	2	0	0	5	0	0	0	0	0	0	0
Mvmt Flow	40	788	20	0	700	0	0	0	0	0	0	40

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	700	0	0	808	0	0	1598	1578	798	1578	1588	700
Stage 1	-	-	-	-	-	-	878	878	-	700	700	-
Stage 2	-	-	-	-	-	-	720	700	-	878	888	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	906	-	-	826	-	-	87	110	389	90	109	443
Stage 1	-	-	-	-	-	-	345	368	-	433	444	-
Stage 2	-	-	-	-	-	-	422	444	-	345	365	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	906	-	-	826	-	-	74	101	389	85	100	443
Mov Cap-2 Maneuver	-	-	-	-	-	-	74	101	-	85	100	-
Stage 1	-	-	-	-	-	-	317	339	-	398	444	-
Stage 2	-	-	-	-	-	-	384	444	-	317	336	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.4	0	0	13.9
HCM LOS			A	B

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	906	-	-	826	-	-	443
HCM Lane V/C Ratio	-	0.044	-	-	-	-	-	0.09
HCM Control Delay (s)	0	9.2	0	-	0	-	-	13.9
HCM Lane LOS	A	A	A	-	A	-	-	B
HCM 95th %tile Q(veh)	-	0.1	-	-	0	-	-	0.3

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	3	1	8	4	1	9	5	254	20	6	300	4
Future Vol, veh/h	3	1	8	4	1	9	5	254	20	6	300	4
Conflicting Peds, #/hr	0	0	1	1	0	0	2	0	0	0	0	2
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	50	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	60	60	60	60	60	60	100	81	60	60	95	75
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	5	2	13	7	2	15	5	314	33	10	316	5

Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	690	698	322	688	684	331	323	0	0	347	0	0
Stage 1	341	341	-	341	341	-	-	-	-	-	-	-
Stage 2	349	357	-	347	343	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	362	367	724	363	374	715	1248	-	-	1223	-	-
Stage 1	678	642	-	678	642	-	-	-	-	-	-	-
Stage 2	671	632	-	673	641	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	349	362	722	351	369	715	1246	-	-	1223	-	-
Mov Cap-2 Maneuver	349	362	-	351	369	-	-	-	-	-	-	-
Stage 1	674	636	-	675	639	-	-	-	-	-	-	-
Stage 2	653	629	-	653	635	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	12		12.2		0.1		0.2	
HCM LOS	B		B					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1246	-	-	535	524	1223	-	-
HCM Lane V/C Ratio	0.004	-	-	0.037	0.045	0.008	-	-
HCM Control Delay (s)	7.9	-	-	12	12.2	8	-	-
HCM Lane LOS	A	-	-	B	B	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0.1	0.1	0	-	-

Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	1	0	480	7	5	503
Future Vol, veh/h	1	0	480	7	5	503
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	100	100	2	100	100	2
Mvmt Flow	1	0	522	8	5	547

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	810	265	0	0	530
Stage 1	526	-	-	-	-
Stage 2	284	-	-	-	-
Critical Hdwy	8.8	8.9	-	-	6.1
Critical Hdwy Stg 1	7.8	-	-	-	-
Critical Hdwy Stg 2	7.8	-	-	-	-
Follow-up Hdwy	4.5	4.3	-	-	3.2
Pot Cap-1 Maneuver	176	507	-	-	575
Stage 1	349	-	-	-	-
Stage 2	514	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	174	507	-	-	575
Mov Cap-2 Maneuver	174	-	-	-	-
Stage 1	349	-	-	-	-
Stage 2	508	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	25.8	0	0.2
HCM LOS	D		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	174	-	575
HCM Lane V/C Ratio	-	-	0.006	-	0.009
HCM Control Delay (s)	-	-	25.8	0	11.3
HCM Lane LOS	-	-	D	A	B
HCM 95th %tile Q(veh)	-	-	0	-	0

Intersection						
Int Delay, s/veh	0.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	41	12	467	13	4	467
Future Vol, veh/h	41	12	467	13	4	467
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	6	0	0	4
Mvmt Flow	45	13	508	14	4	508

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	777	261	0	0	522
Stage 1	515	-	-	-	-
Stage 2	262	-	-	-	-
Critical Hdwy	6.8	6.9	-	-	4.1
Critical Hdwy Stg 1	5.8	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2
Pot Cap-1 Maneuver	338	744	-	-	1055
Stage 1	570	-	-	-	-
Stage 2	764	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	336	744	-	-	1055
Mov Cap-2 Maneuver	336	-	-	-	-
Stage 1	570	-	-	-	-
Stage 2	760	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	15.6	0	0.1
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	336	744	1055
HCM Lane V/C Ratio	-	-	0.133	0.018	0.004
HCM Control Delay (s)	-	-	17.3	9.9	8.4
HCM Lane LOS	-	-	C	A	A
HCM 95th %tile Q(veh)	-	-	0.5	0.1	0














INNOVATIVE IDEAS
EXCEPTIONAL DESIGN
UNMATCHED CLIENT SERVICE

CONSOLIDATED ACCESS



Lanes, Volumes, Timings
2610: Schaefer Hwy & Coolidge Driveway

11/02/2021

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	160	68	581	90	45	487
Future Volume (vph)	160	68	581	90	45	487
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	150	
Storage Lanes	1	1		0	1	
Taper Length (ft)	65				65	
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Frt		0.850	0.980			
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1456	1099	3291	0	1043	3539
Flt Permitted	0.950				0.215	
Satd. Flow (perm)	1456	1099	3291	0	236	3539
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		74	33			
Link Speed (mph)	30		30			30
Link Distance (ft)	221		600			1664
Travel Time (s)	5.0		13.6			37.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	24%	47%	2%	43%	73%	2%
Adj. Flow (vph)	174	74	632	98	49	529
Shared Lane Traffic (%)						
Lane Group Flow (vph)	174	74	730	0	49	529
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		0			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Turn Type	Prot	Perm	NA		pm+pt	NA
Protected Phases	8		2		1	6
Permitted Phases		8			6	
Minimum Split (s)	22.5	22.5	22.5		9.5	22.5
Total Split (s)	22.5	22.5	23.0		9.5	32.5
Total Split (%)	40.9%	40.9%	41.8%		17.3%	59.1%
Maximum Green (s)	15.6	15.6	18.1		5.0	27.6
Yellow Time (s)	3.0	3.0	3.2		3.5	3.2
All-Red Time (s)	3.9	3.9	1.7		1.0	1.7
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.9	6.9	4.9		4.5	4.9
Lead/Lag			Lag		Lead	
Lead-Lag Optimize?			Yes		Yes	
Walk Time (s)			4.0			4.0
Flash Dont Walk (s)			9.0			9.0
Pedestrian Calls (#/hr)			0			0
Act Effct Green (s)	15.6	15.6	18.1		28.0	27.6
Actuated g/C Ratio	0.28	0.28	0.33		0.51	0.50

Lanes, Volumes, Timings
 2610: Schaefer Hwy & Coolidge Driveway

11/02/2021

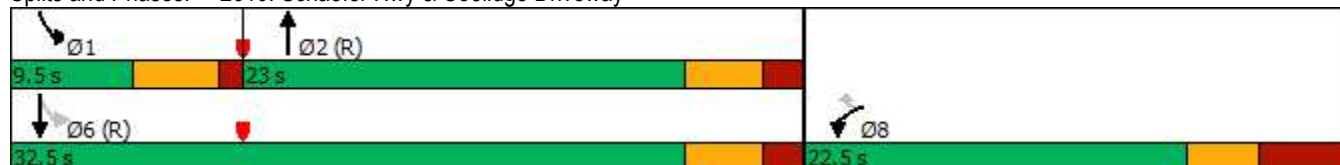


Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
v/c Ratio	0.42	0.20	0.66		0.25	0.30
Control Delay	19.9	6.2	18.5		10.4	8.6
Queue Delay	0.0	0.0	0.0		0.0	0.0
Total Delay	19.9	6.2	18.5		10.4	8.6
LOS	B	A	B		B	A
Approach Delay	15.8		18.5			8.7
Approach LOS	B		B			A
Queue Length 50th (ft)	46	0	100		8	48
Queue Length 95th (ft)	93	24	151		21	74
Internal Link Dist (ft)	141		520			1584
Turn Bay Length (ft)					150	
Base Capacity (vph)	412	364	1105		193	1775
Starvation Cap Reductn	0	0	0		0	0
Spillback Cap Reductn	0	0	0		0	0
Storage Cap Reductn	0	0	0		0	0
Reduced v/c Ratio	0.42	0.20	0.66		0.25	0.30

Intersection Summary

Area Type: Other
 Cycle Length: 55
 Actuated Cycle Length: 55
 Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green
 Natural Cycle: 55
 Control Type: Pretimed
 Maximum v/c Ratio: 0.66
 Intersection Signal Delay: 14.5
 Intersection Capacity Utilization 45.5%
 Analysis Period (min) 15
 Intersection LOS: B
 ICU Level of Service A

Splits and Phases: 2610: Schaefer Hwy & Coolidge Driveway



Lanes, Volumes, Timings
2610: Schaefer Hwy & Coolidge Driveway

11/02/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	116	42	415	65	39	411
Future Volume (vph)	116	42	415	65	39	411
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	150	
Storage Lanes	1	1		0	1	
Taper Length (ft)	65				65	
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Frt		0.850	0.980			
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1556	1188	3219	0	992	3539
Flt Permitted	0.950				0.332	
Satd. Flow (perm)	1556	1188	3219	0	347	3539
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		46	31			
Link Speed (mph)	30		30			30
Link Distance (ft)	208		600			1664
Travel Time (s)	4.7		13.6			37.8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	16%	36%	2%	60%	82%	2%
Adj. Flow (vph)	126	46	451	71	42	447
Shared Lane Traffic (%)						
Lane Group Flow (vph)	126	46	522	0	42	447
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Turn Type	Prot	Perm	NA		pm+pt	NA
Protected Phases	8		2		1	6
Permitted Phases		8			6	
Minimum Split (s)	24.9	24.9	22.6		9.5	22.6
Total Split (s)	25.0	25.0	25.0		10.0	35.0
Total Split (%)	41.7%	41.7%	41.7%		16.7%	58.3%
Maximum Green (s)	18.1	18.1	20.1		5.5	30.1
Yellow Time (s)	3.0	3.0	3.2		3.5	3.2
All-Red Time (s)	3.9	3.9	1.7		1.0	1.7
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.9	6.9	4.9		4.5	4.9
Lead/Lag			Lag		Lead	
Lead-Lag Optimize?			Yes		Yes	
Walk Time (s)			4.0			4.0
Flash Dont Walk (s)			9.0			9.0
Pedestrian Calls (#/hr)			0			0
Act Effct Green (s)	18.1	18.1	20.1		30.5	30.1
Actuated g/C Ratio	0.30	0.30	0.34		0.51	0.50

Lanes, Volumes, Timings
 2610: Schaefer Hwy & Coolidge Driveway

11/02/2021

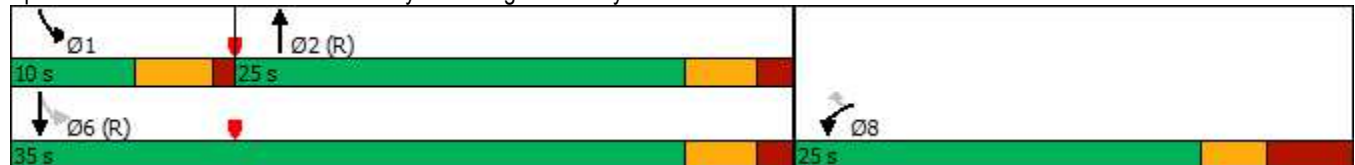


Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
v/c Ratio	0.27	0.12	0.48		0.18	0.25
Control Delay	17.9	6.4	16.5		4.7	5.4
Queue Delay	0.0	0.0	0.0		0.0	0.0
Total Delay	17.9	6.4	16.5		4.7	5.4
LOS	B	A	B		A	A
Approach Delay	14.8		16.5			5.4
Approach LOS	B		B			A
Queue Length 50th (ft)	34	0	72		3	43
Queue Length 95th (ft)	72	19	111		4	14
Internal Link Dist (ft)	128		520			1584
Turn Bay Length (ft)					150	
Base Capacity (vph)	469	390	1098		235	1775
Starvation Cap Reductn	0	0	0		0	0
Spillback Cap Reductn	0	0	0		0	0
Storage Cap Reductn	0	0	0		0	0
Reduced v/c Ratio	0.27	0.12	0.48		0.18	0.25

Intersection Summary

Area Type:	Other
Cycle Length:	60
Actuated Cycle Length:	60
Offset:	0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green
Natural Cycle:	60
Control Type:	Pretimed
Maximum v/c Ratio:	0.48
Intersection Signal Delay:	11.7
Intersection LOS:	B
Intersection Capacity Utilization	37.7%
ICU Level of Service	A
Analysis Period (min)	15

Splits and Phases: 2610: Schaefer Hwy & Coolidge Driveway





INNOVATIVE IDEAS
EXCEPTIONAL DESIGN
UNMATCHED CLIENT SERVICE

EMERGENCY BUS EXIT SCENARIO

Intersection												
Int Delay, s/veh	2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	6	195	2	1	188	5	1	0	7	11	0	26
Future Vol, veh/h	6	195	2	1	188	5	1	0	7	11	0	26
Conflicting Peds, #/hr	3	0	1	1	0	3	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	63	94	50	25	86	50	25	25	50	63	25	50
Heavy Vehicles, %	0	7	0	0	7	0	0	0	0	45	0	73
Mvmt Flow	10	207	4	4	219	10	4	0	14	17	0	52

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	232	0	0	212	0	0	488	470	210	471	467	227
Stage 1	-	-	-	-	-	-	230	230	-	235	235	-
Stage 2	-	-	-	-	-	-	258	240	-	236	232	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.55	6.5	6.93
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.55	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.55	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.905	4	3.957
Pot Cap-1 Maneuver	1348	-	-	1370	-	-	493	495	835	438	496	664
Stage 1	-	-	-	-	-	-	777	718	-	681	714	-
Stage 2	-	-	-	-	-	-	751	711	-	680	716	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1344	-	-	1369	-	-	450	488	834	426	489	662
Mov Cap-2 Maneuver	-	-	-	-	-	-	450	488	-	426	489	-
Stage 1	-	-	-	-	-	-	770	712	-	674	710	-
Stage 2	-	-	-	-	-	-	690	707	-	663	710	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.3	0.1	10.3	12
HCM LOS			B	B

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	701	1344	-	-	1369	-	-	581
HCM Lane V/C Ratio	0.026	0.007	-	-	0.003	-	-	0.12
HCM Control Delay (s)	10.3	7.7	0	-	7.6	0	-	12
HCM Lane LOS	B	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.4

Intersection												
Int Delay, s/veh	2.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	26	1	12	8	7	8	2	327	11	8	305	1
Future Vol, veh/h	26	1	12	8	7	8	2	327	11	8	305	1
Conflicting Peds, #/hr	0	0	0	0	0	0	1	0	1	1	0	1
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	50	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	60	60	60	60	60	60	100	85	63	60	87	60
Heavy Vehicles, %	88	0	25	0	0	0	0	5	0	0	3	0
Mvmt Flow	43	2	20	13	12	13	2	385	17	13	351	2

Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	789	786	353	788	779	395	354	0	0	403	0	0
Stage 1	379	379	-	399	399	-	-	-	-	-	-	-
Stage 2	410	407	-	389	380	-	-	-	-	-	-	-
Critical Hdwy	7.98	6.5	6.45	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.98	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.98	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	4.292	4	3.525	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	225	326	642	311	330	659	1216	-	-	1167	-	-
Stage 1	500	618	-	631	606	-	-	-	-	-	-	-
Stage 2	479	601	-	639	617	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	212	321	641	297	325	658	1215	-	-	1166	-	-
Mov Cap-2 Maneuver	212	321	-	297	325	-	-	-	-	-	-	-
Stage 1	499	611	-	629	604	-	-	-	-	-	-	-
Stage 2	459	599	-	610	610	-	-	-	-	-	-	-

Approach	EB		WB		NB			SB		
HCM Control Delay, s	22.5		15.6		0			0.3		
HCM LOS	C		C							

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1215	-	-	270	379	1166	-	-
HCM Lane V/C Ratio	0.002	-	-	0.241	0.101	0.011	-	-
HCM Control Delay (s)	8	-	-	22.5	15.6	8.1	-	-
HCM Lane LOS	A	-	-	C	C	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0.9	0.3	0	-	-



INNOVATIVE IDEAS
EXCEPTIONAL DESIGN
UNMATCHED CLIENT SERVICE

APPENDIX C SHIFT ANALYSIS

**Space Needs Program
Employee Shift Analysis**

Begin End
Shift Start Shift

1/29/2020	Arrive	Shift Start	Shift End	Depart	Projected Number of Staff	12:00 AM	12:30 AM	1:00 AM	1:30 AM	2:00 AM	2:30 AM	3:00 AM	3:30 AM	4:00 AM	4:30 AM	5:00 AM	5:30 AM	6:00 AM	6:30 AM	7:00 AM	7:30 AM	8:00 AM	8:30 AM	9:00 AM	9:30 AM	10:00 AM	10:30 AM	11:00 AM	11:30 AM	12:00 PM	12:30 PM	1:00 PM	1:30 PM	2:00 PM	2:30 PM	3:00 PM	3:30 PM	4:00 PM	4:30 PM	5:00 PM	5:30 PM	6:00 PM	6:30 PM	7:00 PM	7:30 PM	8:00 PM	8:30 PM	9:00 PM	9:30 PM	10:00 PM	10:30 PM	11:00 PM	11:30 PM	12:00 AM				
FLEET MAINTENANCE																																																										
Existing 2020 Staff												0																																														
1 Rolling Stock Superintendent												1																																														
2 Office Assistant												1																																														
3 Auto Repair Foreman												4																																														
GAM Shift 1 - Weekday												1																																														
GAM Shift 2 - Weekday												1																																														
GAM Shift 3 - Weekday												1																																														
4 General Machinist												0																																														
5 Sheet Metal Worker												0																																														
6 Floor Operations Maintenance (GAM)												43																																														
GAM Shift 1 - Weekday												15																																														
GAM Shift 2 - Weekday												15																																														
GAM Shift 3 - Weekday												9																																														
7 Automotive Research Assistant												0																																														
8 Vehicle Maintenance Instructor												0																																														
9 Rolling Stock Superintendent (Body Shop)												0																																														
10 Foreman (Body Shop)												0																																														
11 General Mechanic (Body Shop)												0																																														
12 Vehicle Painter & Letterer (Body Shop)												0																																														
13 Manager (Electronics)												0																																														
14 Supervisor (Electronics)												0																																														
15 Repair (Electronics)												0																																														
16 Manager (Sign Shop)												0																																														
17 Supervisor (Sign Shop)												0																																														
18 Repair (Sign Shop)												0																																														
<i>keep this row blank</i>												49																																														
Existing 2020 Staff						10	10	10	10	10	10	0	16	16	16	16	16	16	16	16	16	18	18	18	18	18	18	18	34	34	34	34	34	34	34	34	34	18	18	18	18	18	28	28	28	26	26	26	26	26	26	10	10	10	10	10	10	10
250 BUS Program												0																																														
1 Rolling Stock Superintendent												1																																														
2 Office Assistant												1																																														
3 Auto Repair Foreman												4																																														
GAM Shift 1 - Weekday												1																																														
GAM Shift 2 - Weekday												1																																														
GAM Shift 3 - Weekday												1																																														
4 General Machinist												1																																														
5 Sheet Metal Worker												1																																														
6 Floor Operations Maintenance (GAM)												100																																														
GAM Shift 1 - Weekday												30																																														
GAM Shift 2 - Weekday												30																																														
GAM Shift 3 - Weekday												20																																														
7 Automotive Research Assistant												3																																														
8 Vehicle Maintenance Instructor												1																																														
9 Rolling Stock Superintendent (Body Shop)												1																																														
10 Foreman (Body Shop)												1																																														
11 General Mechanic (Body Shop)												13																																														
GAM Shift 1 - Weekday												5																																														
GAM Shift 2 - Weekday												5																																														
GAM Shift 3 - Weekday												3																																														
12 Vehicle Painter & Letterer (Body Shop)												1																																														
13 Manager (Electronics)												1																																														
14 Supervisor (Electronics)												1																																														
15 Repair (Electronics)												15																																														
Shift 1 - Weekday												5																																														
Shift 2 - Weekday												5																																														
Shift 3 - Weekday												3																																														
16 Manager (Sign Shop)												1																																														
17 Supervisor (Sign Shop)												1																																														
18 Repair (Sign Shop)												4																																														
<i>keep this row blank</i>												151																																														
250 BUS Program						27	27	27	27	27	27	0	42	42	42	42	42	42	43	43	57	57	57	57	57	57	57	100	100	100	100	100	100	100	100	100	58	58	58	58	58	85	84	84	70	70	70	70	70	70	27	27	27	27	42	42	42	

Space Needs Program
Employee Shift Analysis

Begin End
Shift Start Shift

Coolidge Bus Terminal

		Arrive	Shift Start	Shift End	Depart	Projected Number of Staff	12:00 AM	12:30 AM	1:00 AM	1:30 AM	2:00 AM	2:30 AM	3:00 AM	3:30 AM	4:00 AM	4:30 AM	5:00 AM	5:30 AM	6:00 AM	6:30 AM	7:00 AM	7:30 AM	8:00 AM	8:30 AM	9:00 AM	9:30 AM	10:00 AM	10:30 AM	11:00 AM	11:30 AM	12:00 PM	12:30 PM	1:00 PM	1:30 PM	2:00 PM	2:30 PM	3:00 PM	3:30 PM	4:00 PM	4:30 PM	5:00 PM	5:30 PM	6:00 PM	6:30 PM	7:00 PM	7:30 PM	8:00 PM	8:30 PM	9:00 PM	9:30 PM	10:00 PM	10:30 PM	11:00 PM	11:30 PM	12:00 AM
BUS OPERATIONS																																																							
216 BUS PROGRAM																																																							
6 District Superintendent (Admin/HR)																																																							
		11:30 PM	12:00 AM	8:00 AM	8:30 AM	3																																																	
Shift 1						1																																																	
Shift 2		7:30 AM	8:00 AM	4:00 PM	4:30 PM	1																																																	
Shift 3		3:30 PM	4:00 PM	12:00 AM	12:30 AM	0																																																	
7 Asst. Superintendent (Admin/HR)																																																							
		7:30 AM	8:00 AM	4:00 PM	4:30 PM	1																																																	
8 Transportation Emergency Dispatchers																																																							
						20																																																	
Shift 1		3:30 AM	4:00 AM	3:00 PM	3:30 PM	5																																																	
Shift 2		4:30 AM	5:00 AM	4:00 PM	4:30 PM	5																																																	
Shift 3		5:30 AM	6:00 AM	5:00 PM	5:30 PM	5																																																	
9 District Superintendent (Terminal)																																																							
District Superintendent (Terminal)		3:30 AM	4:00 AM	3:00 PM	3:30 PM	1																																																	
District Superintendent (Terminal)		1:30 PM	2:00 PM	11:00 PM	11:30 PM	1																																																	
10 Transportation Terminal Supervisor																																																							
						7																																																	
Transportation Terminal Supervisor		4:30 AM	5:00 AM	2:00 PM	2:30 PM	1																																																	
Transportation Terminal Supervisor		7:30 AM	8:00 AM	5:00 PM	5:30 PM	1																																																	
Transportation Terminal Supervisor		1:30 PM	2:00 PM	11:00 PM	11:30 PM	1																																																	
Transportation Terminal Supervisor		5:30 PM	6:00 PM	2:00 AM	2:30 AM	1																																																	
11 Road Supervisor																																																							
						38																																																	
Road Supervisor Shift 1 - Weekday		3:30 AM	4:00 AM	12:30 PM	1:00 PM	12																																																	
Road Supervisor Shift 2 - Weekday		11:30 AM	12:00 PM	8:30 PM	9:00 PM	12																																																	
Road Supervisor Shift 3 - Weekday		7:30 PM	8:00 PM	4:30 AM	5:00 AM	12																																																	
12 Office Assistant																																																							
		7:30 AM	8:00 AM	5:00 PM	5:30 PM	1																																																	
13 Administrative Assistant (Training)																																																							
		7:30 AM	8:00 AM	5:00 PM	5:30 PM	1																																																	
14 Terminal Instructors																																																							
						11																																																	
Terminal Instructors		7:30 AM	8:00 AM	5:00 PM	5:30 PM	4																																																	
Terminal Instructors		1:30 PM	2:00 PM	11:00 PM	11:30 PM	3																																																	
15 Bus Operators																																																							
						648																																																	
<i>Bus Pull-Out (Number of Buses)</i>																																																							
						158																																																	
Bus Operators Shift 1 - Weekday		3:30 AM	4:00 AM	12:30 PM	1:00 PM	35																																																	
Bus Operators Shift 1 - Weekday		4:00 AM	4:30 AM	1:00 PM	1:30 PM	35																																																	
Bus Operators Shift 1 - Weekday		4:30 AM	5:00 AM	1:30 PM	2:00 PM	35																																																	
Bus Operators Shift 1 - Weekday		5:00 AM	5:30 AM	2:00 PM	2:30 PM	34																																																	
<i>Bus Pull-Out (Number of Buses)</i>																																																							
						139																																																	
Bus Operators Shift 2 - Weekday		11:30 AM	12:00 PM	8:30 PM	9:00 PM	35																																																	
Bus Operators Shift 2 - Weekday		12:00 PM	12:30 PM	9:00 PM	9:30 PM	35																																																	
Bus Operators Shift 2 - Weekday		12:30 PM	1:00 PM	9:30 PM	10:00 PM	35																																																	
Bus Operators Shift 2 - Weekday		1:00 PM	1:30 PM	10:00 PM	10:30 PM	34																																																	
<i>Bus Pull-Out (Number of Buses)</i>																																																							
						70																																																	
Bus Operators Shift 3 - Weekday		7:00 PM	7:30 PM	12:30 AM	1:00 AM	18																																																	
Bus Operators Shift 3 - Weekday		7:30 PM	8:00 PM	1:00 AM	1:30 AM	18																																																	
Bus Operators Shift 3 - Weekday		8:00 PM	8:30 PM	2:00 AM	2:30 AM	17																																																	
Bus Operators Shift 3 - Weekday		8:30 PM	9:00 PM	2:00 AM	2:30 AM	17																																																	
16 Transportation Station Worker																																																							
						10																																																	
Transportation Station Worker - Shift 1 Weekday		3:30 AM	4:00 AM	12:30 PM	1:00 PM	3																																																	
Transportation Station Worker - Shift 2 Weekday		11:30 AM	12:00 PM	8:30 PM	9:00 PM	3																																																	
<i>keep this row blank</i>																																																							
216 BUS PROGRAM																																																							
						742	83	83	83	65	47	47	12	68	103	144	178	171	171	171	171	180	180	180	180	180	180	180	230	265	300	334	289	254	219	184	184	178	178	171	167	160	160	178	208	225	242	242	192	157	122	88	88	83	
Total Buses	198	Based a 25% Artic Buses																																																					
Less Spare Ratio	158	20%																																																					
Drivers using alternatate forms of travel	139	12%																																																					

**Space Needs Program
Employee Shift Analysis**

Coolidge Bus Terminal

		Begin	End																																																										
		Shift Start	Shift																																																										
12/15/2020	Arrive	Shift Start	Shift End	Depart	Projected Number of Staff	12:00 AM	12:30 AM	1:00 AM	1:30 AM	2:00 AM	2:30 AM	3:00 AM	3:30 AM	4:00 AM	4:30 AM	5:00 AM	5:30 AM	6:00 AM	6:30 AM	7:00 AM	7:30 AM	8:00 AM	8:30 AM	9:00 AM	9:30 AM	10:00 AM	10:30 AM	11:00 AM	11:30 AM	12:00 PM	12:30 PM	1:00 PM	1:30 PM	2:00 PM	2:30 PM	3:00 PM	3:30 PM	4:00 PM	4:30 PM	5:00 PM	5:30 PM	6:00 PM	6:30 PM	7:00 PM	7:30 PM	8:00 PM	8:30 PM	9:00 PM	10:00 PM	10:30 PM	11:00 PM	11:30 PM	12:00 AM								
PARTS STOREROOM																																																													
216 BUS PROGRAM																																																													
		Coolidge	Weekday	Schedule																																																									
1	Materials Management Manager	7:30 AM	8:00 AM	5:00 PM 5:30 PM	1																																																								
2	Store Operations Supervisor	7:30 AM	8:00 AM	5:00 PM 5:30 PM	1																																																								
3	Storekeeper				8																																																								
	Storekeeper - Shift 1 Weekday	3:30 AM	4:00 AM	1:00 PM 1:30 PM	2																																																								
	Storekeeper - Shift 2 Weekday	11:30 AM	12:00 PM	9:00 PM 9:30 PM	2																																																								
	Storekeeper - Shift 3 Weekday	7:30 PM	8:00 PM	2:00 AM 2:30 AM	2																																																								
4	Vehicle Operator	7:30 AM	8:00 AM	5:00 PM 5:30 PM	1																																																								
	keep this row blank				11																																																								
216 BUS PROGRAM					11	2	2	2	2	2	2	0	2	2	2	2	2	2	2	2	2	5	5	5	5	5	5	5	5	7	7	7	7	7	7	5	5	5	5	5	5	5	5	2	2	2	4	4	4	4	4	4	2	2	2	2	2				
COACH SERVICES																																																													
216 BUS PROGRAM																																																													
		Coolidge	Weekday	Schedule																																																									
1	Supervisor	5:30 PM	6:00 PM	2:00 AM 2:30 AM	1																																																								
2	Attendant (CSA)				24																																																								
	CSA - Shift 1 Weekday	2:00 AM	2:30 AM	11:00 AM 11:30 AM	3																																																								
	CSA - Shift 2 Weekday	10:30 AM	11:00 AM	6:00 PM 6:30 PM	3																																																								
	CSA - Shift 3 Weekday	5:30 PM	6:00 PM	2:30 AM 3:00 AM	20																																																								
	keep this row blank				25																																																								
216 BUS PROGRAM					25	21	21	21	21	24	24	23	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	6	6	6	3	3	3	3	3	3	3	3	3	3	3	3	3	3	24	24	24	21	21	21	21	21	21	21	21	21	21	21			
PLANT MAINTENANCE																																																													
216 BUS PROGRAM																																																													
1	Manager	6:30 AM	7:00 AM	4:00 PM 4:30 PM	1																																																								
2	Administrative Assistant	6:30 AM	7:00 AM	4:00 PM 4:30 PM	1																																																								
3	Quality Manager	6:30 AM	7:00 AM	4:00 PM 4:30 PM	1																																																								
4	Electrician	6:30 AM	7:00 AM	4:00 PM 4:30 PM	2																																																								
5	Carpenter	6:30 AM	7:00 AM	4:00 PM 4:30 PM	1																																																								
6	Millwrights	6:30 AM	7:00 AM	4:00 PM 4:30 PM	1																																																								
7	Storekeeper	6:30 AM	7:00 AM	4:00 PM 4:30 PM	1																																																								
	keep this row blank				8																																																								
216 BUS PROGRAM					8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
BUS STOP MAINTENANCE																																																													
216 BUS PROGRAM																																																													
1	Manager (Bus Stop Maintenance)	6:30 AM	7:00 AM	4:00 PM 4:30 PM	1																																																								
2	Supervisor (Bus Stop Maintenance)	6:30 AM	7:00 AM	4:00 PM 4:30 PM	1																																																								
3	Repair (Bus Stop Maintenance)	6:30 AM	7:00 AM	4:00 PM 4:30 PM	12																																																								
	keep this row blank				14																																																								
216 BUS PROGRAM					14	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
MAXIMUM STAFF ONSITE - 216 BUS PROGRAM						922	132	132	132	114	96	96	55	102	137	178	212	205	205	240	240	261	261	241	241	241	241	241	244	244	303	335	370	404	356	333	298	263	263	257	244	215	232	213	213	228	266	283	300	300	247	226	191	157	157	132					